# GENERAL NOTES

A. GENERAL SPECIFICATIONS:

MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION-ENGLISH, DATED 2012, AND AMENDMENTS.

B. DESIGN SPECFICATIONS:

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS ~6TH EDITION-2012 AND THE WASHINGTON STATE BRIDGE DESIGN MANUAL-2011,

C. DESIGN METHOD:

LOAD AND RESISTANCE FACTOR DESIGN METHOD (LRFO) USING STRENGTH, EXTREME EVENT, · SERVICE AND FATIGUE LIMIT STATES.

D. DESIGN LOADING:

1. OPERATIONAL IMPORTANCE FACTOR: 1.0 IN ACCORDANCE WITH WSDOT BRIDGE DESIGN MANUAL

2. DEAD LOADS:

UNIT WEIGHT OF REINFORCED CONCRETE IN CIP CLOSURES UNIT WEIGHT OF LIGHTWEIGHT CONCRETE IN PRESTRESSED GIRDERS UNIT WEIGHT OF LIGHTWEIGHT CONCRETE IN ALL OTHER MEMBERS TRAFFIC RAILING BARRIER 265PLF 

- ACCOUNTED FOR IN THE DESIGN
- 4. LIVE LOADS: HL-93 WITH IMPACT
- 5. UTILITIES: NO ALLOWANCE FOR FUTURE UTILITY LOADS HAS BEEN INCLUDED IN THE DESIGN
- 6. WIND LOADS: WIND LOADS ARE IN ACCORDANCE WITH AASHTO SECTION 3,8 AND THE WSDOT BRIDGE DESIGN MANUAL SECTION 3.11.
- 7. TEMPERATURE LOADS:

	STRUCTURAL	ΤĒ	MPERA			COEFFICIENT OF						
-	MATERIAL	MEAN	HIGH	LOW	RANGE	THERMAL EXPANSION						
	CONCRETE	64	+100	+0.0	100	0.000005/°F *						
	BRIDGES	, , ,	1100	10.0	100	0.0000037 F						

\* LIGHTWEIGHT CONCRETE

STATE FED. AID PROJ. NO. SHEET TOTAL

to WASH.

JOB NUMBER

BY APP'D

- 8 EARTHQUAKE LOADS: EARTHQUAKE PROVISIONS ARE IN ACCORDANCE WITH SECTION 4 OF THE BRIDGE DESIGN MANUAL
- E. DISTRIBUTION VALUES (LANES/GIRDER): LIVE LOAD DISTRBUTION IN ACCORDANCE WITH AASHTO SECTION 4.6.2.2
- F. ALL DIMENSIONS ARE MEASURED IN FEET EITHER HORIZONTALLY OR VERTICALLY UNLESS OTHERWISE SHOWN.
- G. UNLESS OTHERWISE NOTED IN THE PLANS, THE CONCRETE COVER MEASURED FROM THE FACE OF THE CONCRETE TO THE FACE OF ANY REINFORCING STEEL SHALL BE 1 1/2". ALL EXPOSED CONCRETE CORNERS SHALL BE CHAMFERED 34" UNLESS NOTED OTHERWISE.

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- H. ELEVATIONS: ALL ELEVATIONS REFER TO NATIONAL AMERICAN VERTICAL DATUM OF 1988 (NAVD-88)
- I, MATERIALS: SHALL BE AS NOTED IN THE PLANS.

DATE

J. ESTIMATED DEAD LOAD SPAN WEIGHT:

1. THE WEIGHT OF THE SUPERSTRUCTURE IS ESTIMATED USING THE FOLLOWING: UNIT WEIGHT OF REINFÖRCED CONCRETE IN CIP CLOSURES UNIT WEIGHT OF LIGHTWEIGHT CONCRETE IN PRESTRESSED GIRDERS UNIT WEIGHT OF LIGHTWEIGHT CONCRETE IN ALL OTHER MEMBERS 

K. SCREEDING DECK SLABS:

SCREED THE RIDING SURFACE OF THE BRIDGE DECK TO ACHIEVE THE FINISH GRADE ELEVATIONS SHOWN IN THE PLANS. ACCOUNT FOR THEORETICAL DEFLECTIONS DUE TO THE SELF WEIGHT, DECK CASTING SEQUENCE, DECK FORMING SYSTEM, TRAFFIC RAILINGS, CONSTRUCTION LOADS, OVERLAYS, AND TEMPORARY SHORING, ETC. AS REQUIRED

L. CONSTRUCTION JOINTS IN CONCRETE:

CONSTRUCTION JOINTS WILL BE PERMITTED ONLY AT LOCATIONS INDICATED ON PLANS. ADDITIONAL CONSTRUCTION JOINTS OR ALTERATIONS TO THOSE SHOWN SHALL REQUIRE WRITTEN APPROVAL OF THE ENGINEER

M. BEAM TEMPORARY BRACING:

THE CONTRACTOR SHALL INVESTIGATE THE BEAM STABILITY AND TEMPORARY BRACING AS REQUIRED AND SUBMIT TO THE ENGINEER FOR APPROVAL PER SPECIFICATION 6-02.3(17)F4. BEAMS SHALL NOT BE LEFT UN-BRACED DURING NON-WORKING HOURS BRACING SHALL REMAIN IN PLACED UNTIL CLOSURE POUR CONCRETE REACHES 3500PSI.

N. FOR CONSTRUCTION SEQUENCE SEE TEMPORARY WORK BENTS PLANS.

# INDEX OF SHEETS

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P-2 BRIDGE LAYOUT

PEDESTAL RETROFIT PLAN AND ELEVATION P-3PEDESTAL RETROFIT DETAILS

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GIRDER SCHEDULE AND NOTES DECK BULB TEE GIRDER DETAILS (1 OF 4) 8-9

DECK BULB TEE GIRDER DETAILS (2 OF 4)

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DECK BULB TEE GIRDER DETAILS (4 OF 4) P-11

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P-13 MEDIAN TRAFFIC BARRIER DETAILS

P-14 INTERMEDIATE DIAPHRAGM DETAILS

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P - 17FINISH GRADE ELEVATIONS (1 OF 2)

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BEARING PAD DETAILS

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I-5 SKAGIT RIVER BRIDGE **SPAN 8 REPLACEMENT** 

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GENERAL NOTES & INDEX OF SHEETS

Bupervisor

Designed By Bridge Projecte Engr.

Checked By

Prelim. Plan By

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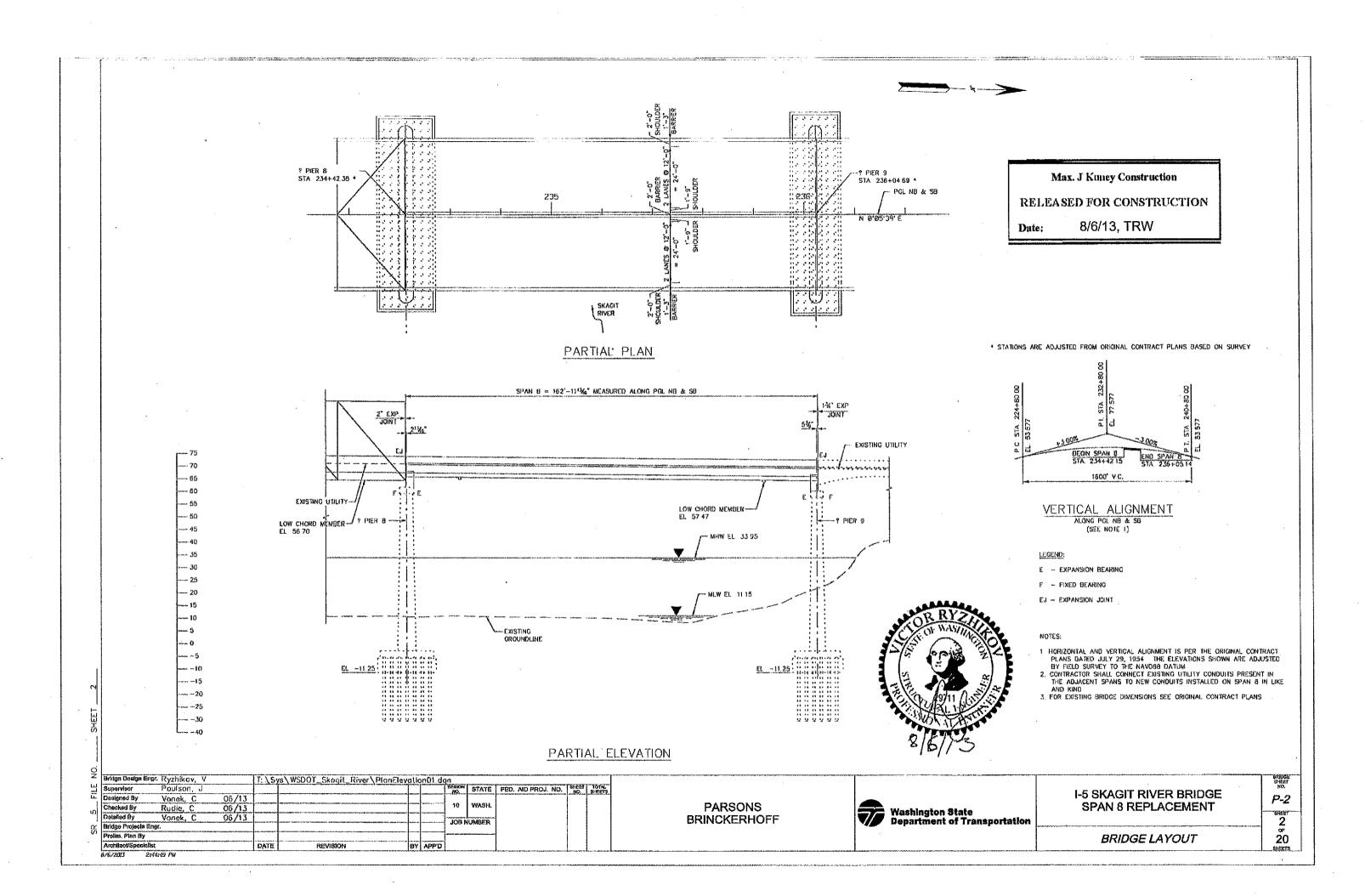
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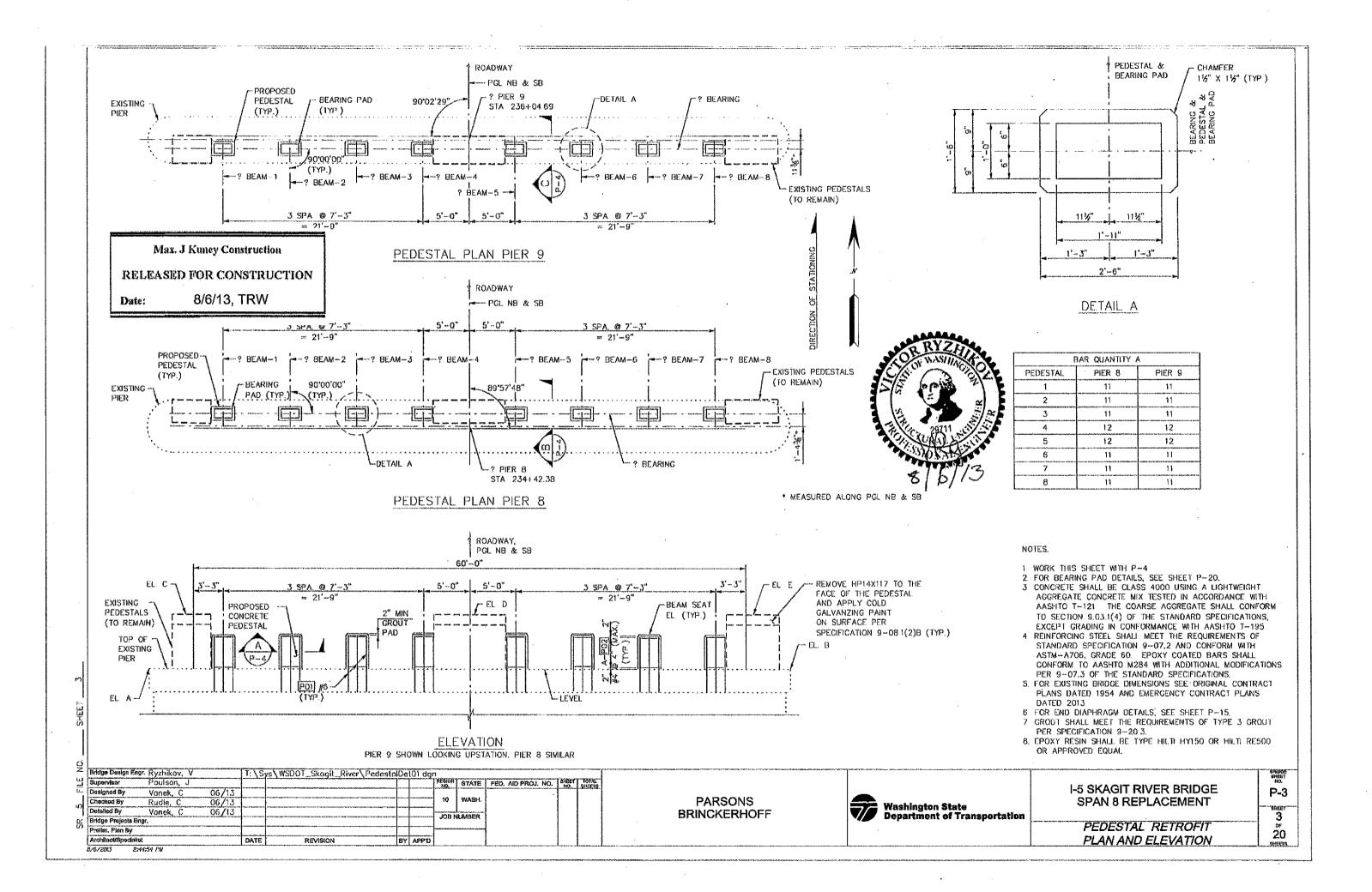
Poulson, J

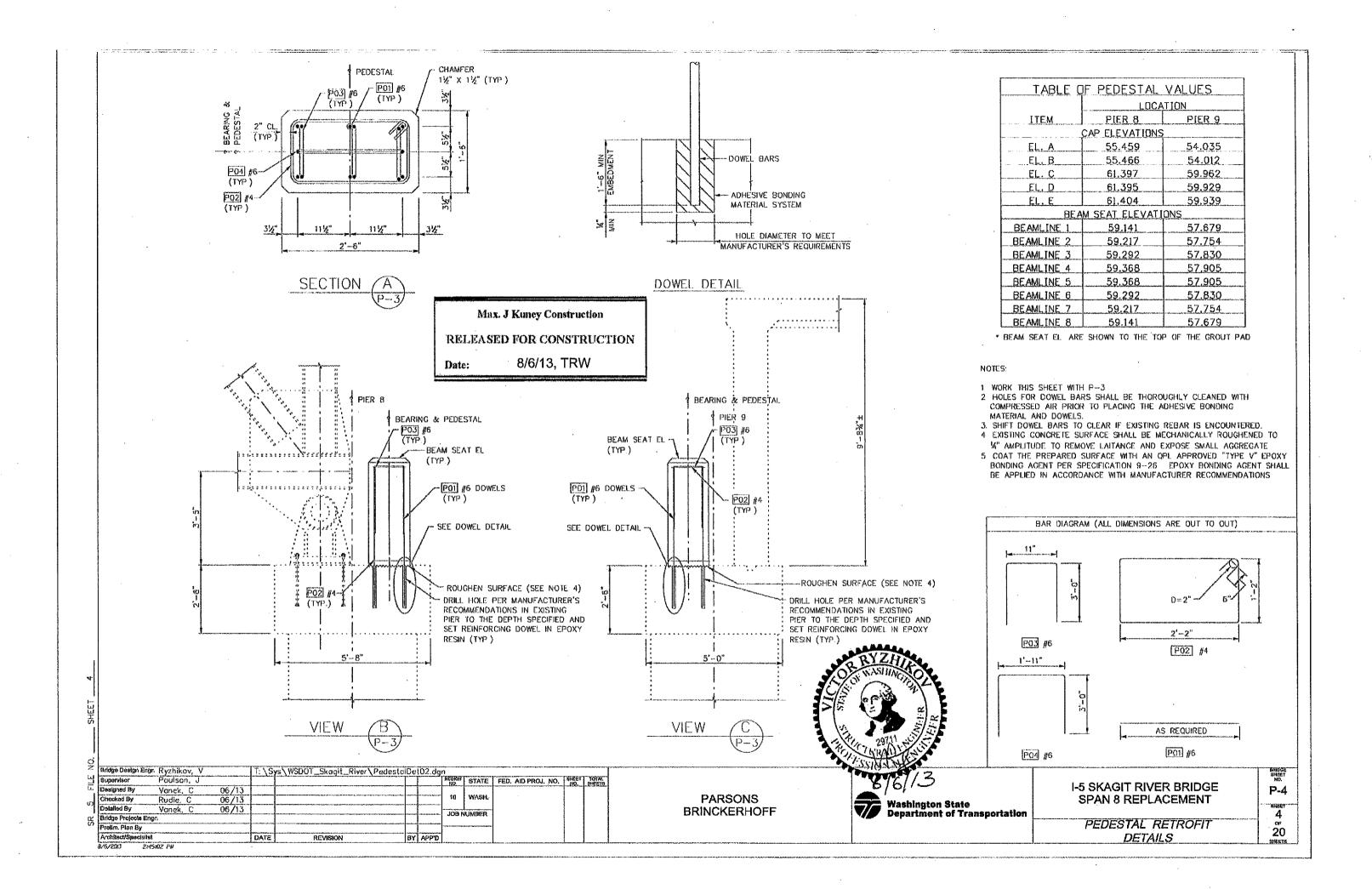
Vanek, C

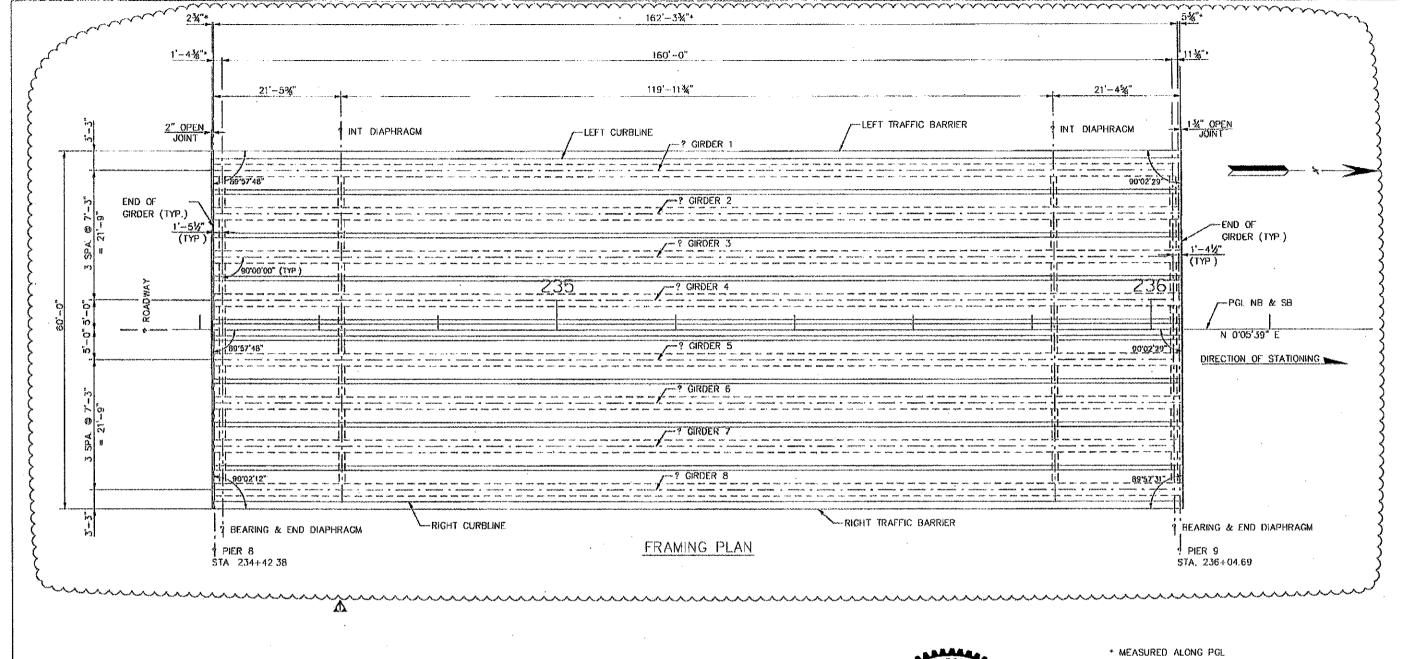
Rudie, C

Vanek, (









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NOTES:

1. FOR SUPERSTRUCTURE SECTION, SEE SHEET P-6.
2. FOR EXPANSION JOINT DATA, SEE SHEET P-19.
3. FOR DIAPHRAGM DETAILS, SEE SHEET P-14 THRU P-15.
4. FOR CAST IN PLACE HEADER DETAILS SEE SHEET P-15

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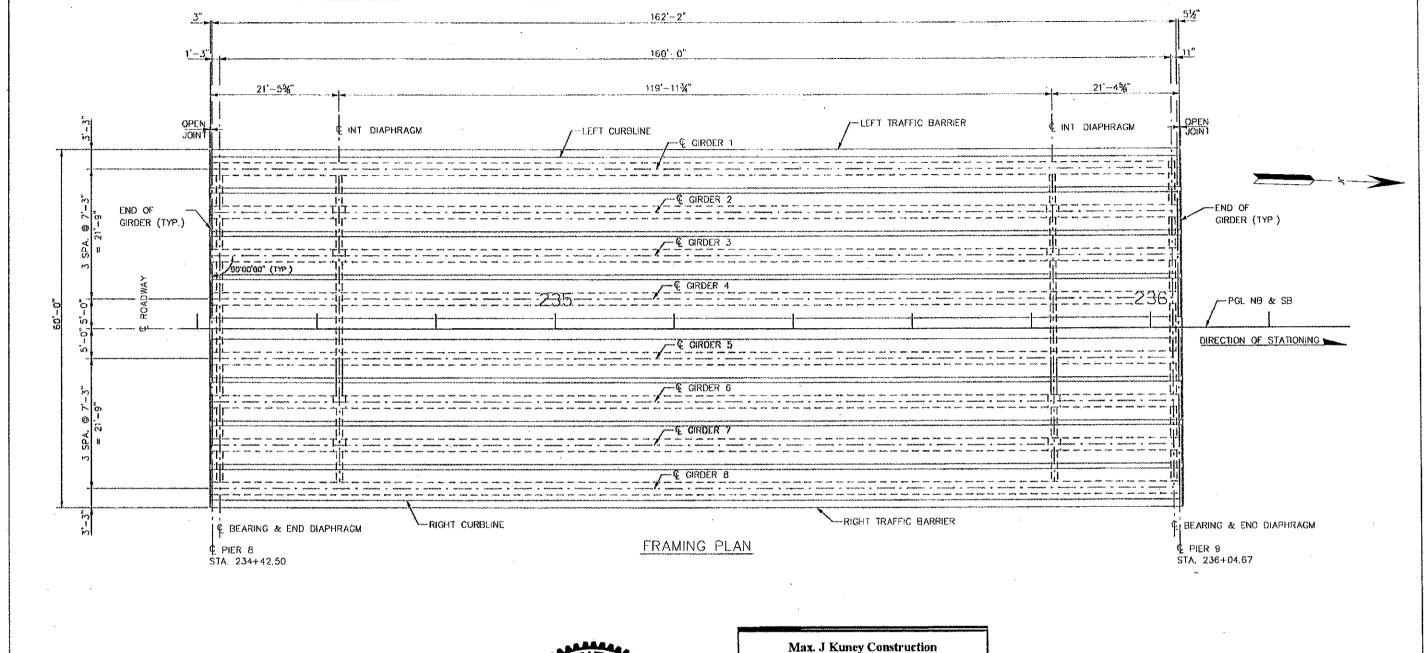


I-5 SKAGIT RIVER BRIDGE **SPAN 8 REPLACEMENT** 

FRAMING PLAN

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### NOTES:

- 1 FOR SUPERSTRUCTURE SECTION, SEE SHEET P-5. 2 FOR EXPANSION JOINT DATA, SEE SHEET P-19. 3. FOR DIAPHRAGM DETAILS, SEE SHEET P-14 THRU P-15

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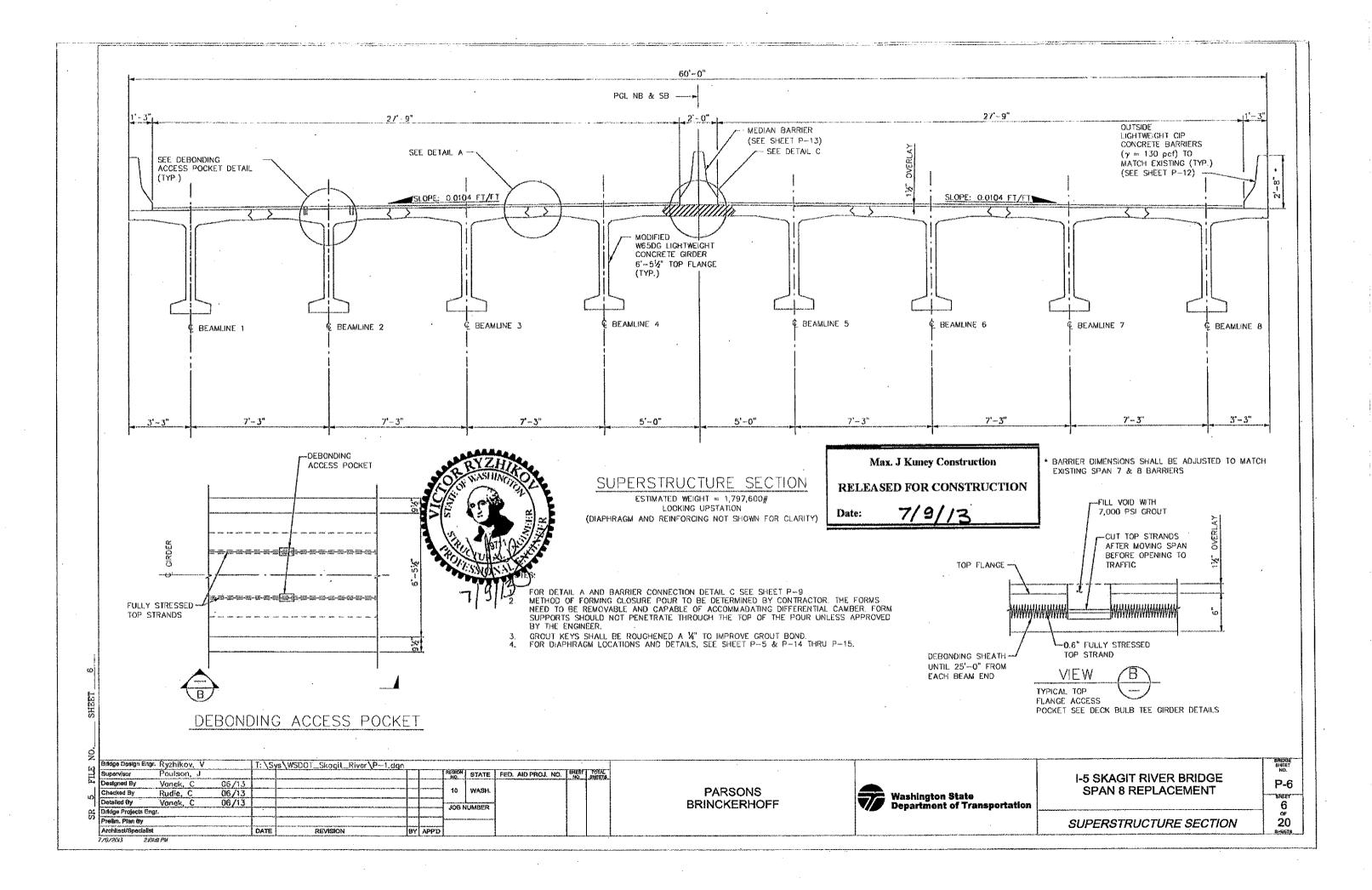
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I-5 SKAGIT RIVER BRIDGE SPAN 8 REPLACEMENT

FRAMING PLAN SPAN 8

5 20



\* THE COMPUTED GIRDER CAMBER AT MIDSPAN IMMEDIATELY BEFORE TIME OF OVERLAY CONCRETE PLACEMENT IS ADJUSTED FOR THE FORM CAMBERING SHOWN ON SHEET P-10 OF THE DECK BULB TEE GIRDER SHEETS

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# GIRDER NOTES

- PLAN LENGTH SHALL BE INCREASED AS NECESSARY TO COMPENSATE FOR SHORTENING DUE TO PRESTRESS AND SHRINKAGE.
- ALL PRETENSIONED AND TEMPORARY STRANDS SHALL BE 0.6"Ø AASHTO M203 GRADE 270 LOW RELAXATION STRANDS, TENSIONED TO 202.5 KSI.
- 3. FOR END TYPES A, C AND D CUT ALL STRANDS FLUSH WITH THE GIRDER ENDS AND PAINT WITH AN APPROVED EPOXY RESIN, EXCEPT FOR EXTENDED STRANDS AS SHOWN. FOR END TYPE B CUT ALL STRANDS 1" BELOW CONCRETE SURFACE AND GROUT WITH AN APPROVED EPOXY GROUT.
- 4. THE TOP SURFACE OF THE GIRDER FLANGE SHALL, BE ROUGHENED IN ACCORDANCE WITH SECTION 6-02.3(25)H OF THE STANDARD SPECIFICATIONS
- LIFTING EMBEDMENTS SHALL BE INSTALLED IN ACCORDANCE WITH SECTION 6-02.3(25)L OF THE STANDARD SPECIFICATIONS.
- 6 CAUTION SHALL BE EXERCISED IN HANDLING AND PLACING GIRDERS ALL GIRDERS SHALL BE CHECKED BY THE CONTRACTOR TO ENSURE THAT THEY ARE BRACED ADEQUATELY TO PREVENT TIPPING AND TO CONTROL LATERAL BENDING DURING SHIPPING. ONCE ERECTED, ALL GIRDERS SHALL BE BRACED LATERALLY TO PREVENT TIPPING UNTIL THE DIAPHRAGMS ARE CAST AND CURED.
- 7. FORMS FOR BEARING PAD RECESSES SHALL BE CONSTRUCTED AND FASTENED IN SUCH A MANNER AS TO NOT CAUSE DAMAGE TO THE GIRDER DURING THE STRAND RELEASE OPERATION.
- 8 ALL REINFORCING STEEL CONFORM TO ASTM A709, GRADE 60 EPOXY COATED BARS SHALL CONFORM TO AASHTO M284 WITH ADDITIONAL MODIFICATION PER 9-07 3 PER THE STANDARD SPECFICIATIONS. HEADED DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A970 INCLUDING ANNEX A1 REQUIREMENTS FOR CLASS HA HEAD DIMENSIONS.
- 9, STUDS SHALL BE MADE FROM COLD DRAWN BAR STOCK CONFORMING TO THE REQUIREMENTS OF ASTM A108, GRADE 1015, 1017, OR 1020, EITHER SEMI-KILLED OR KILLED ALUMINUM OR SILICON DEOXIDATION WITH MECHANICAL PROPERTIES IN CONFORMANCE WITH ASTM A370. FABRICATION SHALL BE PERFORMED IN ACCORDANCE WITH THE CURRENT APPLICABLE EDITION OF AWS D1.1 STRUCTURAL WELDING CODE.

- 10. TEMPORARY TOP STRANDS SHALL BE EITHER PRETENSIONED OR POST—TENSIONED IN ACCORDANCE WITH SECTION 6-02.3(25)L OF THE STANDARD SPECIFICATIONS AND THE GIRDER DETAILS SHEETS. THE LIFTING LOCATION L AND CONCRETE RELEASE STRENGTH F'CI SHOWN IN THE GIRDER SCHEDULE ASSUME THAT THE TEMPORARY TOP STRANDS ARE PRETENSIONED. ALTERNATIVELY, POST—TENSIONED TEMPORARY TOP STRANDS MAY BE USED IF THE LIFTING POINTS IN THE GIRDER SCHEDULE ARE MAINTAINED AND THE STRANDS ARE STRESSED PRIOR TO LIFTING THE GIRDER FROM THE FORM.
- 11. FOR DIAPHRAGMS, PLACE HOLES ON THE FACE OF THE GIRDERS AS SHOWN IN THE DIAPHRAGM DETAILS SHEETS. FOR THE EXTERIOR FACE OF THE WEB ON THE EXTERIOR GIRDER PLACE INSERTS ON THE INTERIOR FACE OF THE EXTERIOR WEB. PLACE HOLES AND INSERTS PARALLEL TO SKEW. INSERTS SHALL BE %" or %"Ø MEADOWBURKE MX-3 HI-TENSILE, %" or %"Ø x 5%" WILLIAMS F22 OPEN FERRULE INSERT, %" or %"Ø x 4%" DAYTON-SUPERIOR F-62 FLARED. THIN SLAB FERRULE INSERT OR APPROVED EQUAL.
- 12. DEFORMED WELDED WIRE REINFORCEMENT CONFORMING TO SECTION 9-07.7 WITH DEFORMED WIRE CONFORMING TO SECTION 9-07.8 MAY BE SUBSTITUTED FOR MILD STEEL REINFORCEMENT IF AASHTO LRFD BRIDGE DESIGN SPECIFICATION REQUIREMENTS (INCLUDING DEVELOPMENT AND ANCHORAGE) ARE MET WELDED WIRE REINFORCEMENT SHALL HAVE THE SAME AREA AND SPACING AS THE MILD STEEL REINFORCEMENT THAT IT REPLACES AND THE MIELD STRENGTH SHALL BE GREATER THAN OR EQUAL TO 60 KSI. SHEAR STIRRUP LONGITUDINAL WIRES AND TACK WELDS SHALL BE EXCLUDED FROM GIRDER WEBS, LONGITUDINAL WIRES FOR ANCHORAGE OF WELDED WIRE REINFORCEMENT SHALL HAVE AN AREA OF 40% OR MORE OF THE AREA OF THE WIRE BEING ANCHORED BUT SHALL NOT BE LESS THAN D4.
- 13. STRUCTURAL STEEL SHAPES AND ASSEMBLIES SHALL BE ASTM A36. THEY SHOULD BE GALVANIZED IN ACCORDANCE WITH ASTM A123.
- 14 CONCRETE SHALL BE CLASS 9000 UTILIZING A LIGHTWEIGHT AGGREGATE CONCRETE MIX IN ACCORDANCE WITH AASHTO T-121, THE COARSE AGGREGATE SHALL CONFORM TO SECTION 9.03:1(4) OF THE STANDARD SPECFICATIONS, EXCEPT GRADING IN CONFORMANCE WITH AASHTO T-195.

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I-5 SKAGIT RIVER BRIDGE SPAN 8 REPLACEMENT

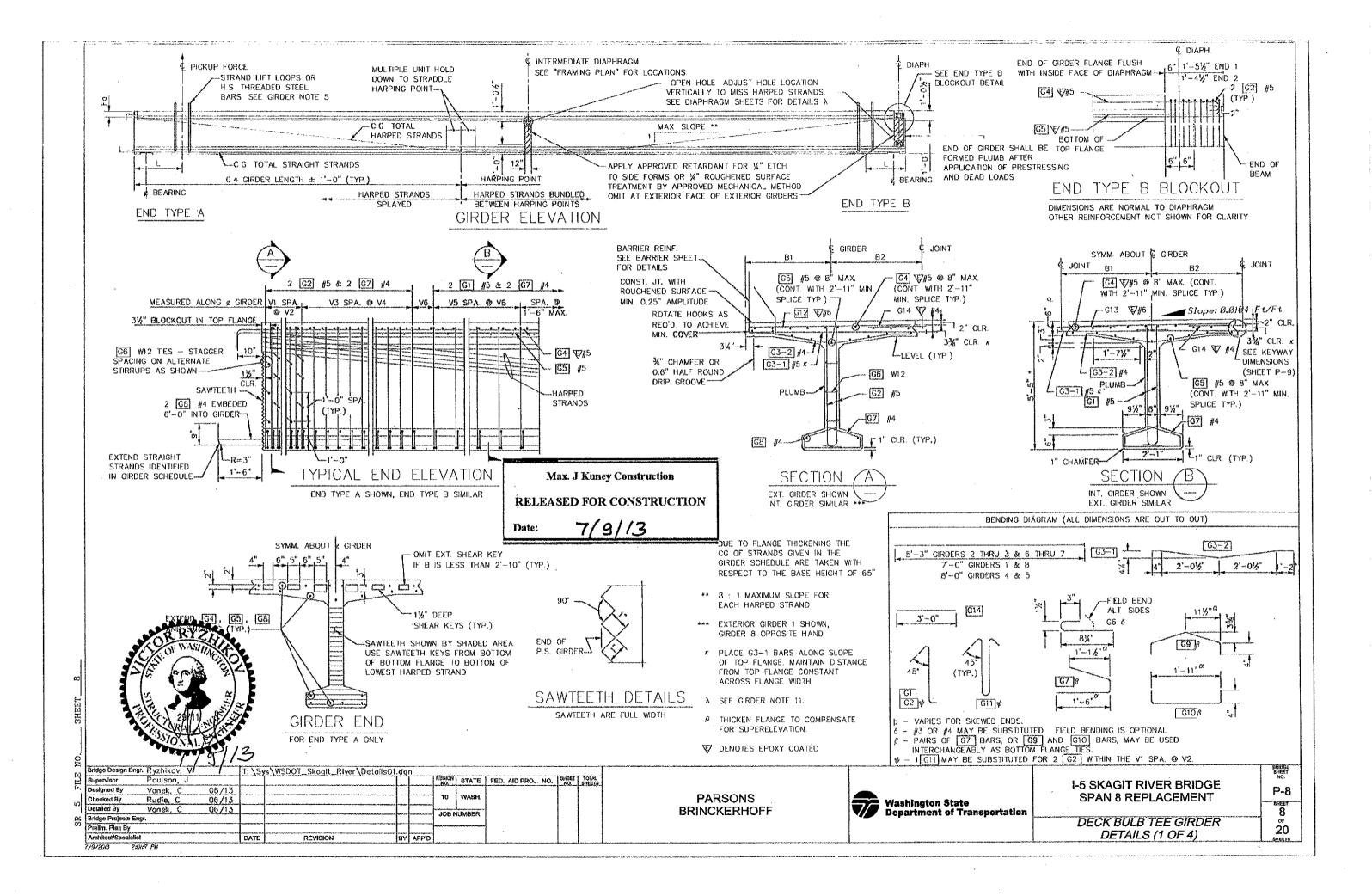
GIRDER SCHEDULE AND NOTES

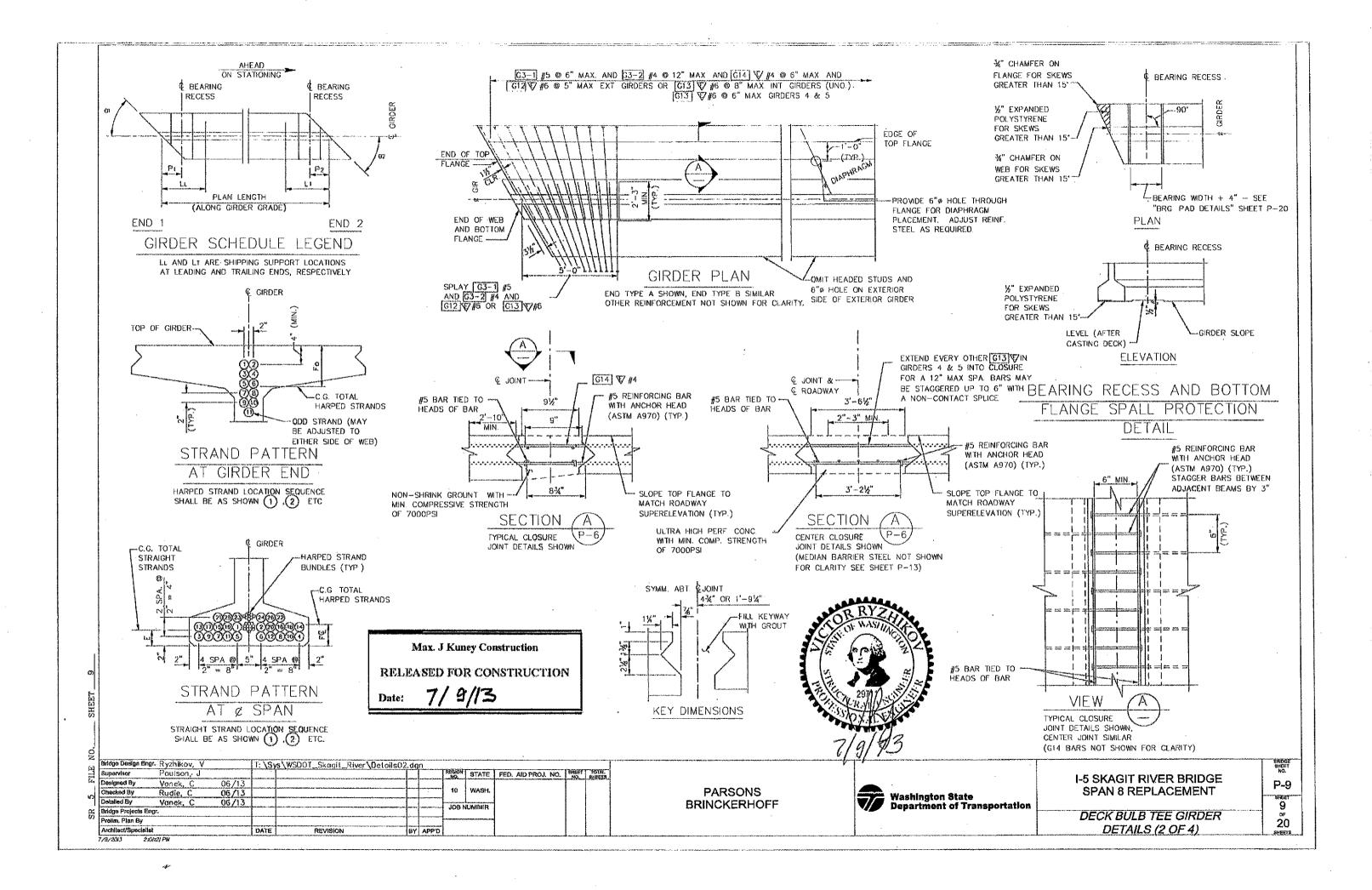
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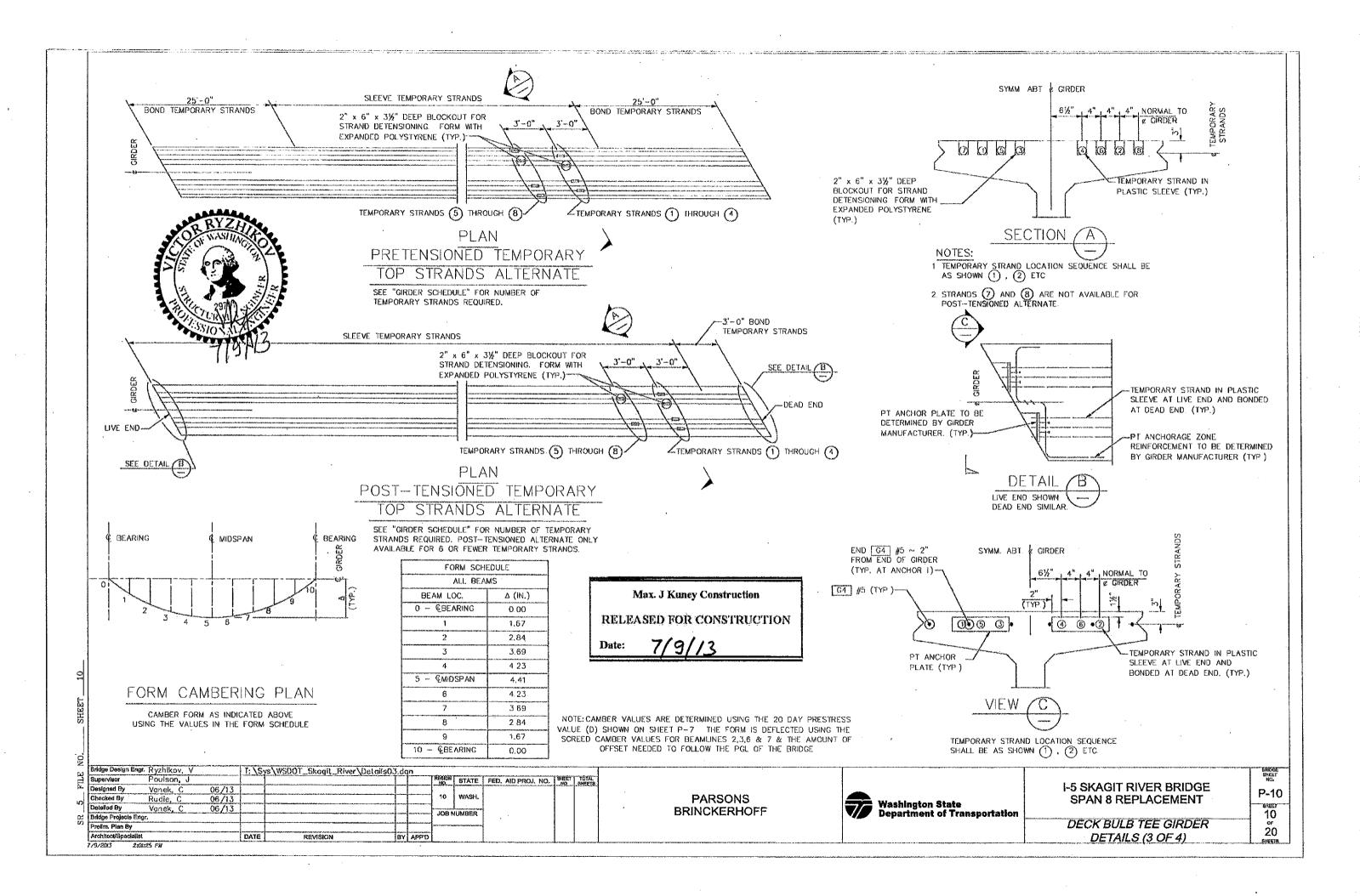
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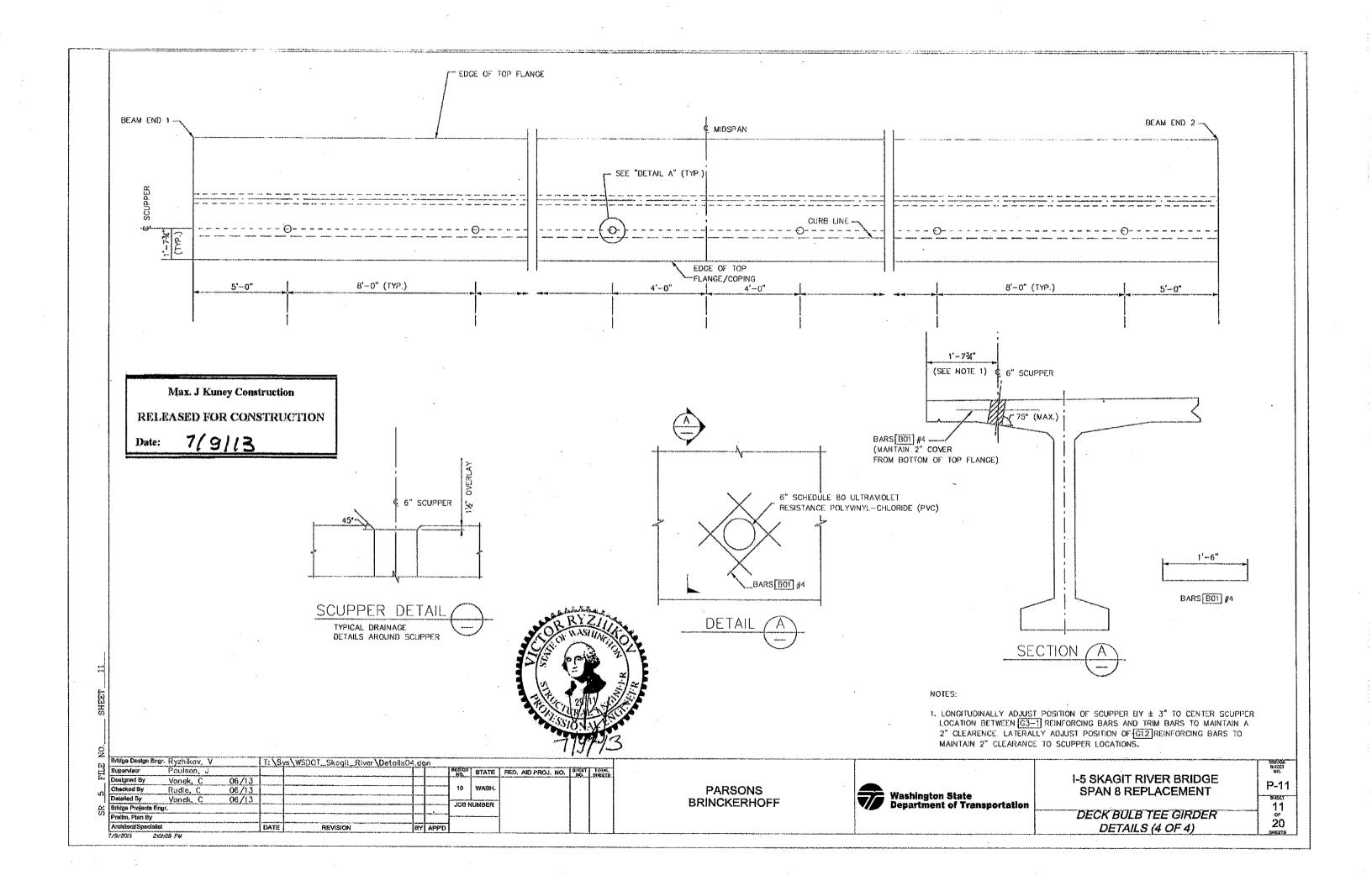
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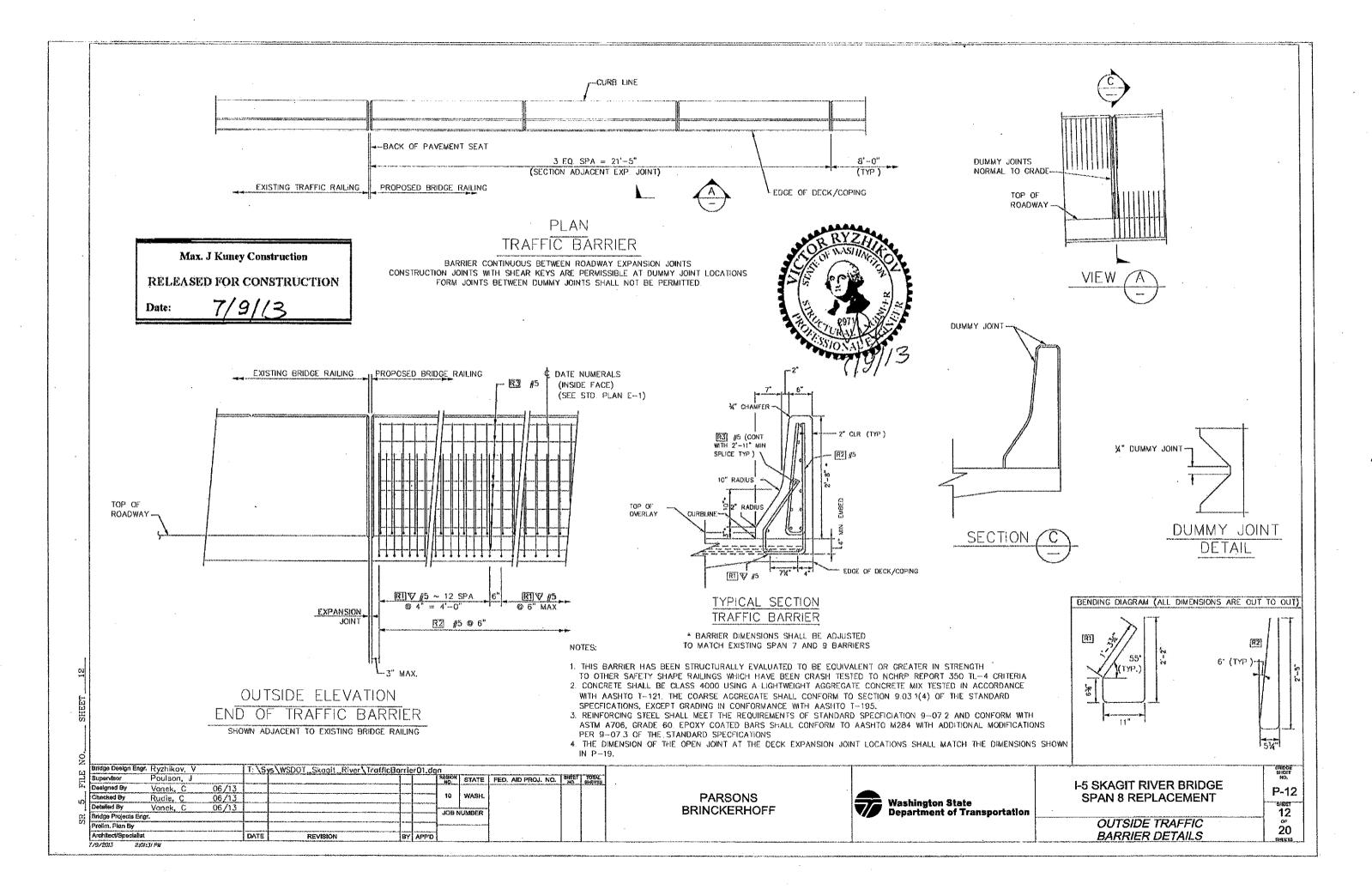
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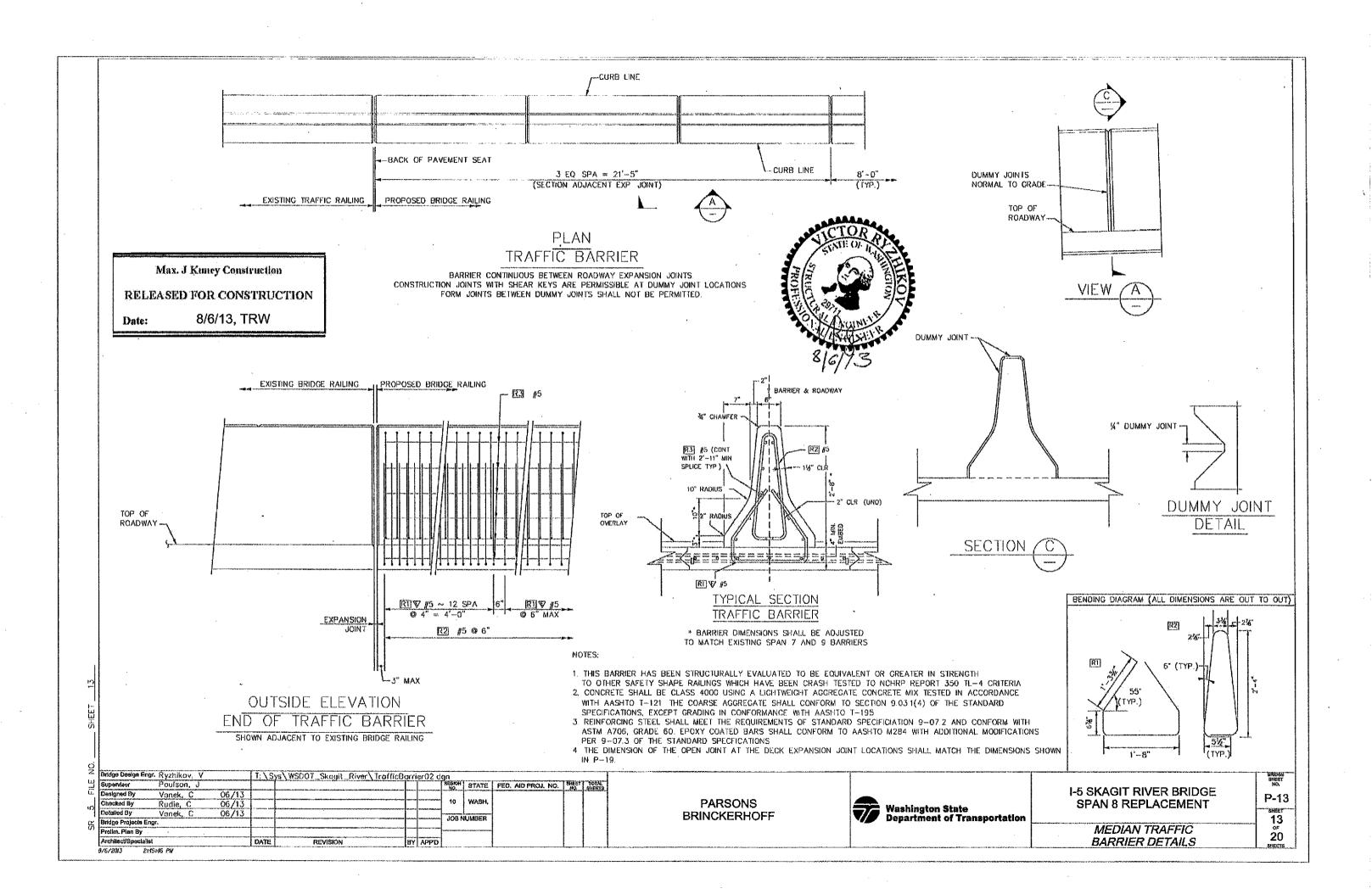


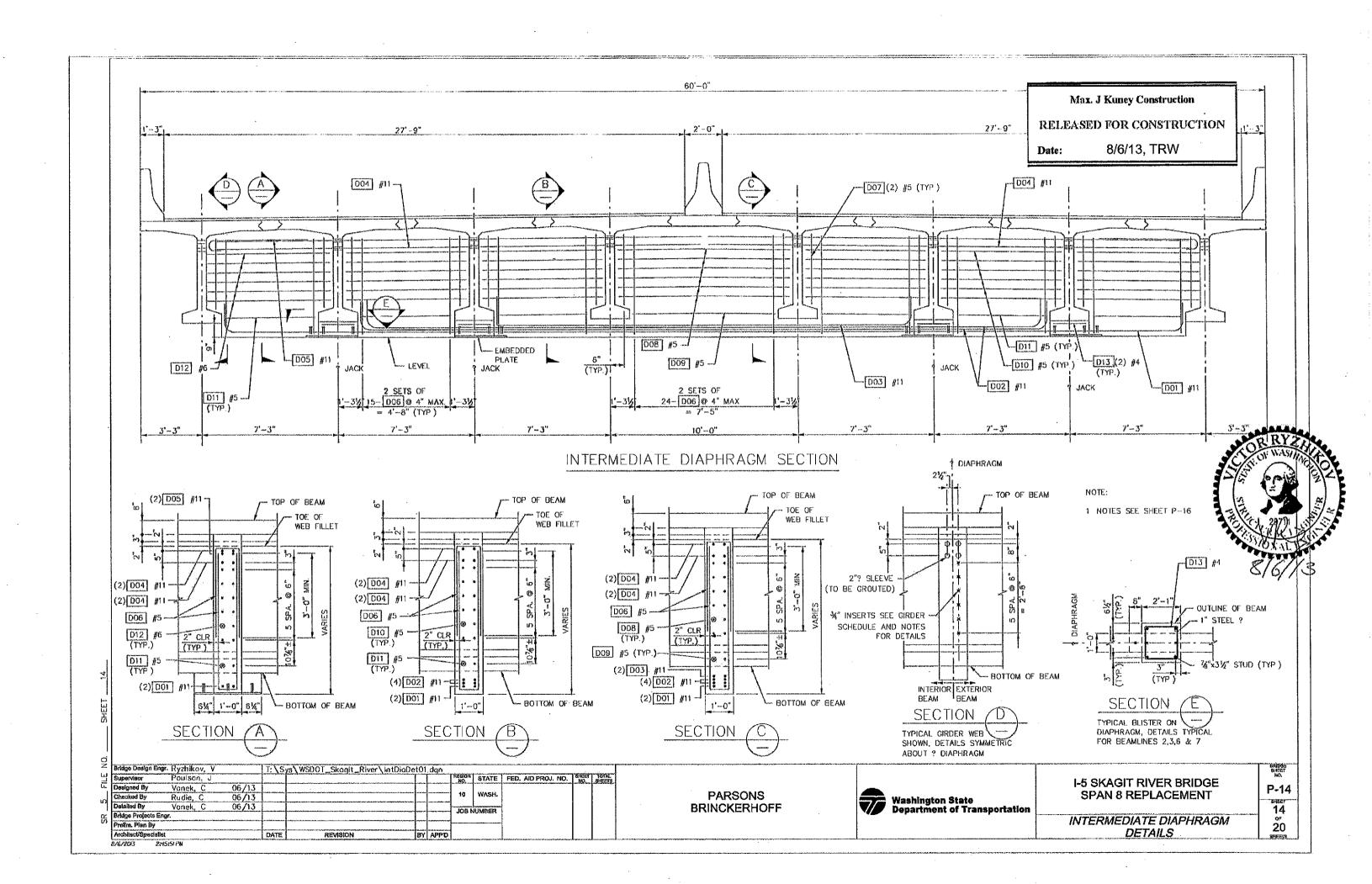


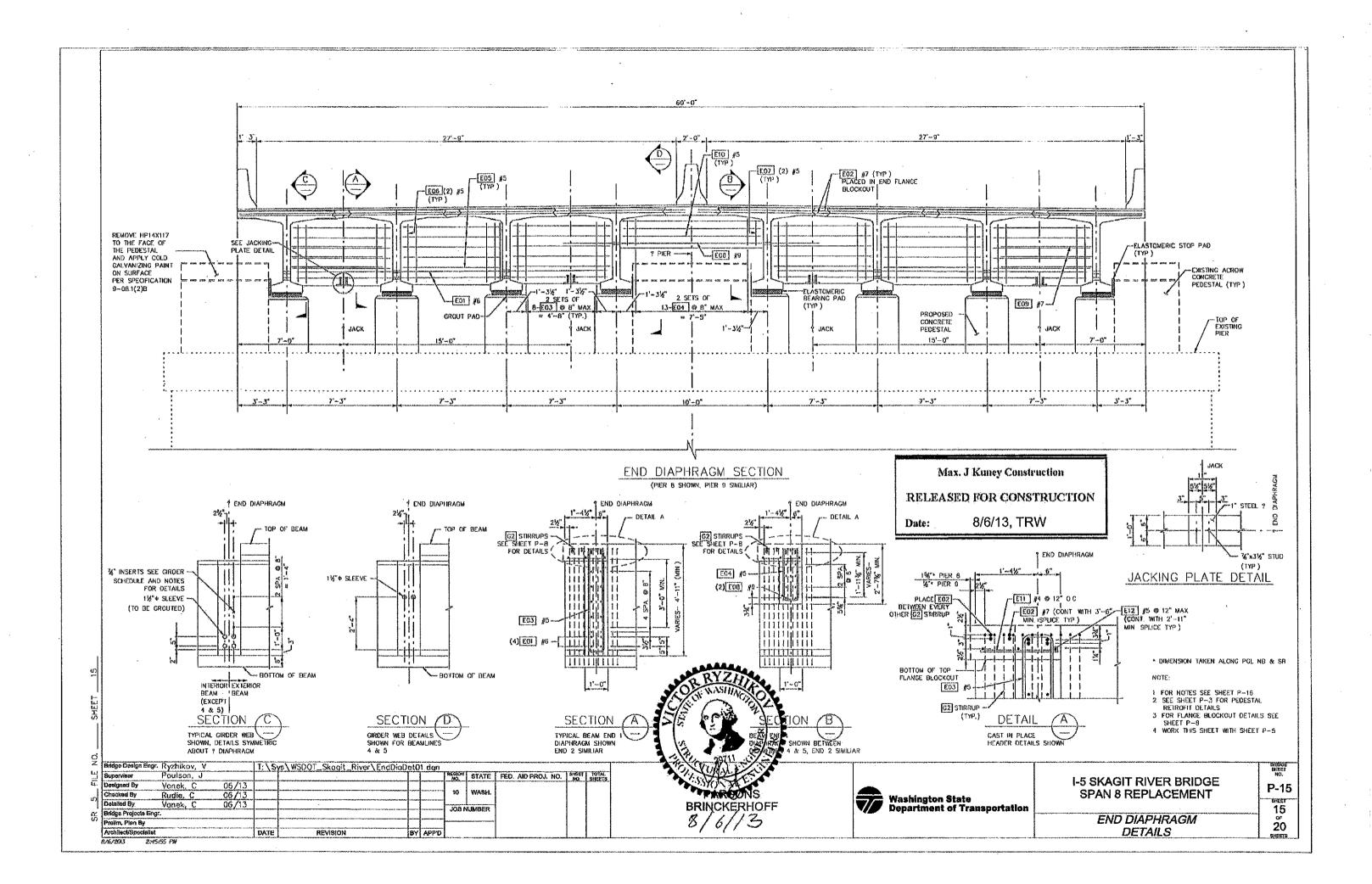












#### NOTES:

- 1 FOR DIAPHRAGM LOCATIONS SEE FRAMING PLAN SHEET P-5.
- 2. WORK THIS SHEET WITH SHEET P-15 & P-16.
- 3 CONCRETE SHALL BE CLASS 4000 USING A LIGHTWEIGHT AGGREGATE CONCRETE MIX TESTED IN ACCORDANCE WITH AASHTO T-121 THE COARSE AGGREGATE SHALL CONFORM TO SECTION 9 03 1(4) OF THE STANDARD SPECFICATIONS, EXCEPT GRADING IN CONFORMANCE WITH AASHTO T-195
- 4. REINFORCING STEEL SHALL CONFORM WITH THE REQUIREMENTS OF STANDARD SPECIFICAIONS 9-07.2 AND CONFORM WITH ASTM A706, GRADE 60,
- 5, STUDS SHALL BE MADE FROM COLD DRAWN BAR STOCK CONFORMING TO THE REQUIREMENTS OF ASTM A108, GRADE 1015. 1017. OR 1020, EITHER SEMI-KILLED OR KILLED ALUMINUM OR SILICON DEOXIDATION WITH MECHANICAL PROPERTIES IN CONFORMANCE WITH ASTM A370, FABRICATION SHALL BE PERFORMED IN ACCORDANCE WITH THE CURRENT APPLICABLE EDITION OF AWS DIJ STRUCTURAL WELDING CODE
- 6 STRUCTURAL STEEL SHALL BE ASTM A36 AND GALVANIZED IN ACCORDANCE WITH ASTM A123
- 7. CARE SHALL BE TAKEN TO PROPERLY CONSOLIDATE DIAPHRAGM CONCRETE UNDER FLANGES OF BEAMS.

Max. J Kuney Construction

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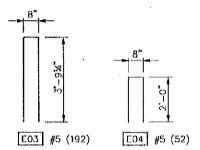
#### END DIAPHRAGM

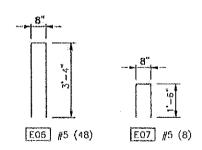
20'-6" - E01 #6 (16) 29'-5" - E02 #7 (32) 6'-5" ~ E05 #5 (120) 20'-0" - E08 #9 (4) \*\* 9'-2" - E10 #5 (12)

> 1'-8" - E11 #4 (122) 59'-8" - [£12] #5 (6) \*\*\*

THREADED ENDS~

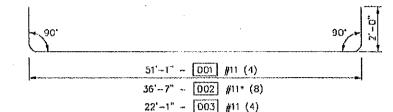
6'-0" - E09 #7 (24)



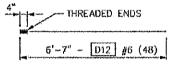


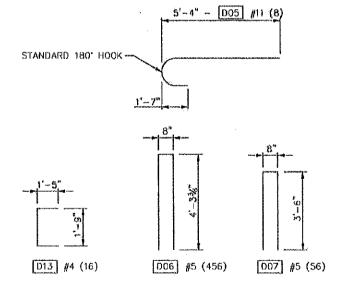
- \*\* #9 E08 BAR MAY BE SPLIT INTO THREE PIECES WITH A MIN. LAP OF 5'-O" NO BAR SPLICES ARE ALLOWED WITH THE MIDDLE THIRD OF THE CENTER DIAPHRAGM SPACING
- \*\*\* FIELD BEN AS NECESSARY OR SPLIT ALL BARS INTO TWO PIECES AND ADD A CENTER LAP BAR WITH MIN. SPLICE LENGTH OF 2'-11".

## INTERMEDIATE DIAPHRACM



25'-11" - 004 #11 (16) 9'-2" - DO8 #5 (24) 7'-7" - 009 #5 (4) 6'-5" - D10 #5 (96) 4'-10" - 011 #5 (24) -- THREADED ENDS





\* PROVIDE (4) AT DO2 DIMENSION SHOWN AND PROVIDE SECOND (4) AT 4" LESS

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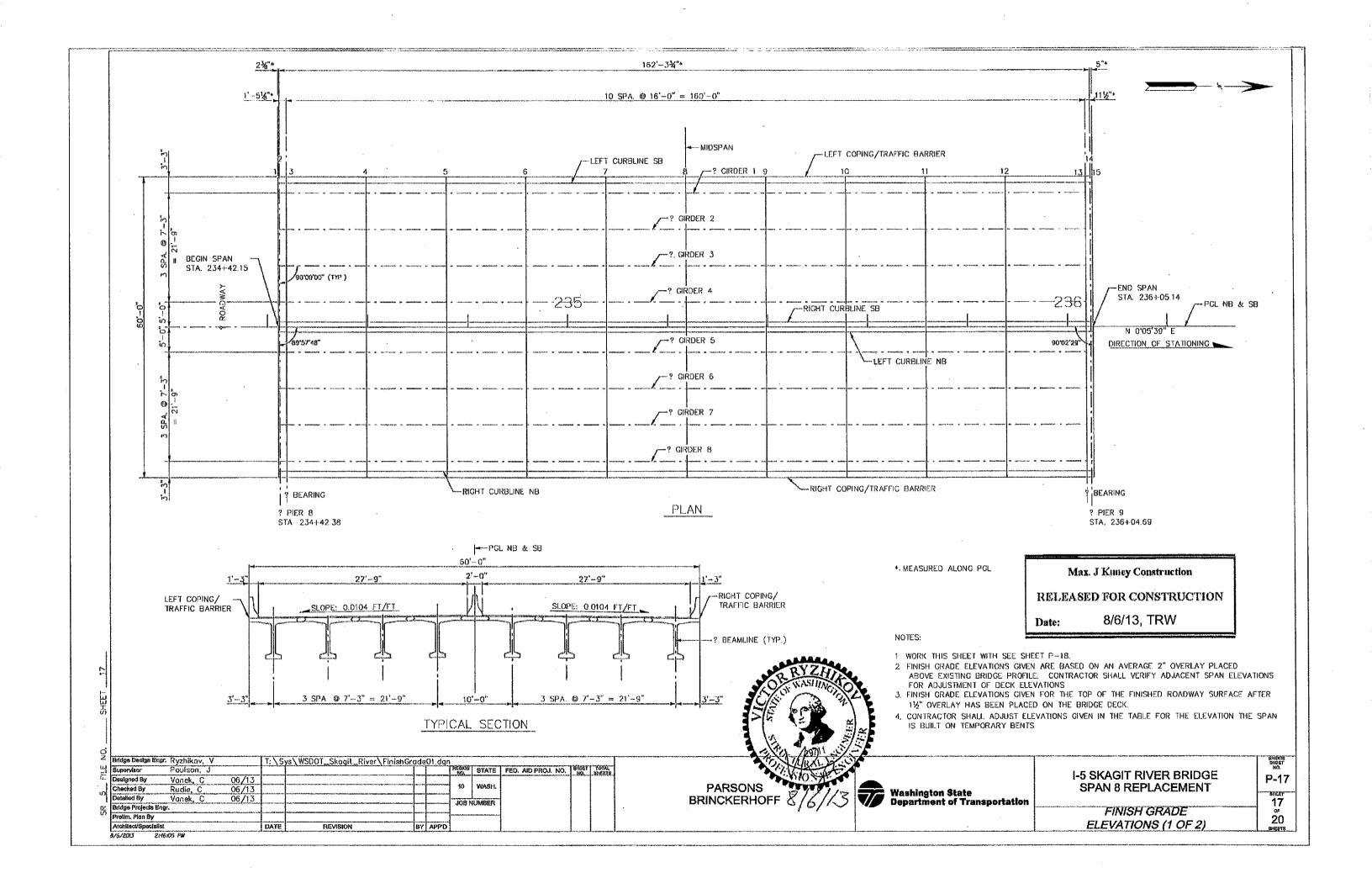


I-5 SKAGIT RIVER BRIDGE **SPAN 8 REPLACEMENT** 

DIAPHRAGM REINFORCING **DETAILS** 

P-16

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					F24-1			SPAN 8	,						g
	1	2	3					8					13	14	15
	BEGIN SPAN	© PIER 8	© BEARING	4	5	6	7	MIDSPAN	g	10	11	12	© BEARING	C PIER 9	END SPAN
STATION	234+42.15	234+42.38	234+43.75	234+59.75	234+75.75	234+91.75	235+07.75	235+23.75	235+39.75	235+55.75	235+71.75	235+87.75	236+03.75	236+04.69	236+05.14
LEFT COPING/TRAFFIC BARRIER	64.938	64,937	64.928	64,825	64,713	64.590	64.459	64.317	64.166	64,005	63.835	63.655	63,466	63.454	63,449
LEFT CURBLINE (SB)	64.951	64.950	64,941	64,838	64.726	64.603	64,472	64,330	64,179	64.018	63.848	63.668	63.479	63.467	63.462
© BEAMLINE 1	64.972	64.971	64.962	64.859	64,747	64.624	64.492	64.351	64,200	64.039	63.869	63.689	63,500	63,488	63.483
C BEAMLINE 2	65,048	65.046	65.038	64.935	64,822	64,700	64.568	64.427	64,275	64.115	63,945	63.765	63,575	63,564	63.558
C BEAMLINE 3	65,123	65,122	65.113	65.010	64.898	64,775	64.643	64.502	64.351	64,190	64.020	63,840	63.651	63,639	63.634
C BEAMLINE 4	65,199	65.197	65,189	65.086	64,973	64.851	64,719	64.578	64.427	64.266	64.096	63.916	63.726	63,715	63.709
RIGHT CURBLINE (SB)	65.240	65.239	65.230	65.127	65.015	64.893	64,761	64.619	64.468	64,308	64.137	63.957	63,768	63,757	63.751
PGL NB & SB	65.251	65.249	65,241	65.138	65.025	64.903	64.771	64.630	64.479	64.318	64,148	63.968	63.778	63,767	63.761
LEFT CURBLINE (NB)	65,240	65.239	65,230	65.127	65.015	64.893	64,761	64,619	64.468	64.308	64.137	63.957	63.768	63.757	63.751
C BEAMLINE 5	65,199	65.197	65.189	65.086	64.973	64.851	64,719	64.578	64,427	64,266	64.096	63.916	63.726	63.715	63,709
C BEAMLINE 6	65.123	65.122	65.113	65.010	64,898	64,775	64.643	64,502	64,351	64,190	64.020	63.840	63.651	63,639	63.634
C BEAMLINE 7	65.048	65.046	65.038	64,935	64,822	64.700	64.568	64.427	64,275	64.115	63,945	63,765	63.575	63.564	63,558
C BEAMLINE 8	64,972	64.971	64,962	64.859	64.747	64.624	64,492	64,351	64.200	64.039	63,869	63.689	63,500	63.488	63.483
RIGHT CURBLINE (NB)	64.951	64,950	64.941	64,838	64.726	64,603	64.472	64.330	64,179	64,018	63,848	63,668	63,479	63,467	63,462
RIGHT COPING/TRAFFIC BARRIER	64,938	64.937	64.928	64.825	64,713	64,590	64,459	64.317	64.166	64.005	63.835	63.655	63,466	63,454	63,449

Max. J Kuney Construction

RELEASED FOR CONSTRUCTION

8/6/13, TRW



## NOTES:

- 1. FOR SUPERSTRUCTURE SECTION, SEE SHEET P-6.
  2. FOR EXPANSION JOINT DATA, SEE SHEET P-19.
  3. FOR DIAPHRAGM DETAILS, SEE SHEET P-14 THRU P-15.
  4. WORK THIS SHEET WITH SHEET P-17

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<u></u>	Supervisor	Poulson, J					T		REGION I	STATE	FED.	AID PROJ.	NO.	KHEET	YOYAL UNKETS	į
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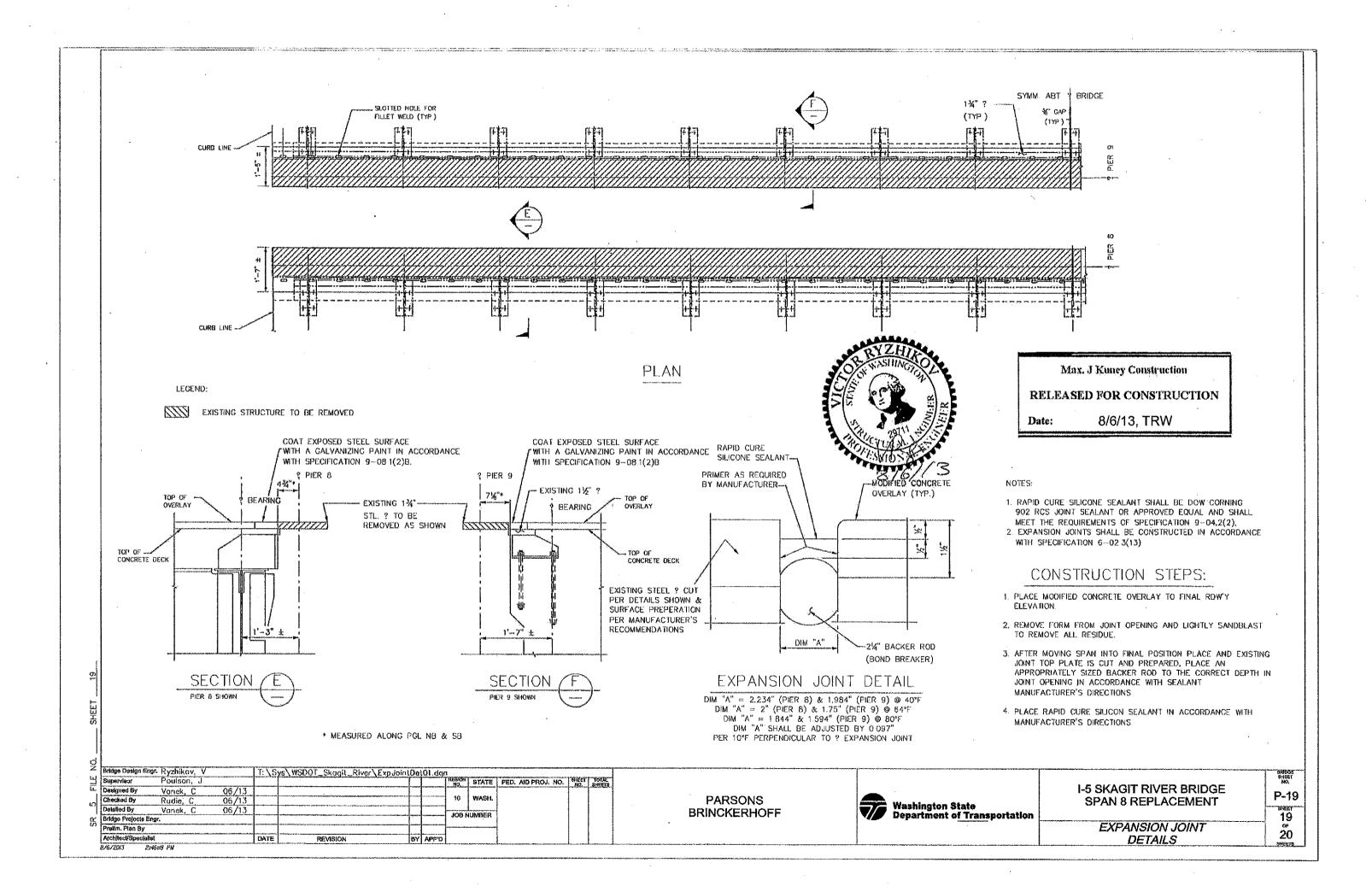
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I-5 SKAGIT RIVER BRIDGE SPAN 8 REPLACEMENT

FINISH GRADE ELEVATIONS (2 OF 2)

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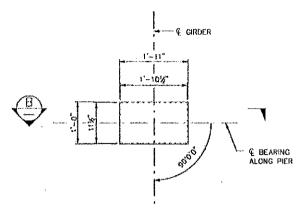


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BEARING DESIGN TAB	LE
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SERVICE - I LIMIT STATE	
DEAD LOAD (DL) REACTION	120 9 KIPS
LIVE LOAD REACTION (W/O IMPACT)	121 3 KIPS
UNLOADED HEIGHT	3.45 IN
LOADED HEIGHT (DL)	3 39 IN
SHEAR MODULUS	165 PSI

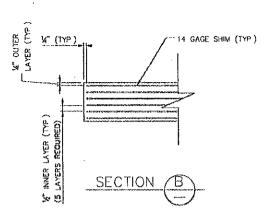
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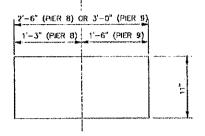
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BEARING PAD DETAIL

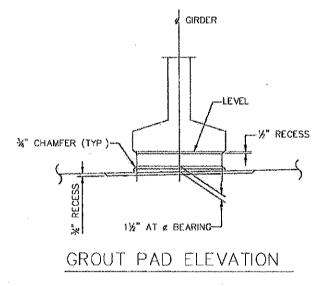


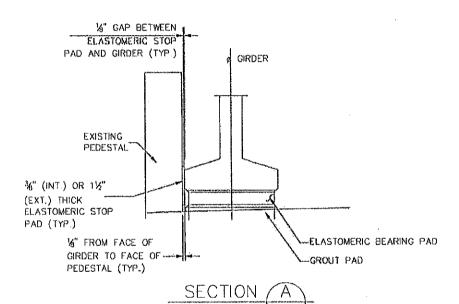
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ELASTOMERIC STOP PAD

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Washington State
Department of Transportation

I-5 SKAGIT RIVER BRIDGE SPAN 8 REPLACEMENT

**BEARING PAD** DETAILS

P-20

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	Supervisor	Poulson, J							REGION	STATE	FED.	AID PROJ.	NO.	SHEET NO.	YOYAL
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