

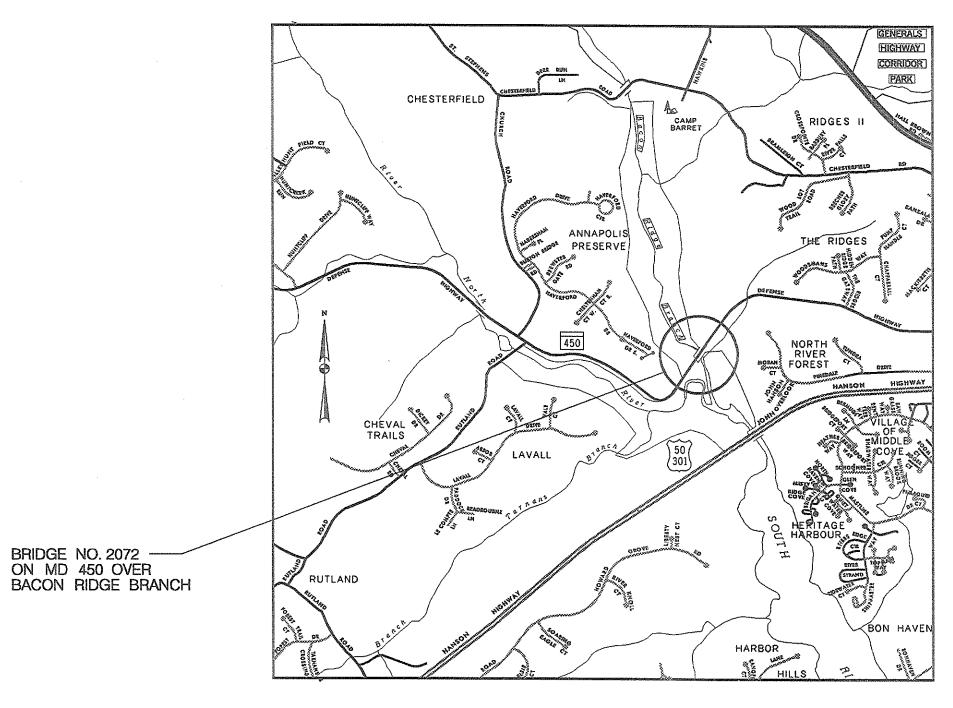
Maryland Department of Transportation

STATE HIGHWAY ADMINISTRATION

PLANS FOR REPLACEMENT OF TWO BRIDGES BRIDGE NO. 10016 ON MARYLAND ROUTE 28 OVER WASHINGTON RUN

OVER WASHINGTON RUN
AND
BRIDGE NO. 02072 ON MARYLAND ROUTE 450
OVER BACON RIDGE BRANCH

S.H.A. CONTRACT NO. AX4695180 F.A.P. CONTRACT NO. AC-BR-HFL-000A(273)E



ANNE ARUNDEL COUNTY

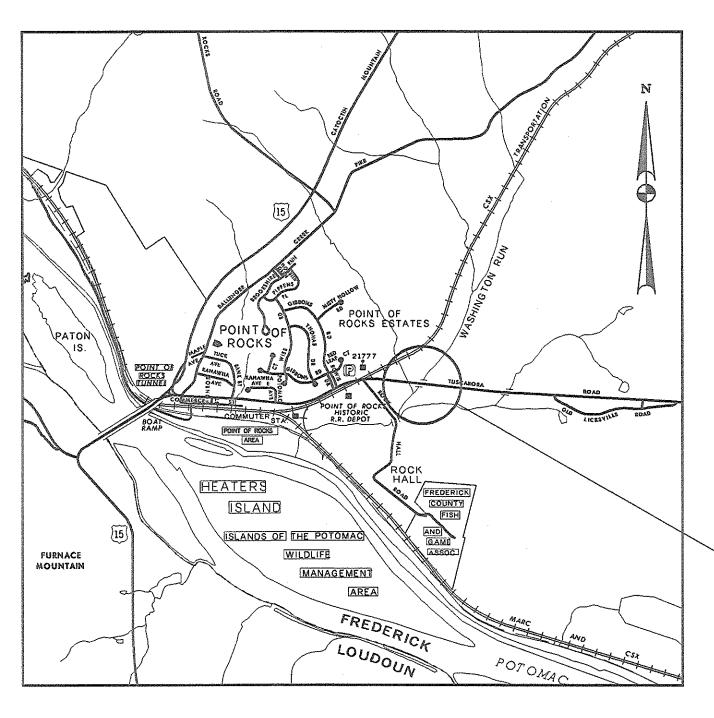
LOCATION MAP

SCALE: 1" = 2000'

PROJECT LENGTH = 0.260 MILES

### MD 450 <u>DESIGN TRAFFIC DATA</u>

|                          | 2006_     | 2026(EST.) |
|--------------------------|-----------|------------|
| A.D.T.                   | 8,000     | 9,900      |
| D.H.V.                   | 11 %      | 11 %       |
| DIRECTIONAL DISTRIBUTION | 56 %      | 56 %       |
| PERCENT TRUCKS-A.D.T.    | 5 %       | 5 %        |
| PERCENT TRUCKS-D.H.V.    | 4 %       | 4 %        |
| DESIGN SPEED             | 50 M.P.H. | 50 M.P.H.  |



FREDERICK COUNTY

LOCATION MAP

SCALE: 1" = 2000'

PROJECT LENGTH = 0.067 MILES

### MD 28 <u>DESIGN TRAFFIC DATA</u>

|                          | 2005      | 2025(EST.) |
|--------------------------|-----------|------------|
| A.D.T.                   | 3,250     | 6,300      |
| D.H.V.                   | 11 %      | 11 %       |
| DIRECTIONAL DISTRIBUTION | 74 %      | 74 %       |
| PERCENT TRUCKS-A.D.T.    | 7 %       | 7 %        |
| PERCENT TRUCKS-D.H.V.    | 4 %       | 4 %        |
| DESIGN SPEED             | 50 M.P.H. | 50 M.P.H.  |

## ADDENDUMS AND RED LINE REVISIONS

[] RED LINE NO.1 4/9/08

BRIDGE 10016: SHEET MOS. 12 TO 15,53A.

BRIDGE 02072: SHEET MOS. 4,7,8, 14,16, 17, 18,19.

20, 21, 22, 42, 800 63.

[2] RED LINE NO. 2 SHEETS NO. 1, 4, 4.01, 6, 15 - 17, 19, 20 6/26/08

REVIEWED AND APPROVAL RECOMMENDED

CHIEF, BRIDGE DESIGN DIVISION

APPROVAL RECOMMENDED

APPROVED

DEPUTY ADMINISTRATOR FOR PLANNING AND ENGINEERING

- BRIDGE NO. 10016 ON MARYLAND ROUTE 28 OVER WASHINGTON RUN

### INDEX OF SHEETS 1. TITLE SHEET 2. TYPICAL SECTIONS 3. TYPICAL SECTIONS 4. GEOMETRIC LAYOUT 5. ROADWAY PLAN 6. ROADWAY PROFILE 7. ROADWAY PROFILE 8. DETOUR PLAN 9. DETOUR DETAILS 10. DETOUR DETAILS 11. SIGN LEGEND 12. EROSION AND SEDIMENT CONTROL NOTES 13. EROSION AND SEDIMENT CONTROL PHASE I 14. EROSION AND SEDIMENT CONTROL PHASE II 15. EROSION AND SEDIMENT CONTROL PHASE III 16. MAINTENANCE OF STREAM FLOW - DETAILS 17. GENERAL NOTES AND PROPOSALS 18. SIGNING AND PAVEMENT MARKING PLAN 19. GRADING TABLE 20. GENERAL PLAN AND ELEVATION 21. HYDRAULIC AND HYDROLOGIC DATA 22. GEOMETRIC DATA 23. ABUTMENT A - PLAN AND ELEVATION 24. ABUTMENT A - PILE PLAN 25. ABUTMENT A - FOOTING REINFORCING 26. ABUTMENT B - PLAN AND ELEVATION 27. ABUTMENT B - PILE PLAN 28. ABUTMENT B - FOOTING REINFORCING 29. ABUTMENT DETAILS 30. ABUTMENT PILE PLACEMENT DETAILS 31. ABUTMENT DETAILS 32. WING WALL ELEVATIONS 33. WING WALL DETAILS 34. WING WALL DETAILS 35, RIPRAP SCOUR PROTECTION 36. RIPRAP SCOUR PROTECTION DETAILS 37. TYPICAL SECTION 38. SLAB LAYOUT 39. 4'-0" EXTERIOR SLAB NO. 1 DETAILS 40.4'-0" INTERIOR SLAB DETAILS 41. 4'-0" EXTERIOR SLAB NO. 10 DETAILS 42. SUPERSTRUCTURE DETAILS 43. SUPERSTRUCTURE DETAILS 44. FINISHED DECK ELEVATIONS 45. RAIL POST SPACING DETAILS 46. RAILING DETAILS 47. DETAILS 48. DETAILS 49. REINFORCING DETAILS 50. REINFORCING DETAILS 51. REINFORCING DETAILS 52. REINFORCING DETAILS 53. FOUNDATION TEST BORINGS 53A. LANDSCAPE PLAN XS-01 ROADWAY CROSS SECTION SHEET XS-02 ROADWAY CROSS SECTION SHEET XS-03 ROADWAY CROSS SECTION SHEET XS-04 ROADWAY CROSS SECTION SHEET FOR THE CONVENIENCE AND INFORMATION OF BIDDERS PRINTS OF PLANS OF EXISTING PERTINENT STRUCTURE(S) ARE INCLUDED WITH THIS CONTRACT. NO RESPONSIBILITY FOR THEIR ACCURACY OR COMPLETENESS IS ASSUMED BY THE STATE HIGHWAY ADMINISTRATION. DIMENSIONS, DETAILS, ETC., AS SHOWN THEREON MAY NOT BE AS BUILT.

INCLUDED FOR YOUR USE ARE: THERE ARE NO PLANS AVAILABLE FOR THE EXISTING BRIDGE.

# STATE HIGHWAY ADMINISTRATION

PLANS FOR REPLACEMENT OF BRIDGE NO. 10016 ON MARYLAND ROUTE 28 OVER WASHINGTON RUN PORTION OF PROJECT

S.H.A. CONTRACT NO. AX4695180 F.A.P. CONTRACT NO. AC-BR-HFL-000A(273)E

MD 28 SURVEY BOOK NO. 13837 HORIZONTAL DATUM: NAD 83/91

VERTICAL DATUM: NAVD 88

RIGHT-OF-WAY PLAY NOS. MD 28 - 57364

SEDIMENT AND EROSION CONTROL REGULATIONS WILL BE STRICTLY ENFORCED DURING CONSTRUCTION.

**ENVIRONMENTAL INFORMATION:** MD 28 MDE PROJECT NO. 08-SF-0023

ALL STORMWATER MANAGEMENT FACILITIES CONSTRUCTED FOR CONTRACT NO. AX4695180 SHALL BE INSPECTED AND MAINTAINED IN ACCORDANCE WITH THE STATE HIGHWAY ADMINISTRATION'S BEST MANAGEMENT PRACTICES (BMP) INSEPCTION AND REMEDIATION PROGRAM.

OWNERS / DEVELOPERS CERTIFICATION : I/WE HEREBY CERTIFY THAT ANY CLEARING, GRADING, CONSTRUCTION AND/OR DEVELOPMENT WILL BE DONE PURSUANT TO THIS PLAN, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A MARYLAND DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I HEREBY AUTHORIZE THE RIGHT OF ENTRY FOR PERIODIC ON-SITE EVALUATION BY STATE OF MARYLAND, DEPARTMENT OF THE ENVIRONMENT, COMPLIANCE INSPECTORS.

STANDARD STABILIZATION NOTE: FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN SEVEN (7) CALENDAR DAYS AS TO THE SURFACE OF ALL PERIMETER CONTROLS, DIKES, SWALES, DITCHES, PERIMETER SLOPES, AND ALL SLOPES GREATER THAN 3 HORIZONTAL TO 1 VERTICAL (3:1) AND FOURTEEN (14) DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE.

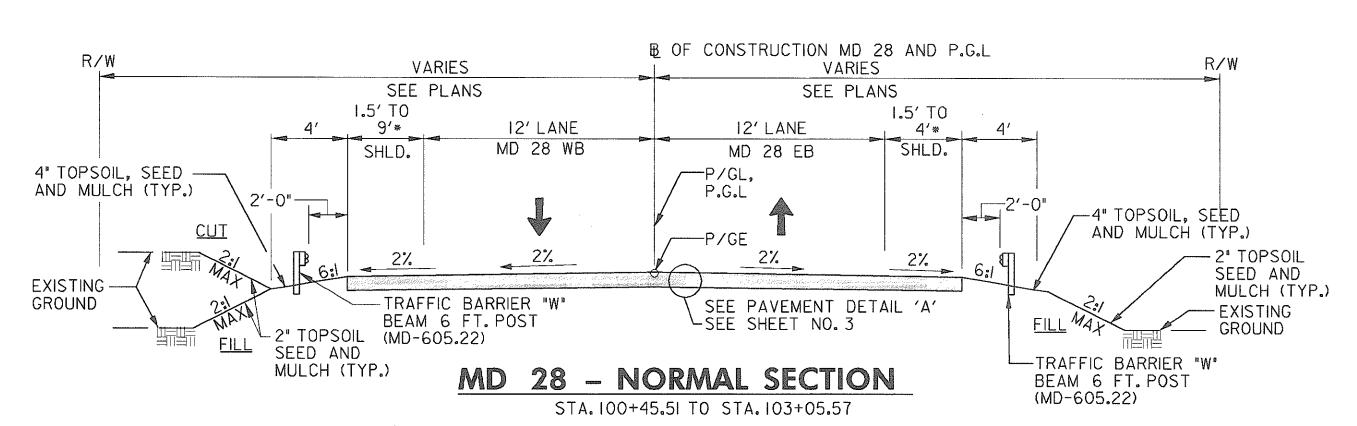
THE STATE HIGHWAY ADMINISTRATION SHALL ONLY BE RESPONSIBLE FOR THE COMPLETENESS OF DOCUMENTS OBTAINED DIRECTLY FROM THE STATE HIGHWAY ADMINISTRATION CASHIER'S OFFICE. FAILURE TO ATTACH ADDENDA MAY CAUSE THE BID TO BE IRREGULAR.

RIGHT-OF-WAY AND EASEMENT LINES SHOWN ON THESE PLANS ARE FOR ASSISTANCE IN INTERPRETING THE PLANS. THESE LINES DO NOT REPRESENT THE OFFICIAL PROPERTY ACQUISITION LINES. FOR OFFICIAL FEE RIGHT OF WAY AND EASEMENT INFORMATION, SEE APPROPRIATE RIGHT OF WAY PLAT.

THE LOCATION OF UTILITIES SHOWN ON THE PLANS ARE FOR INFORMATION AND GUIDANCE ONLY, NO GUARANTEE IS MADE AS TO THE ACCURACY OF SAID LOCATIONS.

### CONVENTIONAL SIGNS

| CONVENTIONAL GIGING         |                 |  | \ \ LB.     |
|-----------------------------|-----------------|--|-------------|
| STATE, COUNTY OR CITY LINES |                 | INTERCEPTOR BERM   | <u> </u>    |
| PROPOSED TRAFFIC BARRIER    |                 |  |             |
| EXISTING TRAFFIC BARRIER    |                 | TEMPORARY SLOPE DRAIN  | T.S.D.      |
| FENCE LINE                  |                 | CHANNEL SILT FENCE   |             |
| RIGHT OF WAY LINE           |                 | SLOPE SILT FENCE   | s.f s.f     |
| EXISTING ROADWAY            |                 | SUPER SILT FENCE   | s.s.fs.s.f  |
| RAILROAD                    |                 | STRAW BALE STRUCTURE   |             |
| BASE OR SURVEY LINE         |                 | PLACED RIPRAP DITCH  |             |
| FIRE HYDRANT                |                 | TEMPORARY STONE OUTLET STRUCTURE   |             |
| PROPOSED CULVERT            | •               |  |             |
| EXISTING CULVERT            | == 1            | GABIONS  |             |
| EXISTING DROP INLET         | :               |  | il I.S.I.   |
| UTILITY POLE                | <del></del> • • | TEMPORARY SEDIMENT TRAP WITH SILT FENCE  |             |
| MARSH                       |                 |  | di T.S.T.   |
| HEDGE                       |                 | TEMPORARY SEDIMENT TRAP WITH STRAW BALES   |             |
|                             | 1 [             |  |             |
| INLET SEDIMENT TRAP         | rts.ī.          | TEMPORARY SEDIMENT TRAP WITH   | View I.S.I. |
|                             | DATUM LINE C    | STONE OUTLET STRUCTURE   |             |
| GROUND ELEVATION            | DATUM LINE C    | TO A DESCRIPTION OF THE PARTY O |             |
|                             | 5.0             | TEMPORARY SEDIMENT TRAP WITH   | Y T.S.T.    |
| GRADE ELEVATION             | DATUM LINE N    | RIPRAP OUTLET STRUCTURE  |             |
|                             |                 |  |             |



PROPOSED PAVEMENT WIDTH TABLE

STATION WB EB

100+00.00 12' WITH 1.5' SHLD. 12' WITH 1.5' SHLD.

100+45.51 12' WITH 1.5' SHLD. 12' WITH 1.5' SHLD.

 101+00.00
 12' WITH 9' SHLD.
 12' WITH 4' SHLD.

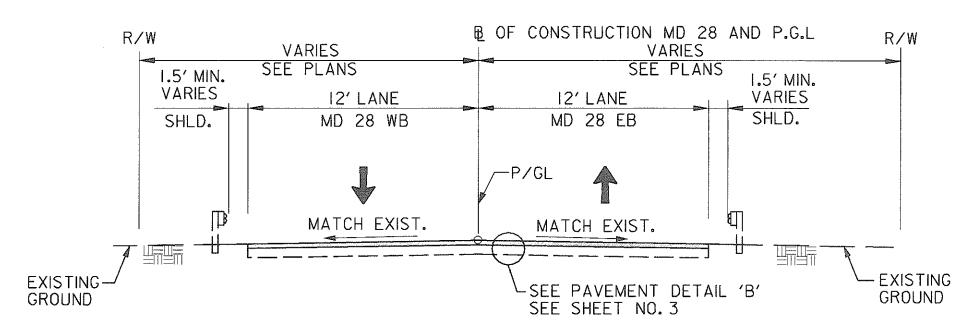
 102+75.00
 12' WITH 9' SHLD.
 12' WITH 4' SHLD.

 103+25.00
 12' WITH 1.5' SHLD.
 12' WITH 1.5' SHLD.

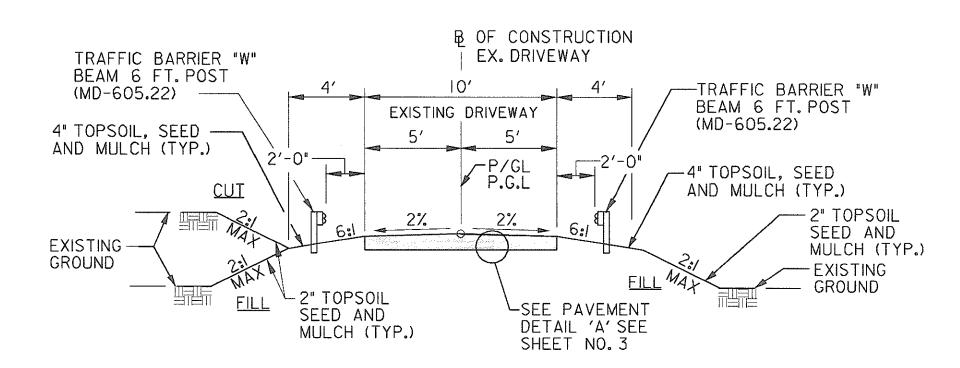
 103+55.00
 12' WITH 1.5' SHLD.
 12' WITH 1.5' SHLD.

\* SHOULDER WIDTH VARIES STA.100+45.51 LT/RT TO 101+00 LT/RT, STA.102+75 LT/RT TO 103+25 LT/RT.

NOTE: FOR TRAFFIC BARRIER LOCATIONS ON BRIDGE SEE BRIDGE PLANS AND BRIDGE TYPICALS.



# MD 28 - GRINDING AND RESURFACING STA. 100+00.00 TO STA. 100+45.51 STA. 103+05.57 TO 103+55.00

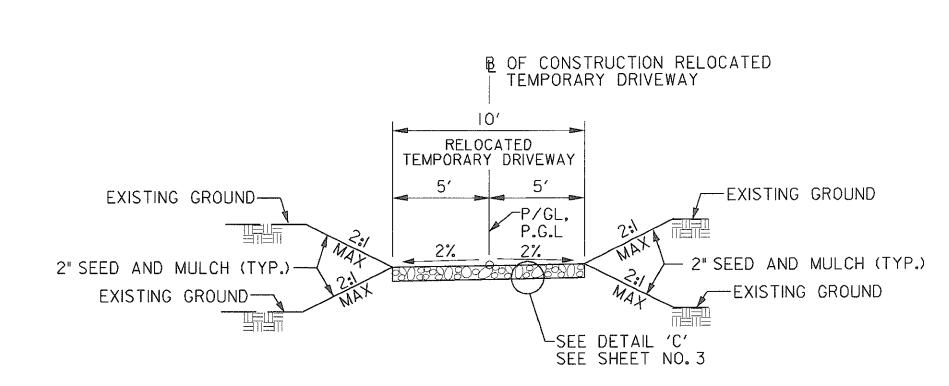


# EXISTING DRIVEWAY- PROPOSED TIE-IN

STA. 200+00 TO STA. 200+53.00

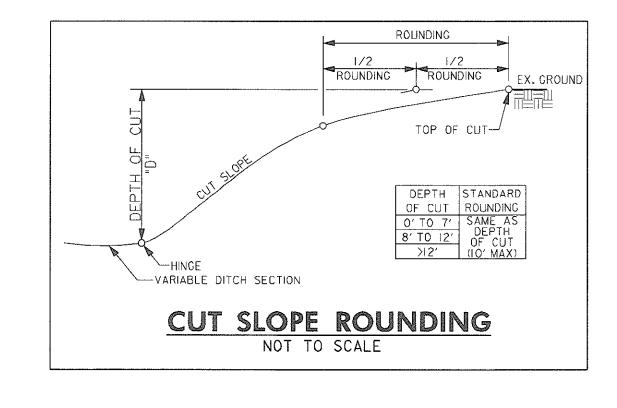
NOTE:

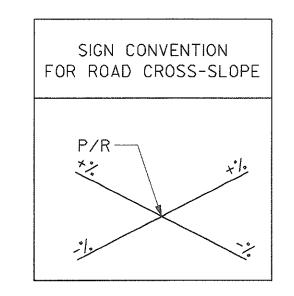
ANY DISTURBED AREAS 3:1 OR FLATTER AND NOT PAVED SHALL RECEIVE 4" TOPSOIL, SEEDING, AND MULCHING UNLESS OTHERWISE NOTED. DISTURBED AREAS STEEPER THAN 3:1 SHALL RECEIVE 2" TOPSOIL, SEEDING, AND MULCHING UNLESS OTHERWISE NOTED.



# RELOCATED TEMPORARY DRIVEWAY

STA.300+00 TO STA.300+90.86 (SEE SHT.NO.4 TITLED GEOMETRIC LAYOUT FOR LOCATION)







STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

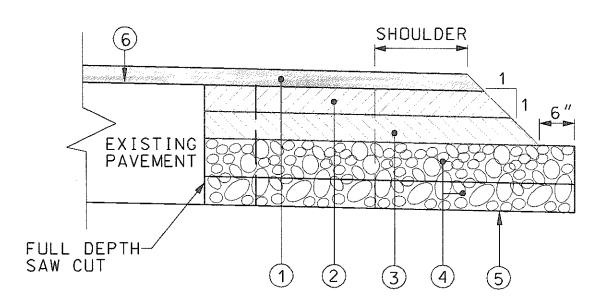
| CROSS REFERENCE   |                                 | R /W PLAT NUMBER | REVISIONS    | TYPICAL SECTIONS  |           |
|---|---------------------------------|------------------|--------------|---|-----------|
| TYPICAL SHEETS  | SHEET NOs.<br>2,3               |                  |              | SCALE 1"=5' DATE NOVEMBER, 2007 CONTRACT NO.  | AX4695180 |
| GEOMETRIC LAYOUT SHEETS ROADWAY PLAN SHEETS ROADWAY PROFILE SHEETS EROSION & SEDIMENT CONTROL SIGNING & MARKING PLANS GRADE TABLE | 4<br>5<br>6,7<br>12–16<br>17,18 |                  | JAN 0 8 2098 | DESIGNED BY T.B COUNTY FREDER  DRAWN BY T.G.P LOGMILE  CHECKED BY R.D HORIZONTAL SCALE  F.A.P. NO. VERTICAL SCALE |           |
|   |                                 |                  |              | DRAWING NO. TS-01 OF 02 SHEET NO.   | 2 OF 53   |

SABRA, WANG & ASSOCIATES, INC.

1504 JOH AVENUE
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FILE: R:\2003\22 BC\$ 2002-26G WMA Bridge\Task 11\DWG\pHT-D001\_MD28.dgn

# MD 28 - MAIN LINE BRIDGE APPROACH, TIE-IN TO EXISTING DRIVEWAY, INCLUDING SHOULDERS NEW CONSTRUCTION AND BASE WIDENING



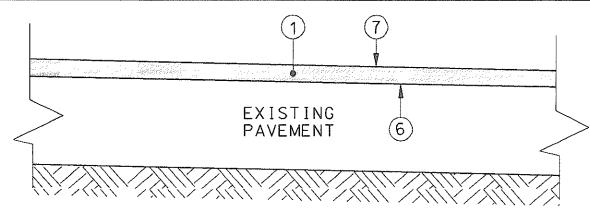
- 1) 1.5" HOT MIX ASPHALT SUPERPAVE 9.5 MM FOR SURFACE, PG64-22, LEVEL-2
- (2) 2.0" HOT MIX ASPHALT SUPERPAVE 19.0 MM FOR BASE, PG64-22, LEVEL-2
- 3 3.5" HOT MIX ASPHALT SUPERPAVE 19.0 MM FOR BASE, PG 64-22, LEVEL 2
- 4) 6.0" BASE COURSE USING GRADED AGGREGATE
- (5) TOP OF SUBGRADE AND LIMIT OF CLASS 1 EXCAVATION
- (6) TOP OF EXISTING HMA PAVEMENT AFTER 1.5" GRINDING

PAVEMENT DETAIL 'A'

### NOTES:

- I. SAWCUT SHALL BE INCIDENTAL TO CLASS 1 EXCAVATION.
- 2. USE THE FOLLOWING ITEM FOR WEDGE AND LEVEL COURSE AS DIRECTED BY THE ENGINEER: HOT MIX ASPHALT SUPERPAVE 9.5 mm FOR WEDGE/LEVEL PG64-22, LEVEL 2 (1.0" MINIMUM LIFT, 2.0" MAXIMUM LIFT).
- 3. BASED ON ROADWAY CORES AND CONSTRUCTION HISTORY, THE PAVEMENT THICKNESS OF MD 28 IS APPROXIMATELY 4" OF HMA OVER 6.5 OF JRCP.

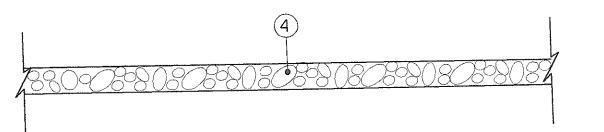
# MD 28 - GRINDING AND RESURFACING



- 1.5" HOT MIX ASPHALT SUPERPAVE 9.5 MM FOR SURFACE, PG64-22, LEVEL-2
- 6 TOP OF EXISTING HMA PAVEMENT AFTER 1.5" GRINDING
- (7) TOP OF PAVEMENT AFTER 1.5" RESURFACING

PAVEMENT DETAIL 'B'

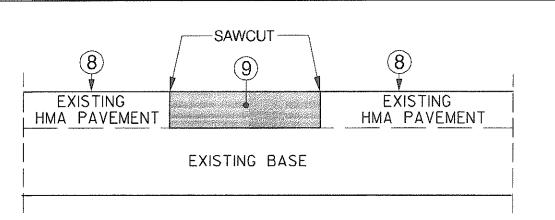
# MD 28 - RELOCATED TEMPORARY DRIVEWAY



4 6.0" BASE COURSE USING GRADED AGGREGATE

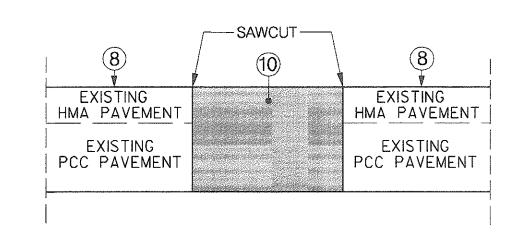
DETAIL 'C'

## MD 28 - PARTIAL DEPTH PATCHING



- 8 TOP OF EXISTING PAVEMENT PRIOR TO GRINDING
- 9 4" HOT MIX ASPHALT SUPERPAVE 19.0 MM FOR PARTIAL-DEPTH PATCHING PG 64-22, LEVEL 2 (2" MIN, LIFT THICKNESS AND 4" MAX.LIFT THICKNESS)

## MD 28 - FULL DEPTH PATCHING



- 8 TOP OF EXISTING PAVEMENT PRIOR TO GRINDING
- VARIABLE DEPTH HOT MIX ASPHALT SUPERPAVE 19.0 MM FOR FULL-DEPTH PATCHING- PG 64-22. LEVEL 2 (2" MIN. LIFT, 4" MAX. LIFT THICKNESS) (SEE NOTE 3)

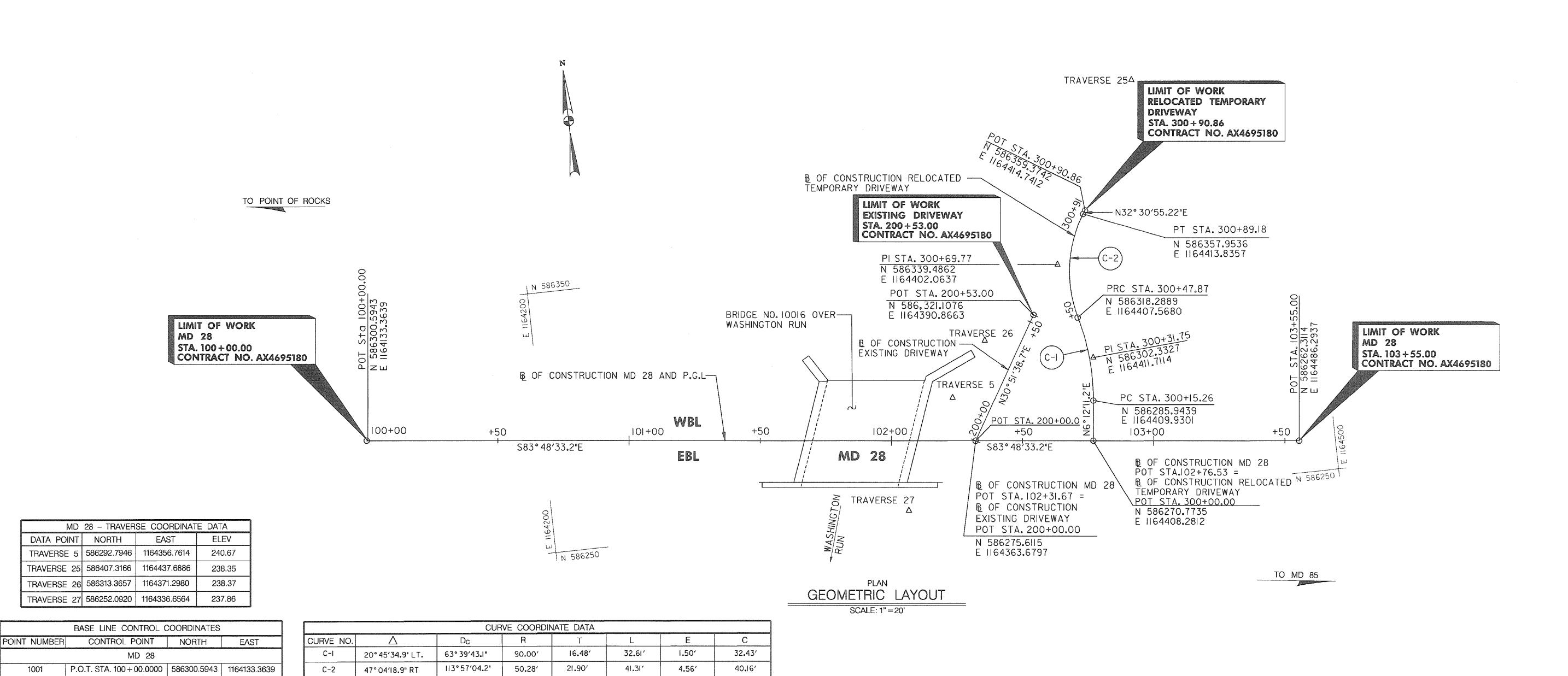
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STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

| CROSS REFERENCE                             |                       | R /W PLAT NUMBER | REVISIONS    | TYPICAL SI                     | ECTIONS                           |
|---|-----------------------|------------------|--------------|--------------------------------|-----------------------------------|
| ITEM TYPICAL SHEETS                         | SHEET NOs.<br>2,3     |                  | E S. L       | SCALE N.T.S. DATE NOVEMBER, 2  | 007 CONTRACT NO. <u>AX4695180</u> |
| GEOMETRIC LAYOUT SHEETS ROADWAY PLAN SHEETS | 4<br>5                |                  | JAN 0 8 2008 | DESIGNED BY T.B                | COUNTY FREDERICK                  |
| ROADWAY PROFILE SHEETS                      | 6,7<br>12–16<br>17,18 |                  |              | DRAWN BY T.G.P  CHECKED BY R.D | LOGMILEHORIZONTAL SCALE           |
| GRADE TABLE                                 | 19                    |                  |              | F.A.P. NO.                     | VERTICAL SCALE                    |
|   |                       |                  |              | DRAWING NO. TS-02 OF           | <b>Q2</b> SHEET NO. 3 OF 53       |

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STATE OF MARYLAND
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STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

GEOMETRIC LAYOUT CROSS REFERENCE R / W PLAT NUMBER REVISIONS ITEM SHEET NOs. DATE NOVEMBER, 2007 CONTRACT NO. AX4695180 SCALE \_\_1" = 20' TYPICAL SHEETS. 2,3 GEOMETRIC LAYOUT SHEETS. JAN 0 8 2008 COUNTY FREDERICK DESIGNED BY \_\_\_\_\_T.B ROADWAY PLAN SHEETS .. ROADWAY PROFILE SHEETS ... 6,7 DRAWN BY \_\_\_\_\_T.G.P LOGMILE EROSION & SEDIMENT CONTROL. 12-16 SIGNING & MARKING PLANS .... 17,18 CHECKED BY R.D HORIZONTAL SCALE GRADE TABLE F.A.P. NO. VERTICAL SCALE \_\_ GS-01 of 01 DRAWING NO. SHEET NO. 4 OF 53

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P.O.T. STA. 300+00.0000 586270.7735 1164408.2812
P.C STA. 300+15.2597 586285.9439 1164409.9301

P.R.C STA. 300 + 47.8690 | 586318.2889 | 1164407.5680

P.T STA. 300 + 89.1778 | 586357.9536 | 1164413.8357

P.O.T STA. 300 + 90.8624 | 586359.3742 | 1164414.7412

 P.O.T STA. 200 + 00.0000
 586275.6115
 1164363.6797

 P.O.T STA. 200 + 53.00
 586321.1076
 1164390.8663

586302.3327

1164411.7114

586339.4862 1164402.0637

RELOCATED TEMPORARY DRIVEWAY

EXISTING DRIVEWAY

P.I STA: 300 + 31.7451

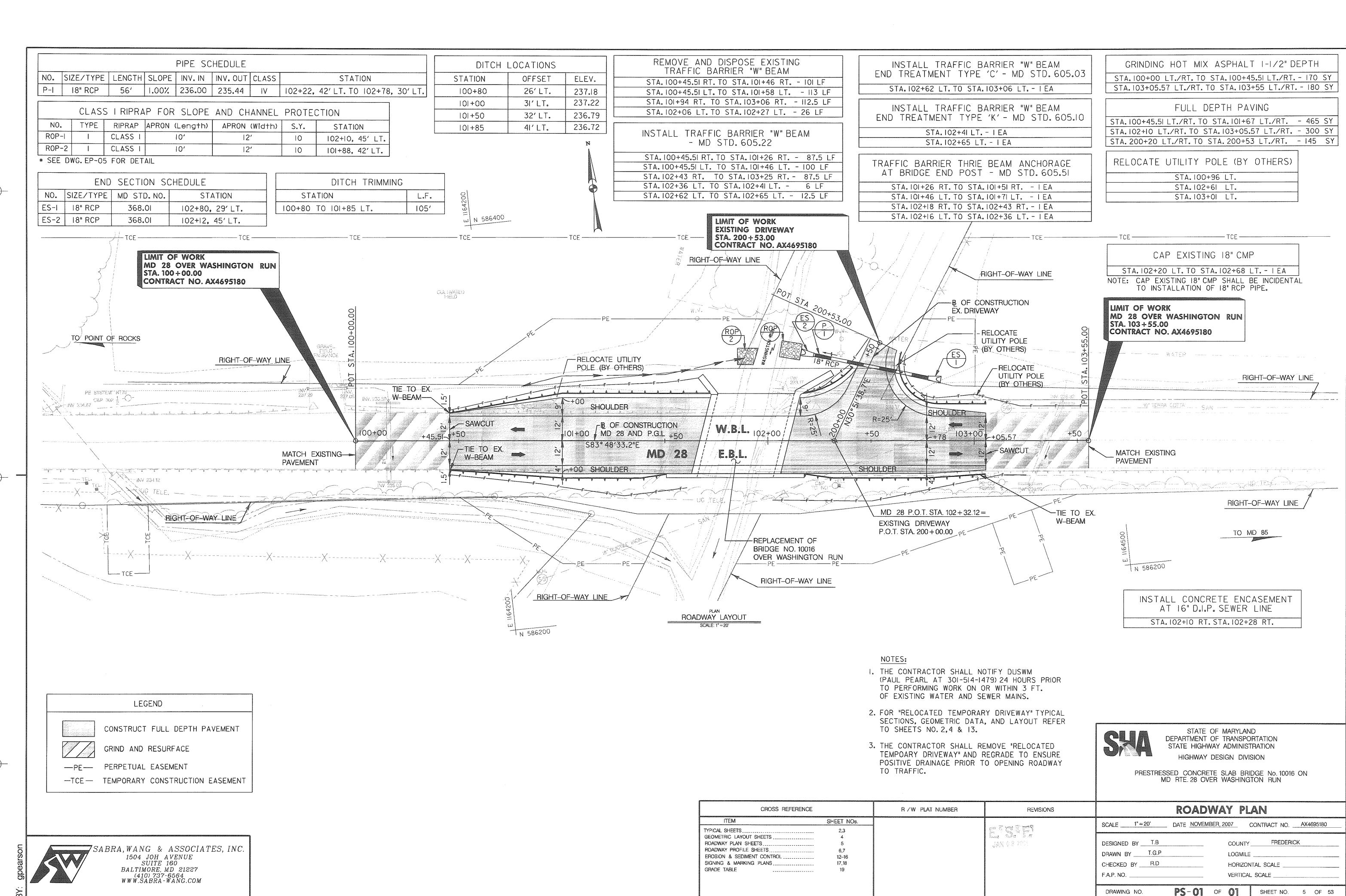
P.I STA, 300 + 69,7693

1005

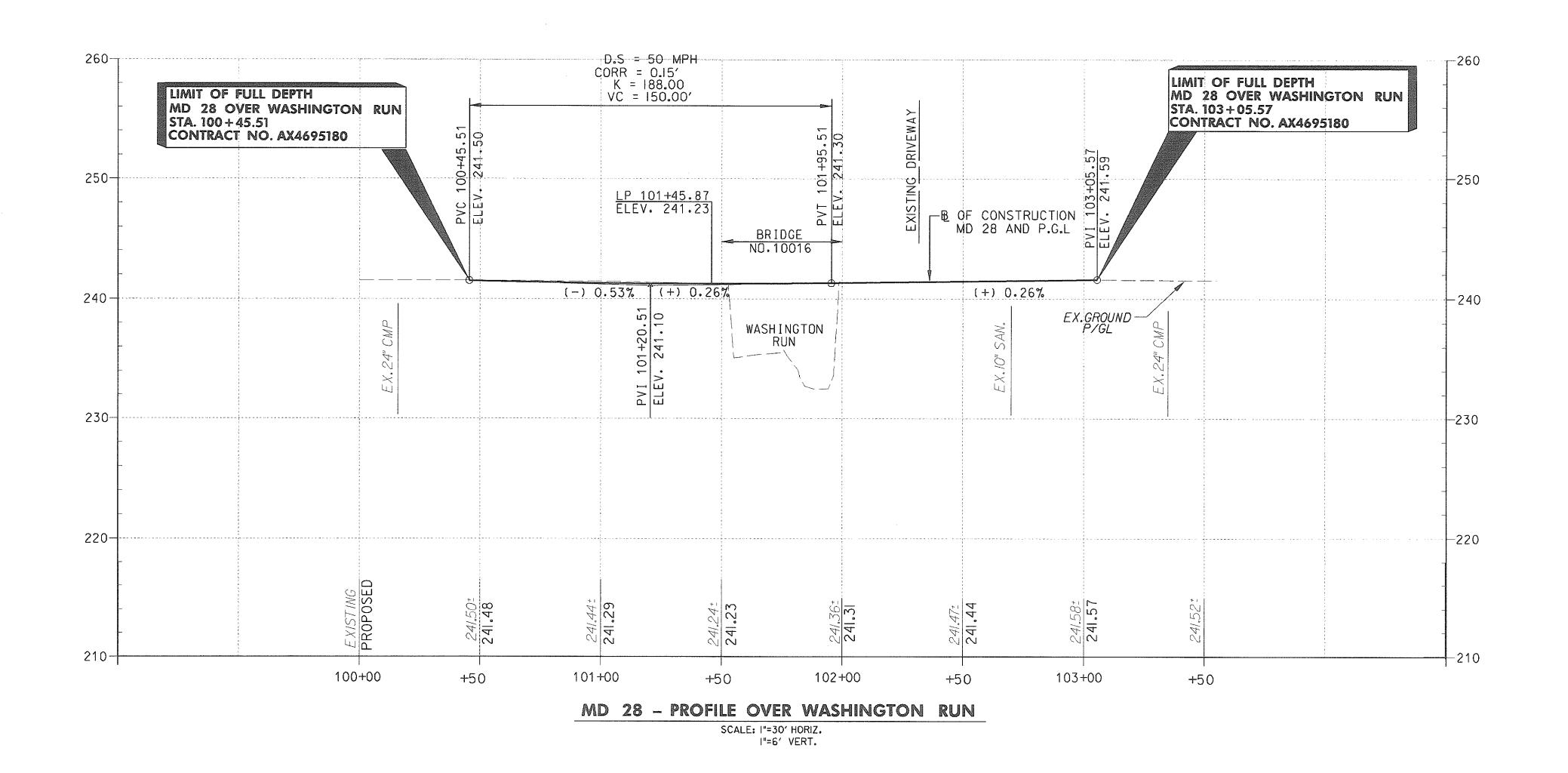
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HORIZONTAL DATUM: NAD 83/91 VERTICAL DATUM: NAVD 88

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SMA

STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

| CROSS REFERENCE   | ## 1985                | R /W PLAT NUMBER | REVISIONS    | ROADW#  | AY PROFILE   |
|---|------------------------|------------------|--------------|---|--|
| ITEM TYPICAL SHEETS   | SHEET NOs.             |                  |              | SCALE DATE NOVEMBE  | ER, 2007 CONTRACT NO. <u>AX4695180</u>                   |
| GEOMETRIC LAYOUT SHEETS ROADWAY PLAN SHEETS ROADWAY PROFILE SHEETS EROSION & SEDIMENT CONTROL SIGNING & MARKING PLANS GRADE TABLE | 4<br>5<br>6,7<br>12–16 |                  | JAN 0 8 2008 | DESIGNED BYT.B<br>DRAWN BYT.G.P<br>CHECKED BYR.D<br>F.A.P. NO | COUNTY FREDERICK  LOGMILE 1"=30'  HORIZONTAL SCALE 1"=6" |
|   |                        |                  |              | DRAWING NO. PR-01   | OF <b>02</b> SHEET NO. 6 OF 53                           |

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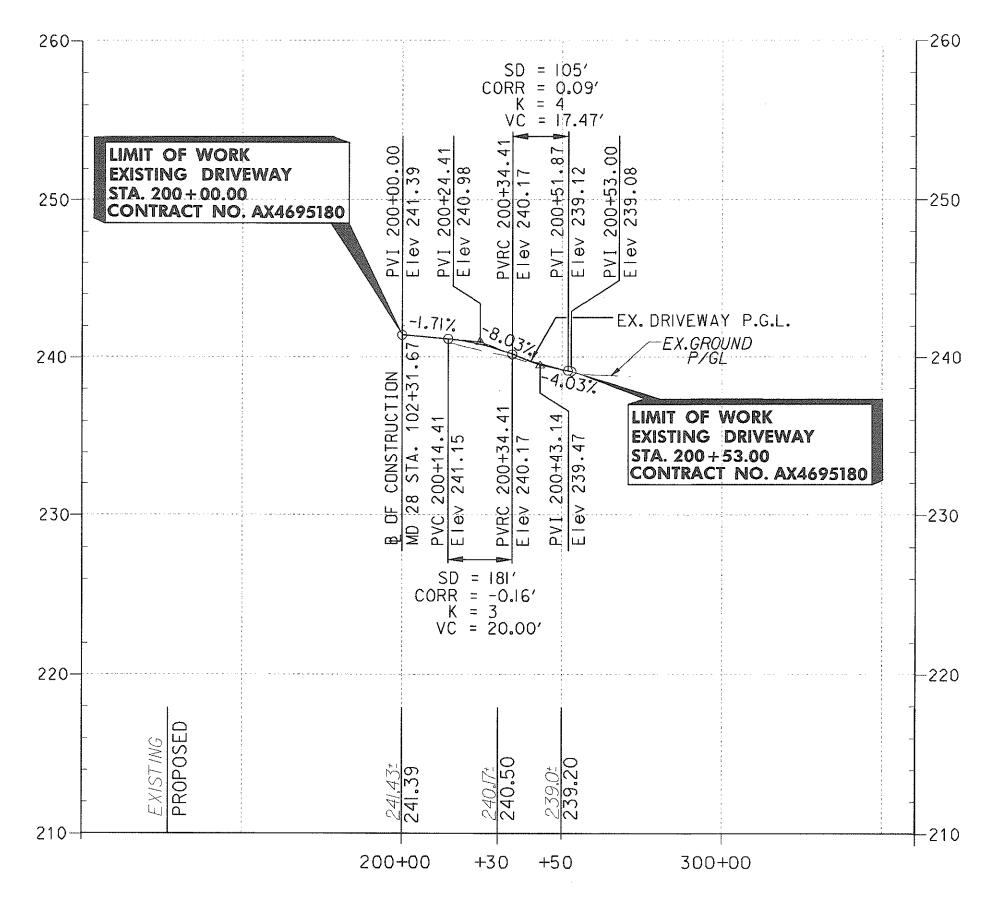
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LIMIT OF WORK (GRAVEL) TEMPORARY DRIVEWAY STA. 300+00.00 CONTRACT NO. AX4695180 SD = 53' CORR = 0.32' K = 4 LIMIT OF WORK (GRAVEL)
TEMPORARY DRIVEWAY
STA. 300 + 89.89.18
CONTRACT NO. AX4695180 PVRC 300+34.00 Elev 240.18 250-250 PVT 300+66.0 Elev 238.72 240--240 230--230 CORR = -0.16' K = 3 VC = 20.00' 220--220 210 +50 301+00 +50 300+00

MD28 - RELOCATED TEMPORARY DRIVEWAY PROFILE

SCALE: I\*=30' HORIZ.
I\*=6' VERT.



MD 28 – EXISTING DRIVEWAY PROFILE

SCALE: I'=30' HORIZ.
I'=6' VERT.



STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

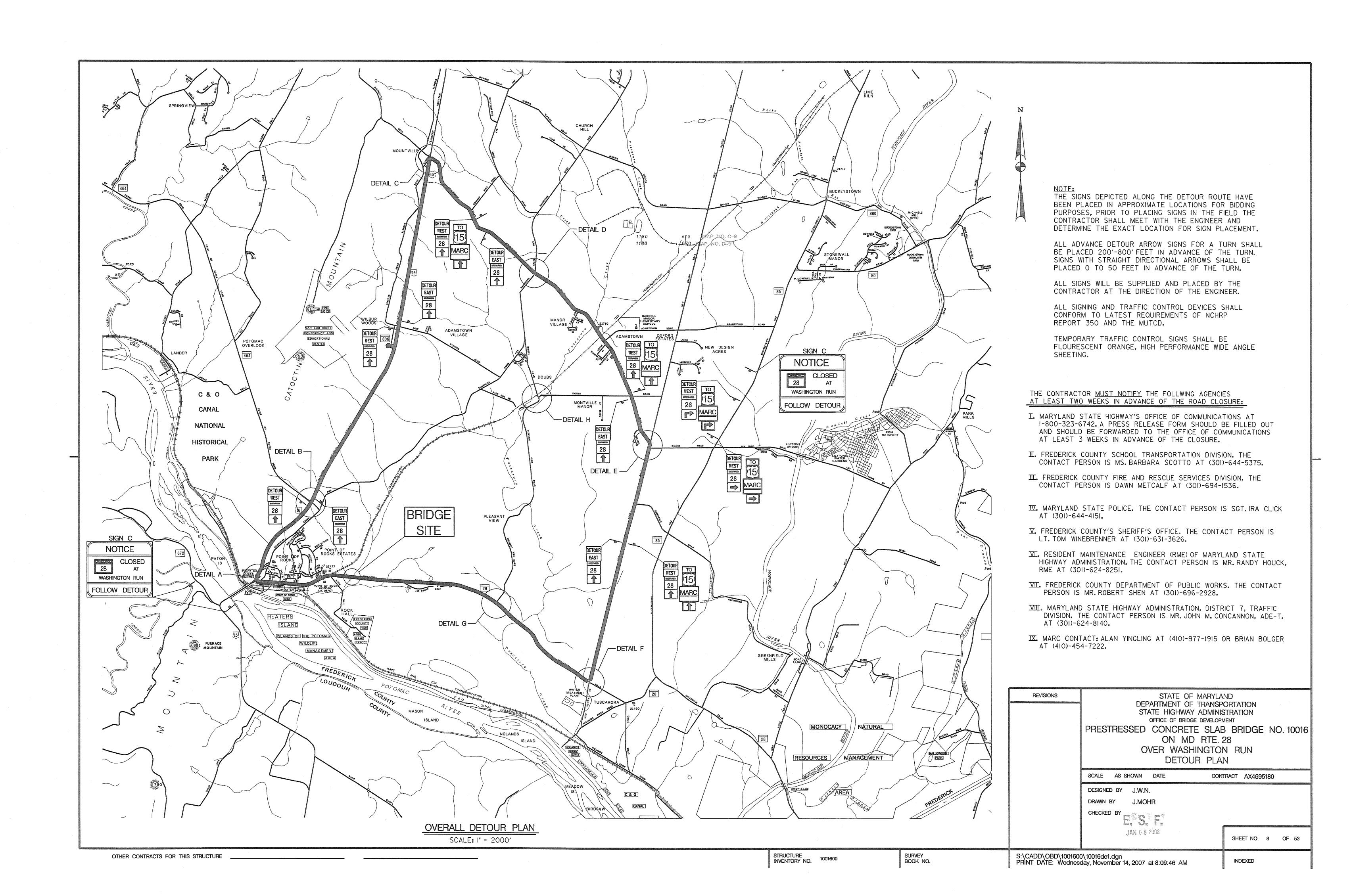
PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

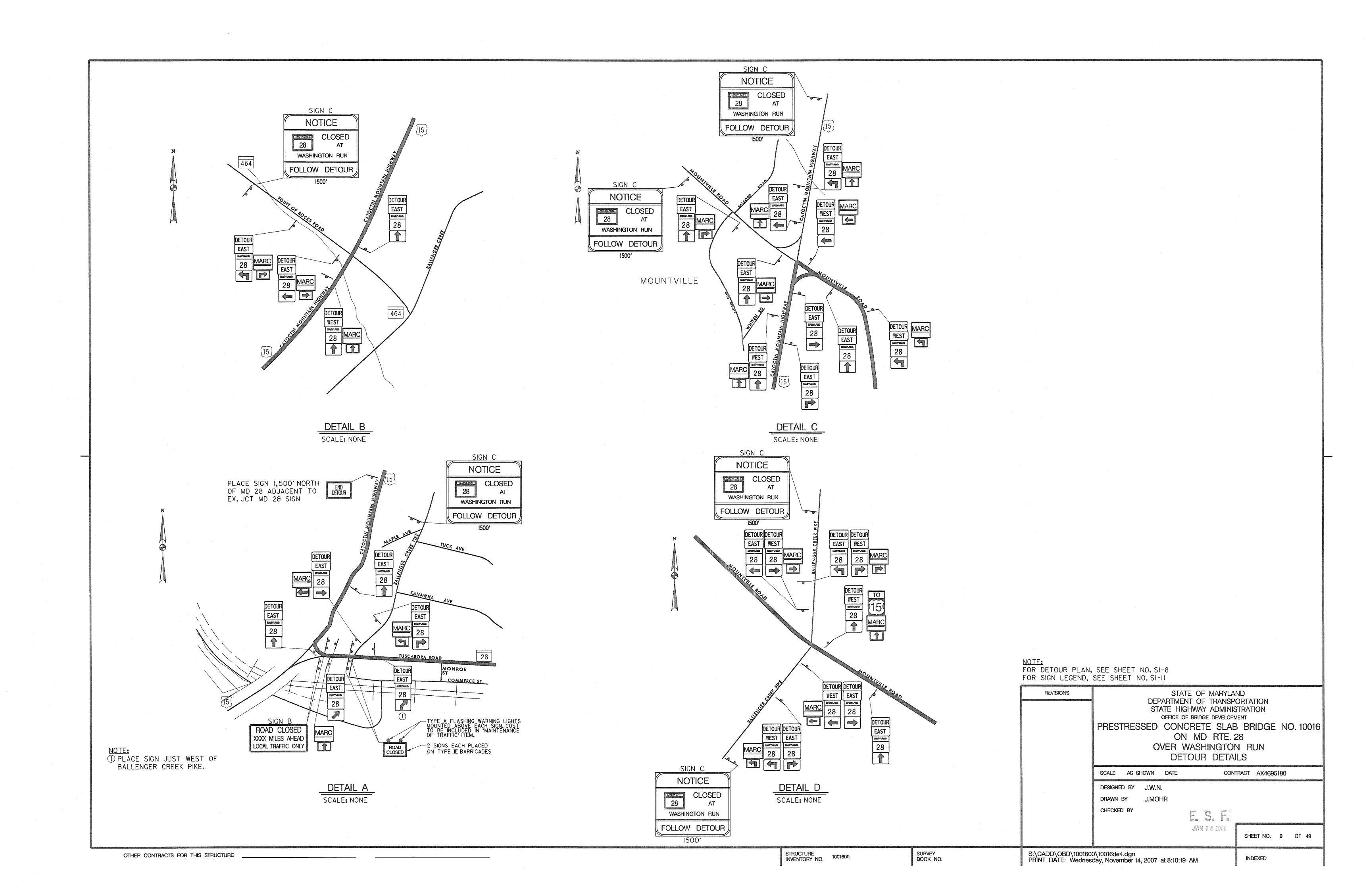
| CROSS REFERENCE   |                                       | R /W PLAT NUMBER | REVISIONS    | ROADW   | AY PROFILE   |
|---|---------------------------------------|------------------|--------------|---|--|
| TYPICAL SHEETS  | SHEET NOs.<br>2,3                     |                  |              | SCALE DATE NOVEME   | BER, 2007 CONTRACT NO. AX4695180   |
| GEOMETRIC LAYOUT SHEETS ROADWAY PLAN SHEETS ROADWAY PROFILE SHEETS EROSION & SEDIMENT CONTROL SIGNING & MARKING PLANS GRADE TABLE | 4<br>5<br>6,7<br>12–16<br>17,18<br>19 |                  | JAN 0 8 2008 | DESIGNED BYT.B<br>DRAWN BYT.G.P<br>CHECKED BYR.D<br>F.A.P. NO | COUNTY FREDERICK  LOGMILE  HORIZONTAL SCALE 1"=30'  VERTICAL SCALE 1"=6" |
|   |                                       |                  |              | DRAWING NO. PR-02   | OF <b>02</b> SHEET NO. 7 OF 53   |

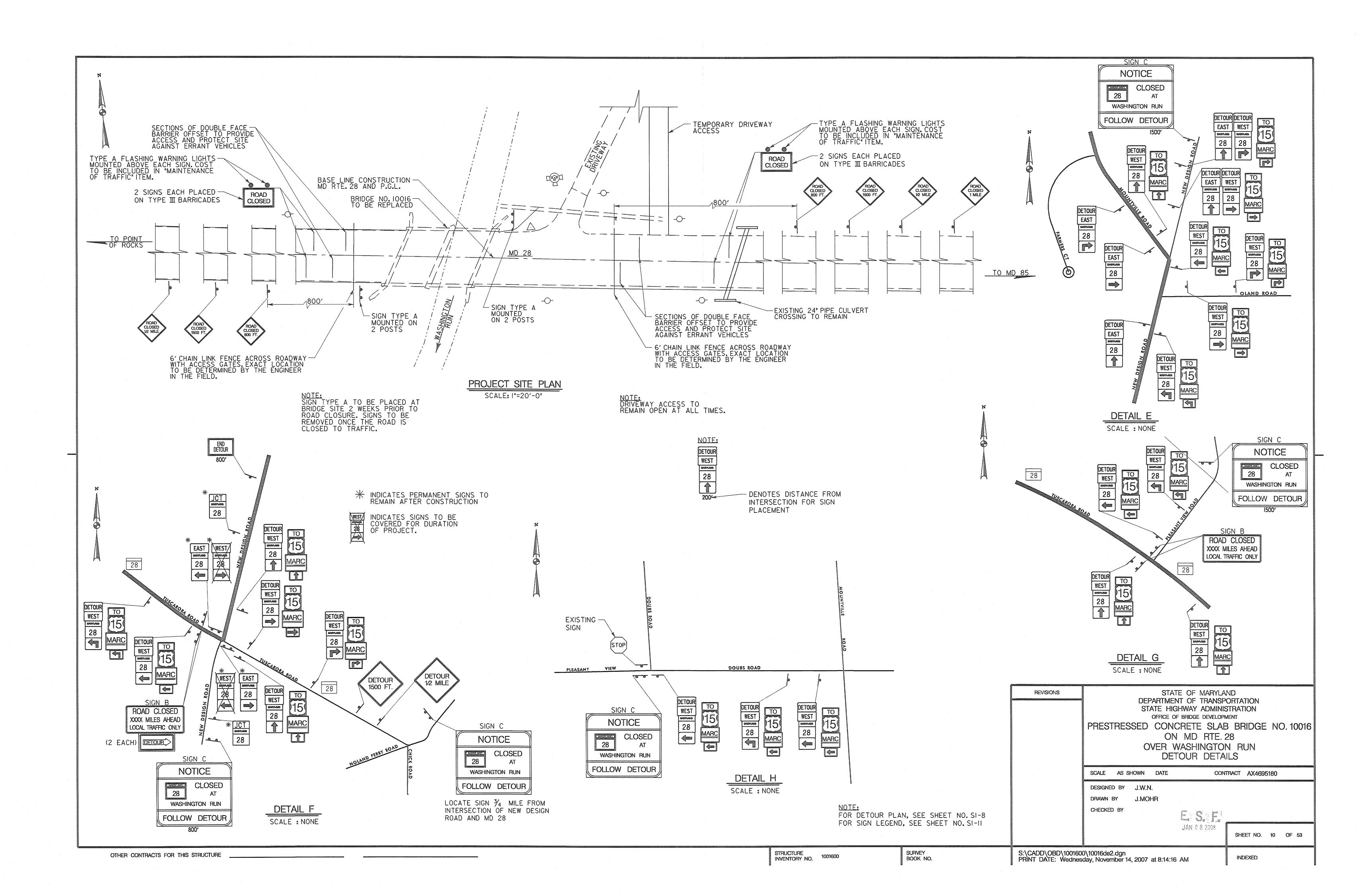
SABRA, WANG & ASSOCIATES, INC.

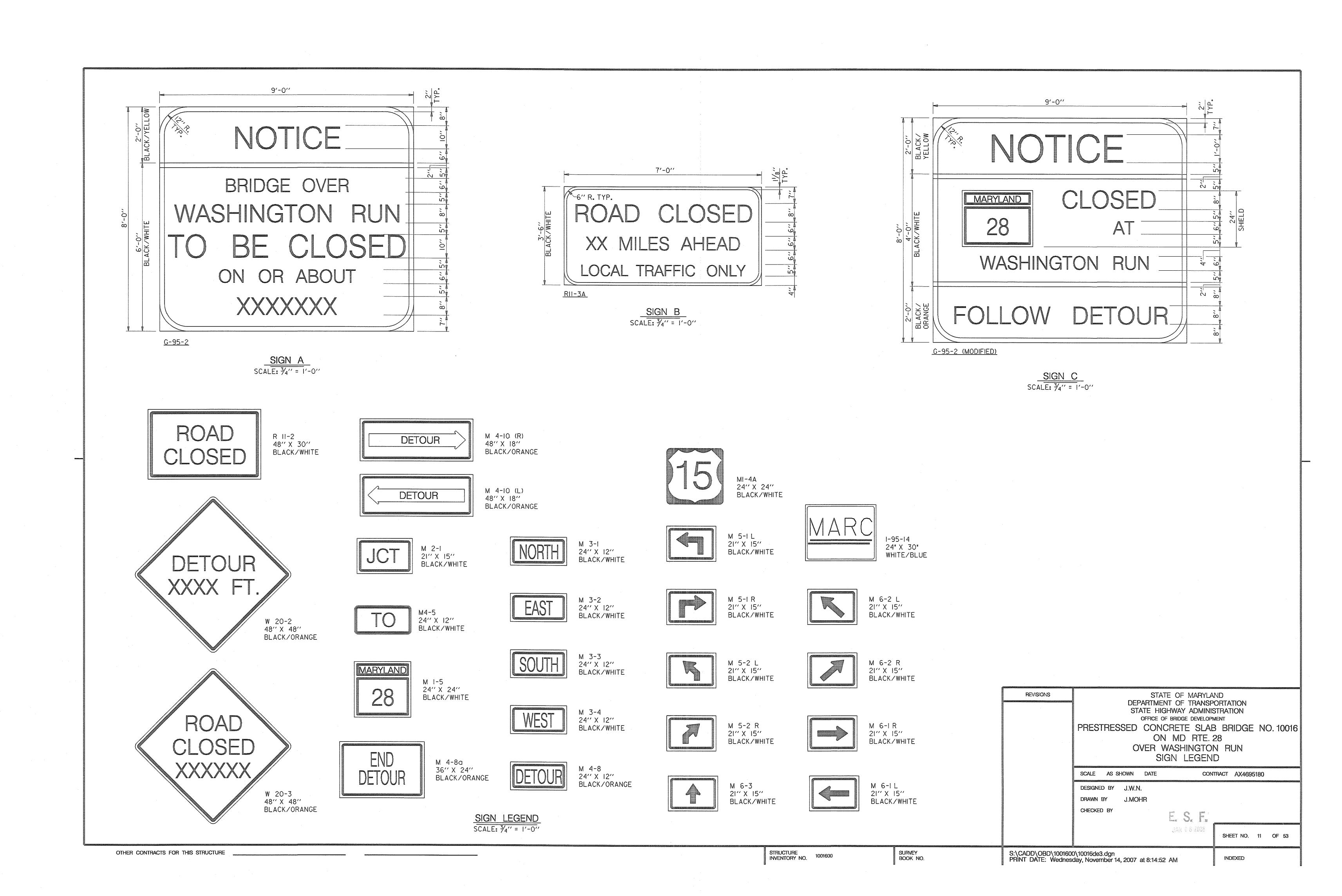
1504 JOH AVENUE
SUITE 160
BALTIMORE, MD 21227
(410) 737-6564
WWW.SABRA-WANG.COM

PLOTTED: Sunday, November 18, 2007 AT 09:02 AM FILE: R:\2003\22 BCS 2002-26G WMA Bridge\Task 11\DWG\pHP-V002\_md28.dgn









# EROSION AND SEDIMENT CONTROL - GENERAL NOTES

1. MDE NOTIFICATION

IF AN EROSION AND SEDIMENT CONTROL PERMIT IS ISSUED FOR THIS PROJECT. THE CONTRACTOR, UPON APPROVAL FROM SHA. MUST NOTIFY MDE IN WRITING AND/OR BY TELEPHONE AT (410) 537-3510 AT THE FOLLOWING POINTS:

- PRE-CONSTRUCTION MEETING

- EROSION AND SEDIMENT CONTROL MEETING (MINIMUM 7 WORKING DAYS PRIOR TO COMMENCING EARTH DISTURBING ACTIVITIES)
   FOLLOWING INSTALLATION OF INITIAL SEDIMENT CONTROL MEASURES
- DURING INSTALLATION OF MAJOR SEDIMENT CONTROL
  BASINS/TRAPS
- PRIOR TO REMOVAL OR MODIFICATION OF ANY SEDIMENT CONTROL STRUCTURE(S)
- PRIOR TO REMOVAL OF ALL SEDIMENT CONTROL DEVICES PRIOR TO FINAL ACCEPTANCE BY SHA

## 2. STANDARDS AND SPECIFICATIONS

THIS PLAN IS DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL. THE 2000 MARYLAND STORMWATER DESIGN MANUAL. VOLUMES I & II AND THE MARYLAND DEPARTMENT OF ENVIRONMENT EROSION AND SEDIMENT CONTROL AND STORMWATER MANAGEMENT REGULATIONS. AND ALL REVISIONS THERE OF. AND AS SPECIFIED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL KEEP A COPY OF THE 1994 "MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL" ON THE SITE AT ALL TIMES.

# 3. INGRESS / EGRESS CONTROLS

THE CONTRACTOR SHALL PROTECT ALL POINTS OF CONSTRUCTION INGRESS AND EGRESS TO PREVENT THE DEPOSITION OF MATERIALS ON PUBLIC ROADS. ALL MATERIALS DEPOSITED ON PUBLIC ROADS SHALL BE MECHANICALLY REMOVED IMMEDIATELY. THE FLUSHING OF ROAD SURFACES IS PROHIBITED.

TYPICALLY. ALL INGRESS AND EGRESS POINTS SHALL BE CONTROLLED THROUGH THE USE OF A "STABILIZED CONSTRUCTION ENTRANCE."

### 4. INSPECTION

THE CONTRACTOR SHALL INSPECT DAILY AND MAINTAIN CONTINUOUSLY IN AN EFFECTIVE OPERATING CONDITION ALL EROSION AND SEDIMENT CONTROL MEASURES.

# 5. SHUTDOWNS AND OR PENALTIES

TOTAL COMPLIANCE WITH THE APPROVED EROSION AND SEDIMENT CONTROL PLAN IS EXPECTED AT ALL TIMES. IN CASES WHERE THE CONTRACTOR IS FOUND TO BE IN NON-COMPLIANCE SHA MAY TAKE STEPS TO IMPOSE SELECTED OR TOTAL SHUTDOWNS AND IMPOSE PER DAY PENALTIES FOR NON-COMPLIANCE.

THE DISTRICT ENGINEER CAN IMPOSE A TOTAL OR PARTIAL SHUTDOWN IF THE PROJECT MAY ADVERSELY IMPACT THE WATERS OF THE STATE.

# 6. RECORD KEEPING

THE PROJECTS' APPROVAL LETTER. APPROVED EROSION AND SEDIMENT CONTROL PLANS. APPROVED CHANGE REQUESTS. DAILY LOG BOOKS AND TEST REPORTS WILL BE AVAILABLE AT THE SITE FOR INSPECTION BY DULY AUTHORIZED OFFICIALS OF MDE.

# 7. EROSION AND SEDIMENT CONTROL EXCAVATION

SILT REMOVED FROM CONTROL DEVICES SHALL BE PLACED
IN AN APPROVED WASTE SITE EITHER ON OR OFF THE PROJECT.
MATERIAL STORED ON SITE MAY BE REUSED ONCE IT
IS DRIED AND IF IT MEETS SHA REQUIREMENTS FOR
EMBANKMENT OR ANY UNSPECIFIED NEED.

## 8. OFF-SITE UTILITY WORK

SEDIMENT CONTROL FOR UTILITY CONSTRUCTION IN AREAS OUTSIDE OF DESIGNED CONTROLS SHALL FOLLOW THESE ADDITIONAL BEST MANAGEMENT PRACTICES:

- (g) CALL "MISS UTILITY" AT 1-800-257-7777 48 HOURS PRIOR TO THE START OF WORK
- (b) EXCAVATED MATERIAL SHALL BE PLACED ON THE HIGH SIDE OF THE TRENCH.
- (c) TRENCHES FOR UTILITY INSTALLATIONS SHALL BE BACKFILLED.

  COMPACTED AND STABILIZED AT THE END OF EACH WORKING

  DAY. WHEN THIS IS NOT POSSIBLE. THE AREA SHALL CONFORM

  TO (d).
- (d) TEMPORARY SILT FENCES SHALL BE PLACED IMMEDIATELY DOWNSTREAM OF ANY DISTURBED AREA INTENDED TO REMAIN DISTURBED FOR MORE THAN ONE DAY.

## 9. SENSITIVE AREAS

NO CONSTRUCTION ACTIVITIES SHALL BE UNDERTAKEN WITHIN
SPECIFIED SENSITIVE AREAS OF THE PROJECT WITHOUT PRIOR
NOTIFICATION OF THE ENGINEER. ALL WORK IN THESE AREAS SHALL
BE MONITORED BY A RESPONSIBLE PARTY DESIGNATED BY THE
CONTRACTOR TO ASSURE THAT REASONABLE CARE IS TAKEN IN OR
ADJACENT TO THESE AREAS. AREAS CONSIDERED SENSITIVE ARE DEFINED
AS: FLOODPLAINS. WETLANDS (TIDAL. NONTIDAL AND ASSOCIATED
BUFFERS) CRITICAL AREAS. FORESTED AREAS. ARCHEOLOGICAL SITES.
HISTORIC SITES. PARKLAND AND OPEN WATER.

## 10. STANDARD STABILIZATION NOTE

FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE. PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN SEVEN (7) CALENDAR DAYS AS TO THE SURFACE OF ALL PERIMETER CONTROLS. DIKES. SWALES. DITCHES. PERIMETER SLOPES, AND ALL SLOPES GREATER THAN 3 HORIZONTAL TO 1 VERTICAL (3:1); AND FOURTEEN DAYS (14) AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE.

### 11. SITE INFORMATION . (NOT FOR BIDDING PURPOSES)

| IDIAL AREA UP SITE     | 0.43 | ACRES       |
|------------------------|------|-------------|
| AREA DISTURBED         | 0.64 | ACRES       |
| AREA TO BE ROOFED      |      | <del></del> |
| OR PAVED               | 0.19 | ACRES       |
| TOTAL CUT              | 555  | CU. YDS.    |
| TOTAL FILL             | 370  | CU. YDS.    |
| OFFSITE WASTE/BORROW   |      |             |
| AREA LOCATION ( IF KNO | MV)  | ACRES       |

### 12. INCREMENTAL STABILIZATION

REFER TO THE CURRENT MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR THE INCREMENTAL STABILIZATION OF CUT AND FILLS.

## 13. DEWATERING PRACTICES

THE CONTRACTOR IS ALERTED THAT MDE CONSIDERS DEWATERING PRACTICES TO BE ELECTIVE IN NATURE. DEWATERING PRACTICES ARE TO BE LOCATED AND OPERATED IN A MANNER THAT DOES NOT DISCHARGE SEDIMENT INTO ANY WATERWAY. NO VISIBLE CHANGES TO STREAM CLARITY ARE PERMITTED.

### 14. MODIFICATIONS

THE CONTRACTOR SHALL SUBMIT MODIFICATIONS TO THE EROSION AND SEDIMENT CONTROLS TO SHA FOR APPROVAL PRIOR TO SUBMISSION TO MOE. NO MODIFICATIONS SHALL BE IMPLEMENTED UNTIL ALL APPROVALS FROM SHA AND MOE ARE OBTAINED.

## STANDARD SYMBOLS

| TEMPORARY SWALE  PERIMETER DIKE/SWALE  STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTI | TEMPORARY SWALE  PERIMETER DIKE/SWALE  STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  | TEMPORARY SWALE  PERIMETER DIKE/SWALE  STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  A-2  |
|--|--|--|
| STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  | STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION   | STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STONE OUTLET STRUCTURE  STONE O |
| STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  | STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION   | STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STONE OUTLET STRUCTURE  STONE O |
| STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  CURD MIP  | STONE CHECK DAM  STONE OUTLET STRUCTURE  SILT FENCE  SUPER SILT FENCE  STRAW BALES  STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION   | STONE CHECK DAM CD  STONE OUTLET STRUCTURE TSOS  SILT FENCE SF SF SSF  |
| SILT FENCE SF—SF—  SUPER SILT FENCE SSF—SSF—  STRAW BALES STANDARD INLET PROTECTION SIP  AT GRADE INLET PROTECTION GIP  CURB INLET PROTECTION CIP  MEDIAN INLET PROTECTION   | SILT FENCE SF—SF—SSF—SSF—STRAW BALES STANDARD INLET PROTECTION SIP  AT GRADE INLET PROTECTION GIP  CURB INLET PROTECTION CIP   | SILT FENCE   |
| SUPER SILT FENCE   | SUPER SILT FENCE   | SILT FENCE   |
| SUPER SILT FENCE   | SUPER SILT FENCE   | SUPER SILT FENCE   |
| STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION   | STRAW BALES  STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  CIP   | SUPER SILT FENCE   |
| STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  | STANDARD INLET PROTECTION AGIP  CURB INLET PROTECTION.   | STRAW BALES  |
| CURB INLET PROTECTION  | CURB INLET PROTECTION  | STANDARD INLET PROTECTION  |
| CURB INLET PROTECTION  | CURB INLET PROTECTION  | STANDARD INLET PROTECTION  |
| MEDIAN INLET PROTECTION.   | 7 m  | STANDARD INLET PROTECTION AGIP   |
| CARION INFLOW PROTECTION   |  | AT GRADE INLET PROTECTION  |
| CAMP Print and make a state of the state of  | GABION INFLOW PROTECTION GM  | STANDARD INLET PROTECTION AGIP  AT GRADE INLET PROTECTION CIP  CURB INLET PROTECTION CIP   |
|  |  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  |
|  |  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GM  GM  |
|  | SUMP PIT   | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  RRP   |
|  |  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  GIV  RIPRAP INFLOW PROTECTION  RRP  SUMP PIT  |
| PORTABLE SEDIMENT TANK   B   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  SIP  AGIP  AGIP  CIP  MIP  SIP  AGIP  CIP  MIP  SIP  AGIP  CIP  MIP  MIP  SIP  AGIP  CIP  MIP  MIP  RRP  SUMP PIT  SIP  AGIP  CIP  MIP  AGIP  CIP  MIP  RRP  SUMP PIT  SIP  AGIP  CIP  MIP  MIP  RRP  SUMP PIT  MIP  SP  REMOVABLE PUMPING STATION  MIP  RRPS   |
|  | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  PST  18  |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  RPS  PORTABLE SEDIMENT TANK  TB  TB  |
| PORTABLE SEDIMENT TANK BERM. INTERCEPTOR BERM. TB  | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  PORTABLE SEDIMENT TANK  INTERCEPTOR BERM  TB  TB  TB  TB  TB  TB  TB  TB  TB  T   |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TB  TB  TB  TB  TB  TB  TB  TB  TB  T  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  PORTABLE SEDIMENT TANK  INTERCEPTOR BERM  TEMPORARY BERM  PIPE SLOPE ORAIN  |
| PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM  PIPE SLOPE DRAIN   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE DRAIN.  STABILIZED CONSTRUCTION ENTRANCE  |
| PORTABLE SEDIMENT TANK  INTERCEPTOR BERM  TEMPORARY BERM  PIPE SLOPE ORAIN  STABILIZED CONSTRUCTION ENTRANCE  SOIL STABILIZATION MATTING   | PORTABLE SEDIMENT TANK   | STANDARD INLET PROTECTION  ACIP  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  STABILIZED CONSTRUCTION ENTRANCE  SOIL STABILIZATION MATTING  |
| PORTABLE SEDIMENT TANK  INTERCEPTOR BERM.  TB  TB  TB  TB  TB  TB  TB  TB  TB  T   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION  MEDIAN INLET PROTECTION  GABION INFLOW PROTECTION  RIPRAP INFLOW PROTECTION  SUMP PIT  REMOVABLE PUMPING STATION  INTERCEPTOR BERM  PIPE SLOPE DRAIN  STABILIZED CONSTRUCTION ENTRANCE  PLACED RIPRAP DITCH  |
| PORTABLE SEDIMENT TANK  INTERCEPTOR BERM.  TEMPORARY BERM  PIPE SLOPE DRAIN  STABILIZED CONSTRUCTION ENTRANCE  SOIL STABILIZATION MATTING  PLACED RIPRAP DITCH  GABIONS  | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION   |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION.  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE DRAIN.  STABILIZATION MATTING.  PLACED RIPRAP DITCH.  GABIONS.  CONCRETE GUTTER  |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE ORAIN.  STABILIZED CONSTRUCTION ENTRANCE  SOIL STABILIZATION MATTING.  PLACED RIPRAP DITCH.  GABIONS.  CONCRETE GUTTER.  STONE DUTLET SEDIMENT TRAP.   | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION.  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  INTERCEPTOR BERM.  PIPE SLOPE DRAIN.  STABILIZED CONSTRUCTION ENTRANCE  PLACED RIPRAP DITCH.  GABIONS.  CONCRETE GUTTER  STONE OUTLET SEDIMENT TRAP.  |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION.  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  CABION INFLOW PROTECTION.  SUMP PIT   |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  AT GRADE INLET PROTECTION   |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AT GRADE INLET PROTECTION.  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE DRAIN.  STABILIZATION MATTING.  PLACED RIPRAP DITCH.  CONCRETE GUTTER.  STONE PUTLET SEDIMENT TRAP.  PIPE OUTLET SEDIMENT TRAP.  PIPE OUTLET SEDIMENT TRAP.   |
| PORTABLE SEDIMENT TANK   | PORTABLE SEDIMENT TANK  PORTABLE SEDIMENT TANK  INTERCEPTOR BERM  TEMPORARY BERM  PIPE SLOPE DRAIN  STABILIZED CONSTRUCTION ENTRANCE  SOIL STABILIZATION MATTING  PLACED RIPRAP DITCH  GABIONS  CONCRETE GUTTER  STONE DUTLET SEDIMENT TRAP  STONE/RIPRAP OUTLET SEDIMENT TRAP  PIPE DUTLET SEDIMENT TRAP  PIPE DUTLET SEDIMENT TRAP  LIMIT OF DISTURBANCE  PST RPS  PST BRANCE  TB  TB  TB  TB  TB  TB  TB  TB  TB  T | STANDARD INLET PROTECTION AGIP  ACT GRADE INLET PROTECTION.  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  GABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE DRAIN.  STABILIZED CONSTRUCTION ENTRANCE.  SOIL STABILIZATION MATTING.  PLACED RIPRAP DITCH.  GABIONS.  CONCRETE GUTTER.  STONE OUTLET SEDIMENT TRAP.  PIPE OUTLET SEDIMENT TRAP.  PIPE OUTLET SEDIMENT TRAP.  PIPE OUTLET SEDIMENT TRAP.  LIMIT OF DISTURBANCE.  LOD  |
| PORTABLE SEDIMENT TANK   | REMOVABLE PUMPING STATION  | STANDARD INLET PROTECTION  AGIP  CURB INLET PROTECTION.  MEDIAN INLET PROTECTION.  CABION INFLOW PROTECTION.  RIPRAP INFLOW PROTECTION.  SUMP PIT.  REMOVABLE PUMPING STATION.  PORTABLE SEDIMENT TANK.  INTERCEPTOR BERM.  TEMPORARY BERM.  PIPE SLOPE ORAIN.  STABILIZED CONSTRUCTION ENTRANCE.  SOIL STABILIZATION MATTING.  PLACED RIPRAP DITCH.  GABIONS.  CONCRETE GUTTER.  STONE OUTLET SEDIMENT TRAP.  RIPRAP OUTLET SEDIMENT TRAP.  PIPE DUTLET SEDIMENT TRAP.  LIMIT OF DISTURBANCE.  LOD  EXISTING CONTOURS.  -100  |

### DESIGN CERTIFICATION

| "I HEREBY CERTIFY THAT THIS PLAN THE 1994 STANDARDS AND SPECIFICATIONS F THE 2000 MARYLAND STORMWATER DESIGN MAN DEPARTMENT OF THE ENVIRONMENT EROSION A MANAGEMENT REGULATIONS. | IIAI . VIII IIMES I A II AND THE MANTENIO |
|--|---|
| Ziad A. Sabra<br>NAME  | SUSNATURE                                 |
| 18560  | 02/11/2008                                |
| MARYLAND. REGISTRATION NUMBER. (P.E.), R.L.S. OR R.L.A. (circle)   | DATE                                      |

### PROFESSIONAL CERTIFICATION

"I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME AND THAT I AM A DULY PROFESSIONAL ENGINEER UNDER THE LAW OF

| STATE | OF | MARYLAND, LICEN | SE No  | 18560    |  |
|-------|----|-----------------|--------|----------|--|
|       |    | EXPIRATION      | DATE _ | 01-14-09 |  |



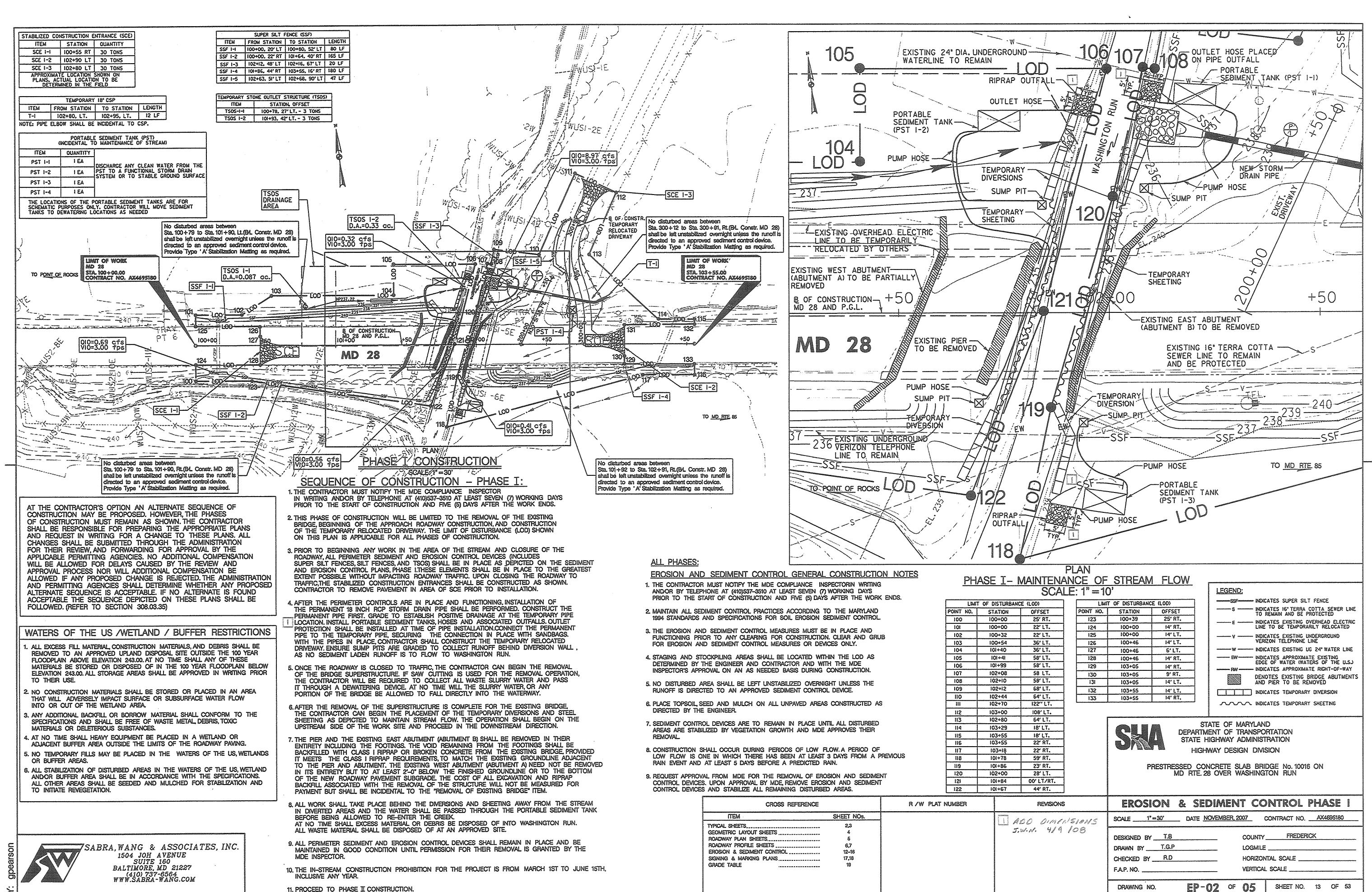
STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

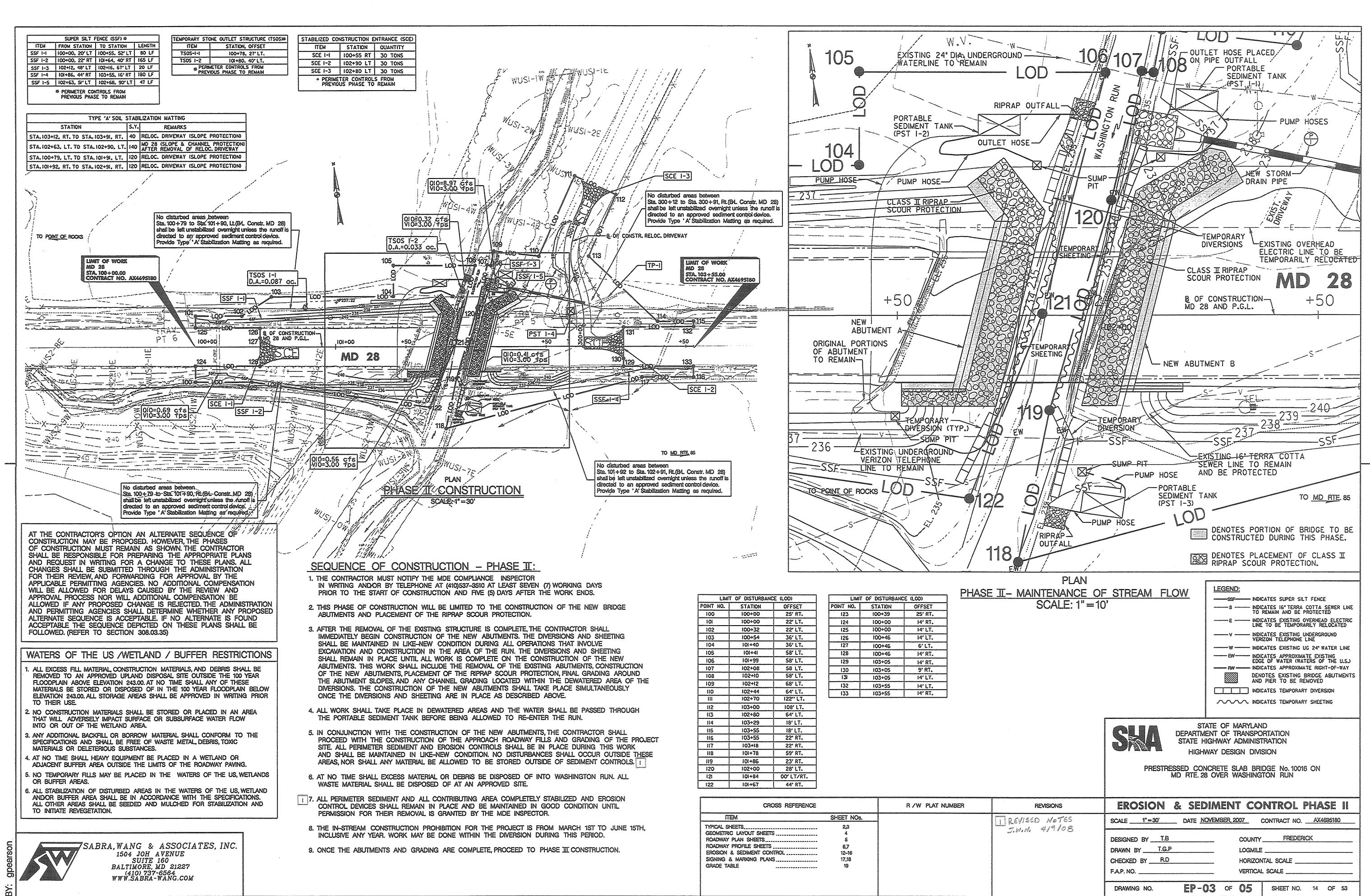
| CROSS REFERENCE |  | R /W PLAT NUMBER | REVISIONS  | EROSION AND SEDIMENT CONTROL NOTES   |
|-----------------|--|------------------|--|--|
| TYPICAL SHEETS  | SHEET NOs.  2,3  4  5  6,7  12-16  17,18  19   |                  | I ADD CERT.<br>J.W.N 4/9 108   | SCALE         N.T.S.         DATE         NOVEMBER, 2007         CONTRACT NO.         AX4695180           DESIGNED BY         T.B         COUNTY         FREDERICK           DRAWN BY         T.G.P         LOGMILE           CHECKED BY         R.D         HORIZONTAL SCALE           FAP. NO.         VERTICAL SCALE           DRAWING NO.         EP-01 OF 05         SHEET NO.         12 OF 53 |
|                 | ezanen erren errikorren erren er |                  | PLOTTED: Monday, February 11, 2008 AT 09:15 AM<br>FRE: R:\2003\22 BCS 2002-26G WMA Bridge\Task 1 | 11/CWG/pES-NC01_MD28.dgn   |

SABRA, WANG & ASSOCIATES, INC.

1504 JOH AVENUE
SUITE 160
BALTIMORE, MD 21227
(410) 737-6584
WWW.SABRA-WANG.COM

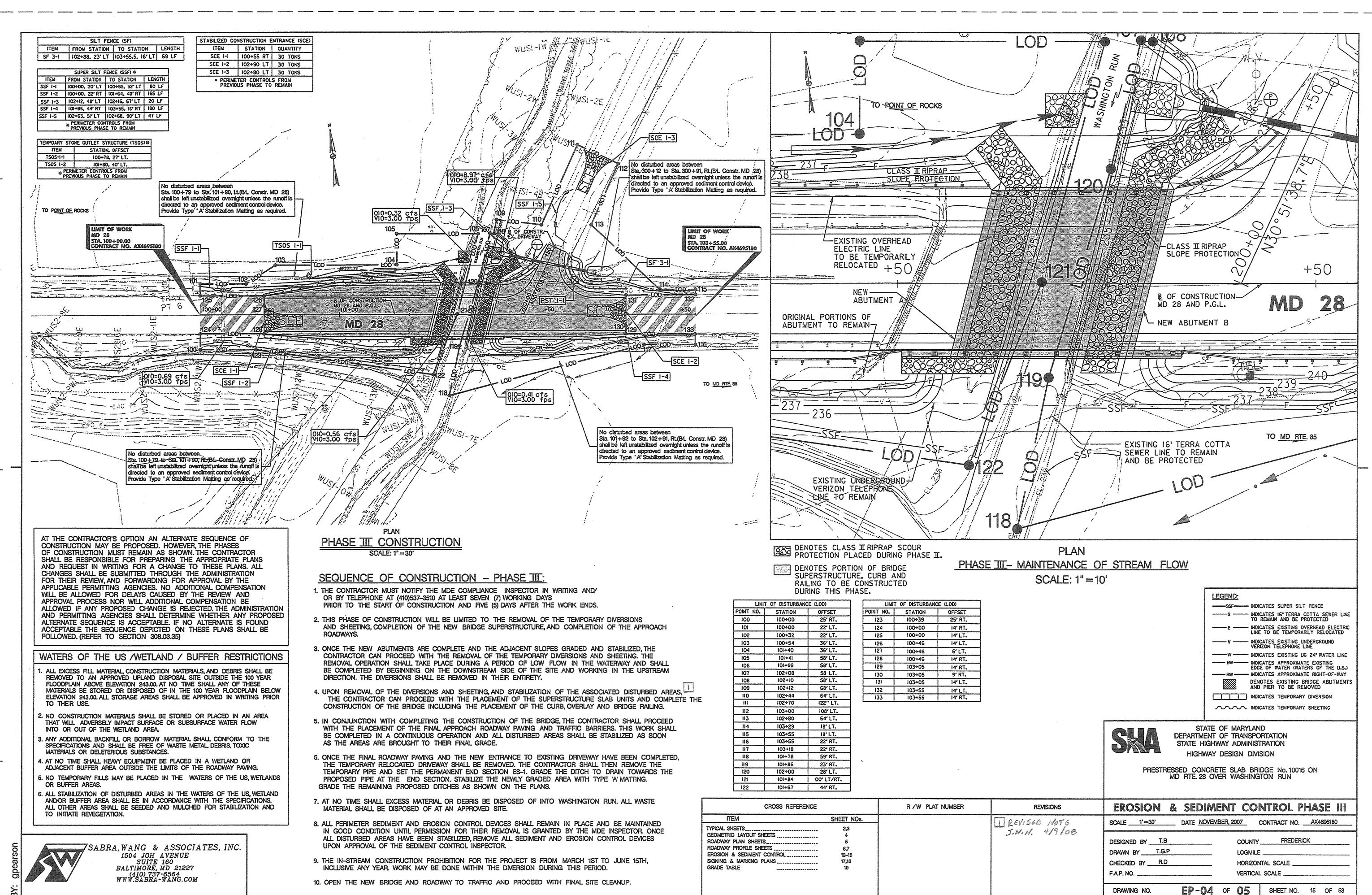


PLOTTED: Monday, February 11, 2008 AT 01:22 PM

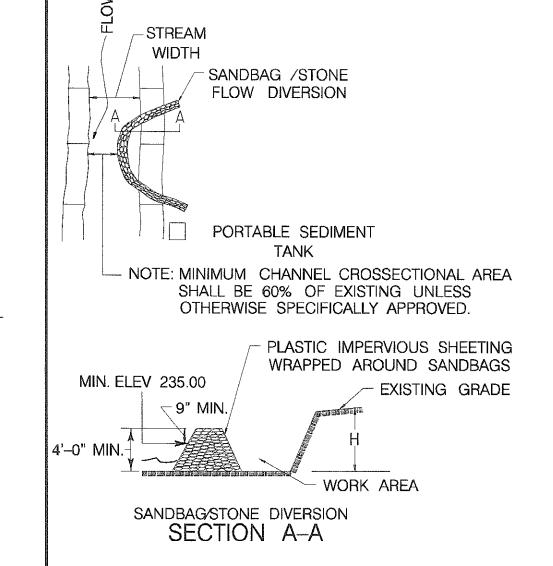


PLOTTED: Monday, February 11, 2008 AT 02:08 PM

FILE: R:\2003\22 BCS 2002-26G WMA Bridge\Task 11\DWG\pES-D201\_MD28.dgm



PLOTTED: Monday, February 11, 2008 AT 02:11 PM FILE: R:\2003\22 BCS 2002-26G WMA Bridge\Task 11\DWG\pES-D301\_MD23.dgn



NOTE: IN SMALL CHANNELS (NOT COVERED BY A

THAN THE LOWEST BANK ELEVATION.

WATERWAY CONSTRUCTION PERMIT) THE HEIGHT

OF THE DIVERSION WILL BE ONE FOOT HIGHER

### SANDBAG / STONE DIVERSION

### I. DESCRIPTION

- THE WORK SHALL CONSIST OF INSTALLING FLOW DIVERSIONS FOR THE PURPOSE OF EROSION CONTROL WHEN CONSTRUCTION ACTIVITIES TAKE PLACE WITHIN THE STREAM CHANNEL SUCH AS BANK STABILIZATION OR BRIDGE ABUTMENT CONSTRUCTION.

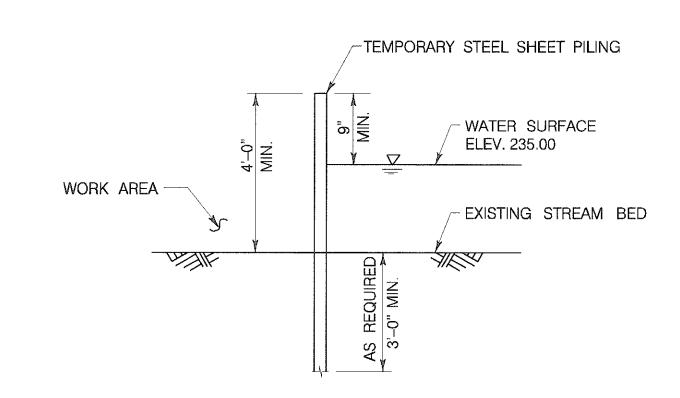
  II. MATERIAL SPECIFICATIONS
- 1. SANDBAGS: SANDBAGS SHALL CONSIST OF MATERIALS WHICH ARE RESISTANT TO ULTRA-VIOLET RADIATION, TEARING AND PUNCTURE AND WOVEN TIGHTLY ENOUGH TO PREVENT LEAKAGE OF FILL MATERIAL (I.E., SAND, FINE GRAVEL, ETC.).
- 2. STONE: STONE SHALL BE WASHED AND HAVE A MINIMUM DIAMETER OF 6
- 3. PLASTIC SHEETING: PLASTIC SHEETING SHALL CONSIST OF POLYETHYLENE OR OTHER MATERIAL WHICH IS IMPERVIOUS AND RESISTANT TO PUNCTURE AND TEARING.

  III. CONSTRUCTION REQUIREMENTS
- 1. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL BE INSTALLED AS THE FIRST ORDER OF WORK.
- 2. THE DIVERSION STRUCTURE SHALL BE INSTALLED FROM UPSTREAM TO
- DOWNSTREAM.

  3. THE HEIGHT OF THE DIVERSION STRUCTURE SHALL BE AS SHOWN.
- 4. ALL EXCAVATION MATERIALS SHALL BE DISPOSED OF IN AN <u>APPROVED</u>

  <u>DISPOSAL AREA</u> OUTSIDE THE 100-YEAR FLOODPLAIN UNLESS OTHERWISE

  APPROVED ON THE PLANS.
- 5. ALL DEWATERING OF THE CONSTRUCTION AREA SHALL BE PUMPED TO A PORTABLE SEDIMENT TANK PRIOR TO RE-ENTERING THE STREAM.
- 6. SHEETING SHALL BE OVERLAPPED SUCH THAT THE UPSTREAM PORTION COVERS THE DOWNSTREAM PORTION WITH AT LEAST AN 18-INCH OVERLAP.
- 7. SEDIMENT CONTROL DEVICES SHALL REMAIN IN PLACE UNTIL ALL DISTURBED AREAS ARE <u>STABILIZED</u> IN ACCORDANCE WITH AN APPROVED SEDIMENT AND EROSION CONTROL PLAN AND THE INSPECTING AUTHORITY APPROVES THEIR REMOVAL.



### NOTE:

THE DIVERSION SHALL BE PLACED SO THAT IT IS LOCATED WITHIN THE EXISTING SHA RIGHT-OF-WAY AND ALLOWS CONSTRUCTION, WITHIN THE DIVERSION AREA, TO BE COMPLETED IN THE DRY.

THE DIVERSION IS NOT INTENDED TO BLOCK HIGH WATER EVENTS FROM FLOODING THE DEWATERED AREAS.

ALL SHEETING SHALL BE IN NEW OR LIKE NEW CONDITION. ALL JOINTS BETWEEN ADJACENT SHEETS SHALL PROVIDE A POSITIVE CONNECTION AND WATER TIGHT SEAL. EITHER HOT ROLLED OR COLD FORMED STEEL SHEET PILING MAY BE USED.

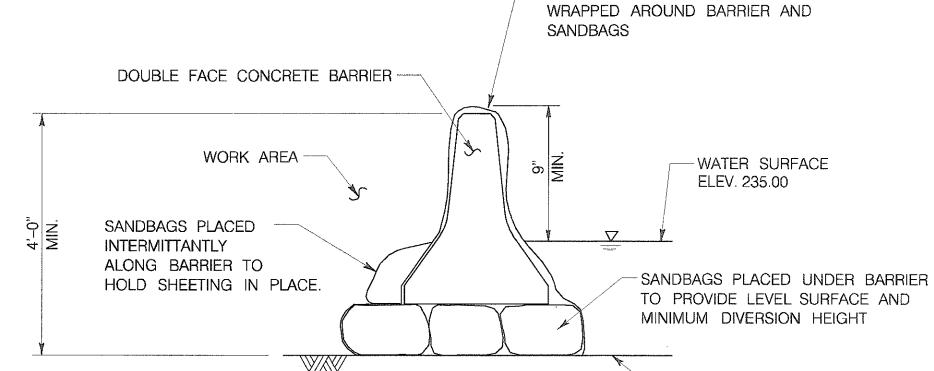
IMPERVIOUS PLASTIC SHEETING

- EXISTING STREAM BED

# SECTION <u>DIVERSION DETAIL USING TEMPORARY SHEETING OPTION</u> SCALE: 1/4"=1'-0"

NOTE:
THE CONTRACTOR SHALL HAVE THE OPTION OF USING ANY
OF THE DIVERSIONS SHOWN. IF THE CONTRACTOR ELECTS
TO USE ONE DIVERSION METHOD AND IS UNABLE TO DEWATER
THE SITE SUCCESSFULLY THERE WILL NOT BE COMPENSATION
FOR CHANGING TO AN ALTERNATE METHOD.

THE CHOICE OF DIVERSION AND DEWATERING METHOD SHALL BE AT THE CONTRACTOR'S RISK AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR THE CONTRACTOR'S INABILITY TO ADEQUATELY DEWATER THE AREA UNDER CONSTRUCTION. (REFER TO SP. 308.03.35)



# SECTION <u>DIVERSION DETAIL USING CONCRETE BARRIER OPTION</u>

SCALE:  $\frac{3}{4}$ " = 1'-0"

1. FOR GENERAL PLAN AND ELEVATION, SEE SHEET NO. S1-1

2. FOR MAINTENANCE OF STREAM FLOW, SEE SHEET NO'S. 13,14,15

3. FOR PORTABLE SENDIMENT TANK DETAILS, REFER TO THE LATEST MDE DESIGN STANDARDS.

4. PORTABLE SEDIMENT TANK WILL NOT BE MEASURED AND PAID FOR ON AN INDIVIDUAL BASIS BUT SHALL BE INCLUDED IN THE LUMP SUM PRICE FOR THE "MAINTENANCE OF STREAM FLOW" ITEM.

AT THE CONTRACTOR'S OPTION AN ALTERNATE SEQUENCE OF CONSTRUCTION MAY BE PROPOSED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PREPARING THE APPROPRIATE PLANS INCLUDING M.O.T. AND STREAM FLOW AND REQUEST IN WRITING FOR A CHANGE TO THESE PLANS. ALL CHANGES SHALL BE SUBMITTED THROUGH THE ADMINISTRATION FOR THEIR REVIEW, AND APPROVAL BY THE APPLICABLE PERMITTING AGENCIES. NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR DELAYS CAUSED BY THE REVIEW AND APPROVAL PROCESS NOR WILL ADDITIONAL COMPENSATION BE ALLOWED IF ANY PROPOSED CHANGE IS REJECTED. THE ADMINISTRATION AND PERMITTING AGENCIES SHALL BE THE SOLE JUDGE AS TO WHETHER ANY PROPOSED ALTERNATE SEQUENCE IS ACCEPTABLE. IF NO ALTERNATE IS FOUND ACCEPTABLE THE SEQUENCE DEPICTED ON THESE PLANS SHALL BE FOLLOWED. (REFER TO SP. 308.03.35)



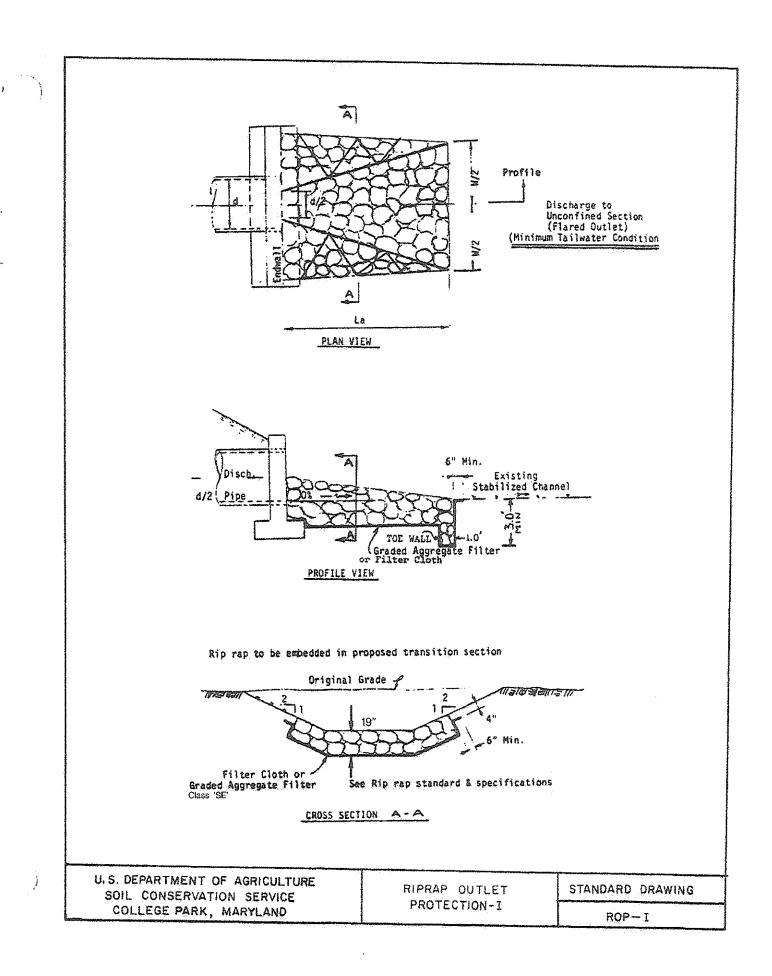
STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
HIGHWAY DESIGN DIVISION

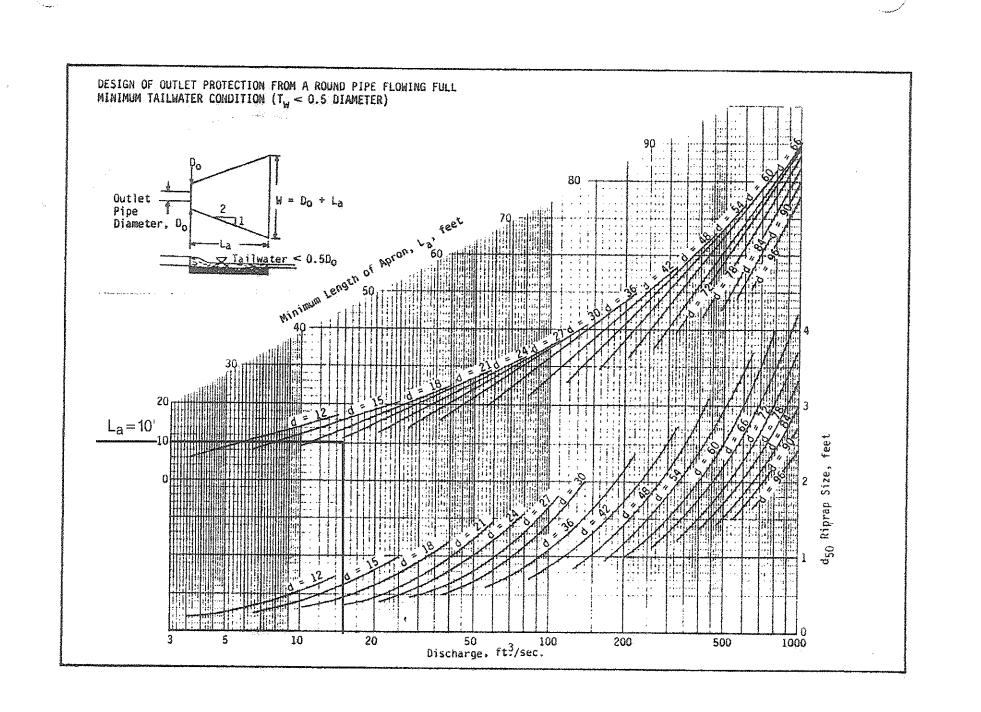
PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

MAINTENANCE OF STREAM FLOW - DETAILS CROSS REFERENCE R / W PLAT NUMBER REVISIONS ITEM SHEET NOs. SCALE DATE NOVEMBER, 2007 CONTRACT NO. AX4695180 TYPICAL SHEETS 2,3 GEOMETRIC LAYOUT SHEETS DESIGNED BY T.B FREDERICK COUNTY\_ ROADWAY PLAN SHEETS ... ROADWAY PROFILE SHEETS. 6,7 DRAWN BY LOGMILE EROSION & SEDIMENT CONTROL. 12-16 SIGNING & MARKING PLANS 17,18 CHECKED BY R.D HORIZONTAL SCALE GRADE TABLE JAN 0 8 2008 VERTICAL SCALE EP-05 of DRAWING NO. SHEET NO. 16 OF 53

# DIVERSION DETAIL USING SANDBAG OPTION

SCALE: NONE





TEMPORARY DIVERSION DEVICE OPTIONS

SABRA, WANG & ASSOCIATES, INC.
1504 JOH AVENUE
SUITE 160
BALTIMORE, MD 21227
(410) 737-6564
WWW.SABRA-WANG.COM

PLOTTED: Sunday, November 18, 2007 AT 08:52 AM
FILE: R:\2003\22 BCS 2002-26G WMA Bridge\Task 11\DWG\pES-D401\_md28.dgn

# SCHEDULE OF PROPOSAL ITEMS DESCRIPTION UNIT QUANTITY NO BRA, WANG & ASSOCIATES, INC.

1504 JOH AVENUE SUITE 160

BALTIMORE, MD 21227

(410) 737-6564 WWW.SABRA-WANG.COM

# GENERAL NOTES

### CRITERIA

THE CONTRACTOR SHALL BE GOVERNED BY THE STANDARDS AND REQUIREMENTS OF THE FOLLOWING PUBLICATIONS, EXCEPT AS MODIFIED BY THE SPECIAL PROVISIONS OF THIS CONTRACT.

### DESIGN

MDSHA - "MARYLAND MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", 2006 EDITION AND SUBSEQUENT REVISIONS. (MD M.U.T.C.D.)

### A A S H T O - "HIGHWAY SAFETY DESIGN AND OPERATIONS GUIDE" -1997

A A S H T O - "STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS LUMINAIRES AND TRAFFIC SIGNALS", 2001 EDITION (CATEGORY II FOR ALL OVERHEAD AND CANTILEVER SIGN STRUCTURES).

### MATERIALS AND CONSTRUCTION

MARYLAND STATE HIGHWAY ADMINISTRATION - "STANDARD SPECIFICATIONS FOR CONSTRUCTION & MATERIALS", 2001 EDITION AND SUBSEQUENT SUPPLEMENTS.

### DESIGN WIND

100 MPH - WOOD SUPPORTS 10 YEAR RECURRENCE INTERVAL

100 MPH - GROUND MOUNT SIGN STEEL SUPPORTS ALL DISTRICTS 10 YEAR RECURRENCE INTERVAL

100 MPH - OVERHEAD AND CANTILEVER STRUCTURES 50 YEAR RECURRENCE INTERVAL

### DESIGN STRESS

SOIL BEARING PRESSURE - S = 3,000 P.S.F. (ASSUMED) SEE MATERIAL & CONSTRUCTION ABOVE AND SPECIAL PROVISIONS FOR DESIGN STRESSES FOR STRUCTURAL STEEL, ALUMINUM,

# CHAMFER

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" X 3/4" CHAMFER.

### CLASSIFICATION OF SIGNS

SIGNS ARE DIVIDED INTO TWO (2) GENERAL CATEGORIES.

### 1 GUIDE SIGNS

A) STRUCTURAL TYPES

REINFORCING STEEL AND CONCRETE.

OH - OVERHEAD

C - CANTILEVER

GM - GROUND MOUNT, BREAKAWAY OR NON-BREAKWAY

BM - BRIDGE MOUNTED 2 STANDARD SIGNS (REGULATORY, WARNING, ETC.)

A) STRUCTURAL TYPES WOOD SUPPORTS GALVANIZED STEEL 'U' CHANNEL

# IDENTIFICATION OF SIGNS AND PANELS

### GUIDE SIGNS

EACH GUIDE SIGN IS IDENTIFIED BY A SIGN NUMBER ON THE PLANS AND IN THE TABULATIONS. PANELS ON GUIDE SIGNS ARE IDENTIFIED WITH A NUMBER AND WHERE VARIATIONS OCCUR, A LOWER CASE LETTER.

B) PANELS

B) PANELS

MATERIAL - EXTRUDED ALUMINUM

MATERIAL - SHEET ALUMINUM

COPY - NON-DEMOUNTABLE

EXISTING SIGNS)

1) BUTTON REFLECTOR (REVISIONS TO

2) HIGH INTENSITY (NEW SIGNS AND

REVISIONS TO EXISTING SIGNS)

COPY - DEMOUNTABLE

### STANDARD SIGNS

STANDARD SIGNS ARE IDENTIFIED BY PANEL NUMBERS AND ARE CLASSIFIED AS FOLLOWS

### R - REGULATORY W - WARNING

M - ROUTE MARKERS AND ACCESSORIES

D - DESTINATION AND MILEAGE PANELS

PANELS SHALL BE DESIGNATED TO AGREE WITH MARYLAND STANDARD SIGN BOOK.

### PANEL LAYOUT AND ALPHABETS

1. GUIDE SIGN PANEL LAYOUTS ARE BASED ON THE A.A.S.H.T.O. MANUALS NOTED ABOVE. 2. STANDARD SIGN PANEL LAYOUTS ARE BASED ON THE M.U.T.C.D. WITH SPECIFICATIONS DETAILED IN THE MARYLAND STATE HIGHWAY ADMINISTRATION PUBLICATION, "STANDARD SIGN BOOK", AVAILABLE THROUGH THE SHA CASHIER'S OFFICE.

### REFLECTORIZATION

BACKGROUNDS, BORDERS, TEXTS AND ALL OTHER ELEMENTS OF SIGN PANELS SHALL BE REFLECTORIZED EXCEPT WHERE NOTED.

### SIGN LOCATIONS

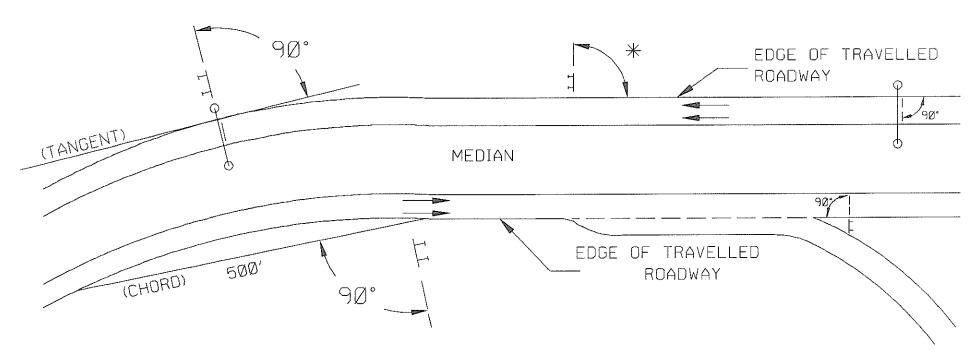
1. GUIDE SIGNS ARE LOCATED ON THE PLANS BY DIMENSION TO SURVEY STATIONS,

OR WHEN NECESSARY, TO IDENTIFIABLE PHYSICAL FEATURES.

2. ALL CHANGES IN THE LOCATIONS OF SIGNS AS SHOWN ON THE PLAN SHALL HAVE THE PRIOR APPROVAL OF THE ENGINEER.

# EXISTING UTILITIES

THE ENGINEER DOES NOT WARRANT OR GUARANTEE THE ACCURACY OR COMPLETENESS OF INFORMATION SHOWN ON THE PLAN. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE AND PROTECT ALL EXISTING FACILITIES WHICH MIGHT BE AFFECTED BY THIS WORK OR HIS OPERATION.



\* UNDER 30 FEET FROM TRAVELLED ROADWAY TO NEAR EDGE OF SIGN - 93° TO AVOID SPECULAR REFLECTION AS INDICATED IN 813,03 OF THE MARYLAND STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS.

ORIENTATION OF SIGN FACES

OVER 30 FEET FROM TRAVELLED ROADWAY TO NEAR EDGE OF SIGN - 90°

### ROADSIDE SIGNS

1. VERTICAL ALIGNMENT

POSITION PANEL SO FACE IS PLUMB.

2. HORIZONTAL ALIGNMENT (SEE DIAGRAM ABOVE)

NORMAL EDGE OF THE MAINLINE ROADWAY.

A). ON STRAIGHT ROADWAY SECTIONS, ANGLE OF SIGN FACE TO ROADWAY VARIES WITH DISTANCE FROM TRAVELLED ROADWAY TO NEAR EDGE OF SIGN - SEE DIAGRAM. B). ON THE INSIDE OF HORIZONTAL CURVES, POSITION SIGN SO FACE OF PANEL MAKES AN ANGLE OF 90° WITH A CHORD BETWEEN A POINT ON NEAR EDGE OF PAVEMENT AT SIGN LOCATION AND A POINT ON EDGE OF PAVEMENT 500' IN ADVANCE OF SIGN.

C). ON THE OUTSIDE OF HORIZONTAL CURVES, POSITION SIGN SO FACE OF PANEL IS AT RIGHT ANGLES TO THE TANGENT OF THE CURVE AT THE SIGN LOCATION. D.) POSITIONING OF SIGNS AT GORES AND RAMP SEPARATIONS IS REFERRED TO THE

### OVERHEAD SIGNS

1. VERTICAL ALIGNMENT

POSITION PANELS FOR ALL OVERHEAD STRUCTURES SO THAT PANEL FACE IS PLUMB.

2. OVERHEAD SIGN STRUCTURES SHALL NOT BE ERECTED WITHOUT

ATTACHING LUMINARIES SUPPORTS AND/OR SIGN. 3. HORIZONTAL ALIGNMENT

A). POSITION ALL OVERHEAD SIGNS SO THAT THE FACE OF THE PANEL IS AT RIGHT ANGLES TO THE NORMAL EDGE OF ROADWAY, IF ON A STRAIGHT ROADWAY SECTION.

B). POSITION ALL OVERHEAD SIGNS SO THAT THE FACE OF THE PANEL IS AT RIGHT ANGLES TO THE TANGENT OF THE CURVE AT SIGN LOCATION, IF ON A HORIZONTAL CURVE. C). POSITIONING OF SIGNS AT GORES AND RAMP SEPARATIONS IS REFERRED TO THE NORMAL

4. VERTICAL CLEARANCE

EDGE OF THE MAINLINE ROADWAY.

A). OVERHEAD SIGNS SHALL HAVE A MINIMUM VERTICAL CLEARANCE OF 17'-9" FROM ROADWAY TO THE BOTTOM OF LIGHT FIXTURES. ALL LIGHT FIXTURES ARE TO BE AT THE SAME ELEVATION ONLY ON AESTHETIC STRUCTURES.

B). IF THE CONTRACTOR CANNOT OBTAIN 17'-9" (SEE 3A) CLEARANCE, HE IS TO CEASE WORK AND CONTACT THE PROJECT ENGINEER FOR FURTHER INSTRUCTIONS. THE PROJECT ENGINEER MAY CONTACT THE TRAFFIC ENGINEERING DIVISION FOR ASSISTANCE.

C). ON UNLIT OVERHEAD SIGNS, THE MINIMUM CLEARANCE TO BOTTOM OF SIGN: 20'-9".

### PROJECT REQUIREMENTS

1. ALL NEW SIGNS ON THIS PROJECT ARE TO HAVE NON-REFLECTIVE (BLACK COPY) OR HIGH-INTENSITY REFLECTIVE (ALL OTHER COLORS) SHEETING BACKGROUND AND COPY. REFLECTIVE SHEETING SHALL BE TYPE III ENCAPSULATED LENS REFLECTIVE ELEMENT MATERIAL.

2. ALL NEW EXTRUDED ALUMINUM PANELS ARE TO HAVE DEMOUNTABLE COPY.

3. ALL NEW SHEET ALUMINUM SIGNS ARE TO HAVE NON-DEMOUNTABLE COPY.

4. THE FOLLOWING MINIMUM THICKNESS SHALL BE USED FOR THE APPROPRIATE WIDTH OF SHEET ALUMINUM BLANKS.

| LONGEST DIMENSION       | MINIMUM THICKNESS |
|-------------------------|-------------------|
| UP TO 12"               | Ø.Ø4Ø"            |
| GREATER THAN 12" TO 24" | Ø.Ø63"            |
| GREATER THAN 24" TO 36" | Ø.080"            |
| GREATER THAN 36" TO 48" | Ø.100"            |
| OVER 48"                | Ø.125"            |



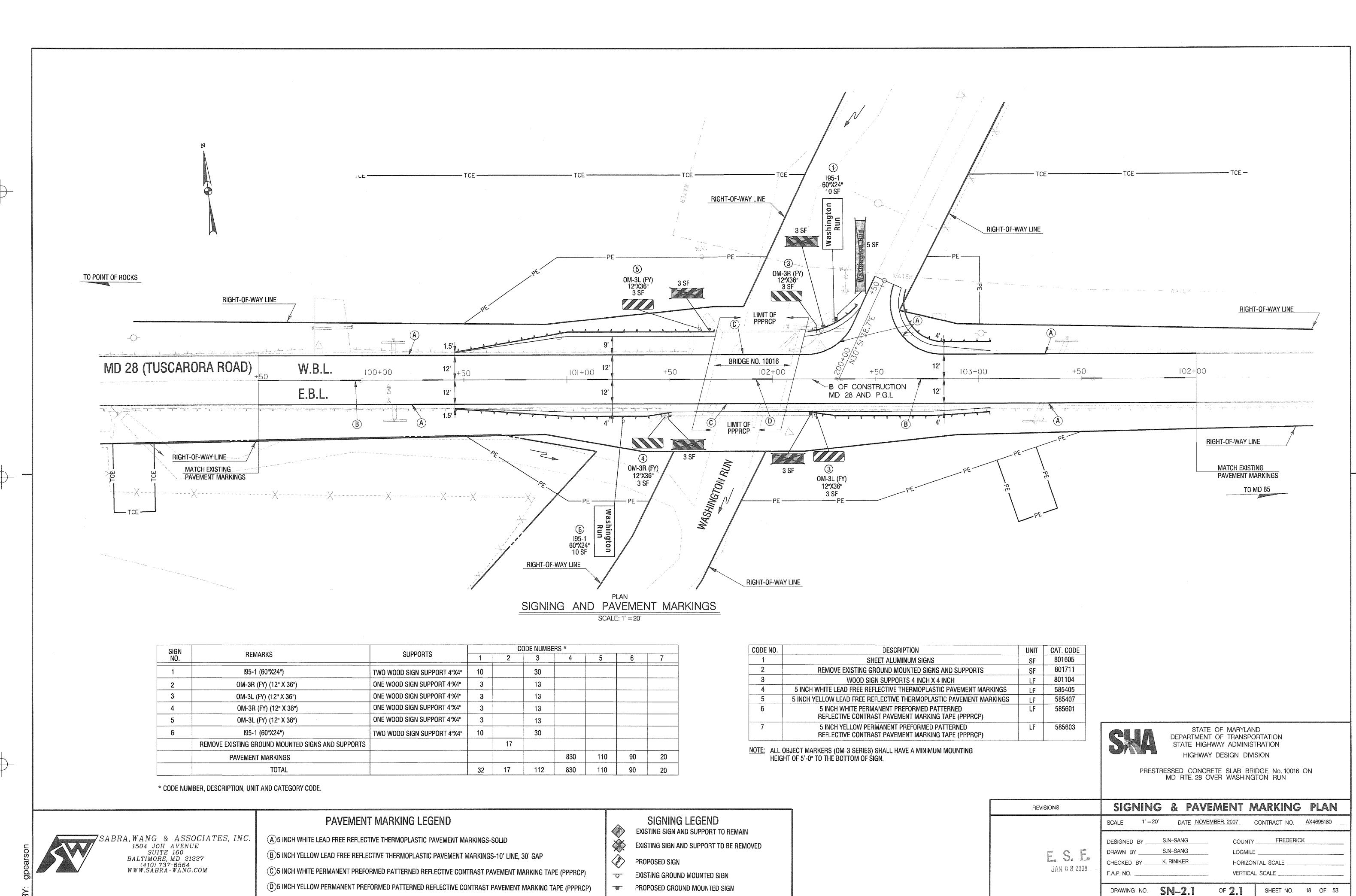
STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON

MD RTE. 28 OVER WASHINGTON RUN

| H <u>ALL HAVE THE</u>  | ·                 |                  |               |   |  |  |  |  |  |  |  |
|--|-------------------|------------------|---------------|---|--|--|--|--|--|--|--|
| CROSS RI   | REFERENCE         | R /W PLAT NUMBER | REVISIONS     | GENERAL NOTES AND PROPOSALS                   |  |  |  |  |  |  |  |
| ITEM  TYPICAL SHEETS  GEOMETRIC LAYOUT SHEETS  ROADWAY PLAN SHEETS  ROADWAY PROFILE SHEETS  EROSION & SEDIMENT CONTROL | 5<br>6,7<br>12–16 |                  |               | DESIGNED BY T.B.                              | VEMBER, 2007 CONTRACT NO. AX4695180  COUNTY FREDERICK  LOGMILE |  |  |  |  |  |  |
| SIGNING & MARKING PLANS<br>GRADE TABLE   | 17,18<br>19       |                  | JAN. 0.8.2008 | CHECKED BY R.D  F.A.P. NO.  DRAWING NO. SN-1- | HORIZONTAL SCALE   |  |  |  |  |  |  |

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|           | GRADING TABLE |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|-----------|---------------|--------------------|----------------|--------|--------|-----------------------------|--------|--------------------------------------|--|------------------------|--------------------------|-----------|--------------------|------------|------------------|----------|--------------|----------|----------------------------|------------------------|--|--------------------|----------|
| LOCA      | TION          |                    |                |        |        |                             |        |                                      |  |                        |                          |           | EMBANKMENT         | EMBANKMENT |                  |          |              |          |                            |                        |  |                    |          |
| STAT      | IONS          | CUT FDOM           | T FROM TOPSOIL |        |        | CLASS I ROOTMAT TOTAL UNUSA |        |                                      |  |                        |                          | CLASS IA  |                    |            |                  |          | CLASS 2      |          |                            |                        |  | FILL               | COMMENTS |
| FROM      | ТО            | CUT FROM<br>XSECTS | CUT            | FILL   | CUT    | FILL                        | TOTAL  | UNUSABLE<br>CUT UNDER<br>EX. ROADWAY | FOR<br>FOR<br>EMBANK                   | SHRINK/SWELL<br>FACTOR | AVAIL.<br>FOR<br>EMBANK. | (ASSUMED) | TOTAL FROM X-SECTS | CUT        | SOIL  <br>  FILL | CUT      | TMAT<br>FILL | TOTAL    | SUITABLE<br>FOR<br>EMBANK. | SHRINK/SWELL<br>FACTOR | AVAIL.<br>FOR<br>EMBANK.   | FROM<br>X-SECTIONS |          |
| 1         | 2             | ③ C.Y.             | (4) C.Y.       | ⑤ C.Y. | ⑥ C.Y. | 7 C.Y.                      | 8 c.y. | 9 C.Y.                               | (i) C.Y.                               | ① C.Y.                 | (2) C.Y.                 | C.Y.      | (3) C.Y.           | (4) C.Y.   | (5) C.Y.         | (6) C.Y. | (7) C.Y.     | (8) C.Y. | (9) C.Y.                   | 20 c.y.                | ② C.Y.   | 2 C.Y.             |          |
| MD 28 / I |               | 515                | 0              | 0      | 0      | 0                           | 515    | 320                                  | 195                                    | 85%                    | 166                      | 50        | 0                  | 0          | 0                | 0        | 0            | 0        | 0                          | 0                      | 0  | 370                |          |
| / RELOC.  | TEMP. DW      |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|           |               |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|           |               |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|           |               |                    |                |        |        |                             |        |                                      | ************************************** |                        |                          |           |                    |            |                  |          |              |          |                            |                        | Vermont Hardway Vermont Hardwa |                    |          |
|           |               |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|           |               |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
|           |               |                    |                |        |        |                             |        |                                      |  |                        |                          |           |                    |            |                  |          |              |          |                            |                        |  |                    |          |
| TOT       | ALS           | 515                | O              | 0      | 0      | 0                           | 515    | 320                                  | 195                                    | 85%                    | 166                      | 50        | 0                  | 0          | 0                | 0        | 0            | 0        | 0                          | 0                      | 0  | 370                |          |

# SUMMARY OF EARTHWORK

### CLASS '1' EXCAVATION CHT EDOM CDOSS\_SECTIONS (3)

| CUT FROM CROSS-SECTIONS 3   | 515 | C.Y. |
|---|-----|------|
| PLUS TOPSOIL REMOVED UNDER FILL (5)   | +0  | C.Y. |
| PLUS ROOTMAT REMOVED UNDER FILL 7   | +0  | C.Y. |
| TOTAL CLASS 'I' EXCAVATION 8  | 515 | C.Y. |
| MINUS TOPSOIL REMOVED IN CUT 4  | -0  | C.Y. |
| MINUS TOPSOIL REMOVED UNDER FILL 5  | -0  | C.Y. |
| MINUS ROOTMAT REMOVED IN CUT 6  | -0  | C.Y. |
| MINUS ROOTMAT REMOVED UNDER FILL (7)  | -0  | C.Y. |
| MINUS UNUSABLE CUT UNDER EX. ROADWAY 9  | 320 | C.Y. |
| TOTAL CLASS 11' EXCAVATION (1)  | 195 | C.Y. |
| SUITABLE FOR EMBANKMENT  TOTAL ADJUSTED CLASS 'I' EXCAVATION (2) —— —— ——  AVAILABLE FOR EMBANKMENT (SHRINKAGE FACTOR 85%*) | 166 | C.Y. |

### CIL V CC 300 HAZALA V V V V VILLA CO

| CLASS '2' EXCAVATION   |    |      |
|--|----|------|
| CUT FROM CROSS-SECTIONS (3)                                      | 0  | C.Y. |
| PLUS TOPSOIL REMOVED UNDER FILL (5)                              | +0 | C.Y. |
| PLUS ROOTMAT REMOVED UNDER FILL (1)                              | +0 | C.Y. |
| TOTAL CLASS '2' EXCAVATION (8)                                   | 0  | C.Y. |
| MINUS TOPSOIL REMOVED IN CUT (4)                                 | -0 | C.Y. |
| MINUS TOPSOIL REMOVED UNDER FILL (5)                             | -0 | C.Y. |
| MINUS ROOTMAT REMOVED IN CUT (6)                                 | -0 | C.Y. |
| MINUS ROOTMAT REMOVED UNDER FILL (17)                            | -0 | C.Y. |
| TOTAL CLASS '2' EXCAVATION (9                                    | 0  | C.Y. |
| SUITABLE FOR EMBANKMENT  TOTAL ADJUSTED CLASS '2' EXCAVATION (2) | 0  | C.Y. |

### EXCAVATION AVAILABLE FOR EMBANKMENT

| TOTAL CLASS 'I' EXCAVATION AVAILABLE FOR EMBANKMENT | © — — –          | 166 | C.Y. |
|---|------------------|-----|------|
| TOTAL CLASS '2' EXCAVATION AVAILABLE FOR EMBANKMENT | ② — — — <u>+</u> | 0   | C.Y. |
| TOTAL EXCAVATION ——<br>AVAILABLE FOR EMBANKN        |                  | 166 | C.Y. |

### EMBANKMENT REQUIRED

| FILL FROM CROSS-SECTIONS (2)                                 | <del> 370</del> | C.Y |
|--|-----------------|-----|
| PLUS TOPSOIL REMOVED UNDER FILL (CLASS 'I' EXCAVATION) (5) - | +0              | C.Y |
| PLUS ROOTMAT REMOVED UNDER FILL (CLASS 'I' EXCAVATION) 7 -   |                 | C.Y |
| PLUS TOPSOIL REMOVED UNDER FILL (CLASS '2' EXCAVATION) (5) - | +0              | C.Y |
| PLUS ROOTMAT REMOVED UNDER FILL (CLASS '2' EXCAVATION)       | - +0            | C.1 |
| TOTAL EMBANKMENT REQUIRED                                    | - 370           | C.Y |
| TOTAL EXCAVATION (AVAILABLE FOR EMBANKMENT)                  | - <b>-</b> 166  | C.Y |
| BORROW REQUIRED  | - 204           | C.Y |
| BORROW, DENSIFICATION (19%)                                  | 39              | C.Y |
| TOTAL BORROW REQUIRED  | 243             | C.Y |

- I. ALL BORROW SHALL CONFORM TO SECTION 203.
- 2. TOPSOIL SHALL BE FURNISHED FOR USE ON THIS PROJECT.

ABRA, WANG & ASSOCIATES, INC.

A,WANG & ASSOCIA 1504 JOH AVENUE SUITE 160 BALTIMORE, MD 21227 (410) 737-6564 WWW.SABRA-WANG.COM

- 3. CLASS 'I' EXCAVATION INCLUDES ALL EXCAVATION WHERE THE WIDTH OF THE BOTTOM OF THE CUT IS 15 FT OR MORE.
- 4. CLASS 'I-A' EXCAVATION IS ASSUMED AND, IF REQUIRED, SHALL BE BACKFILLED WITH GEOSYNTHETIC STABILIZED SUBGRADE USING GRADED AGGREGATE BASE CONFORMING TO SECTION 211 OR OTHER SUITABLE MATERIAL FROM CLASS I EXCAVATION AS APPROVED BY THE ENGINEER AT THE TIME OF CONSTRUCTION.
- 5. THE CONTRACTOR SHALL REMOVE "RELOCATED TEMPORARY DRIVEWAY" AND REGRADE TO ENSURE POSITIVE DRAINAGE PRIOR TO OPENING ROADWAY TO TRAFFIC.

# PROPOSAL QUANTITIES

| CLASS I EXCAVATION —— —— —— —— ——  | 515 | C.Y |
|------------------------------------|-----|-----|
| REMOVAL OF "RELOCATED              |     |     |
| TEMPORARY DRIVEWAY"                | 40  | C.Y |
| TOTAL CLASS   EXCAVATION           | 555 | C.Y |
| CLASS IA EXCAVATION —— —— —— —— —— | 50  | C.Y |
| COMMON BORROW                      | 243 | C.Y |
|                                    |     |     |



DRAWING NO.

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION HIGHWAY DESIGN DIVISION

PRESTRESSED CONCRETE SLAB BRIDGE No. 10016 ON MD RTE. 28 OVER WASHINGTON RUN

### GRADING TABLE

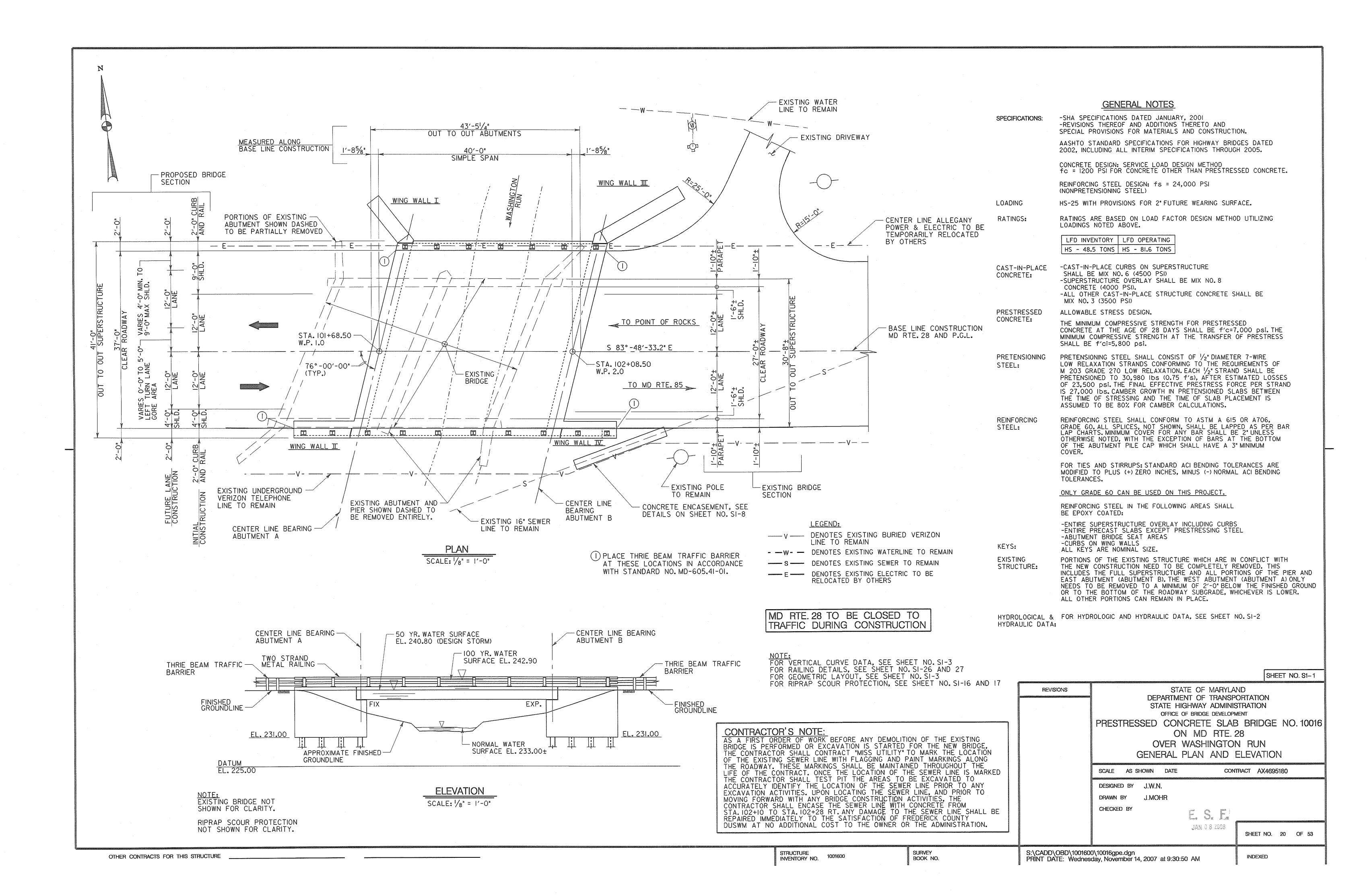
GT-01 OF 01

SCALE N.T.S. DATE NOVEMBER, 2007 CONTRACT NO. AX4695180 COUNTY FREDERICK DESIGNED BY \_\_\_\_\_T.B DRAWN BY T.G.P CHECKED BY R.D HORIZONTAL SCALE F.A.P. NO. VERTICAL SCALE \_

SHEET NO. 19 OF 53

JAN 0 8 2006 -

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# HYDROLOGIC DATA

\_\_DATE: <u>MAY\_2005</u>

SQUARE MILES 2.5

I. SOURCE: HYDROLOGIC ANALYSIS REPORT FOR MD 28 OVER WASHINGTON RUN

PREPARED BY: ⊠ SHA □ CONSULTANT: \_

II. DRAINAGE AREA:

III. METHOD(S) OF ANALYSIS:

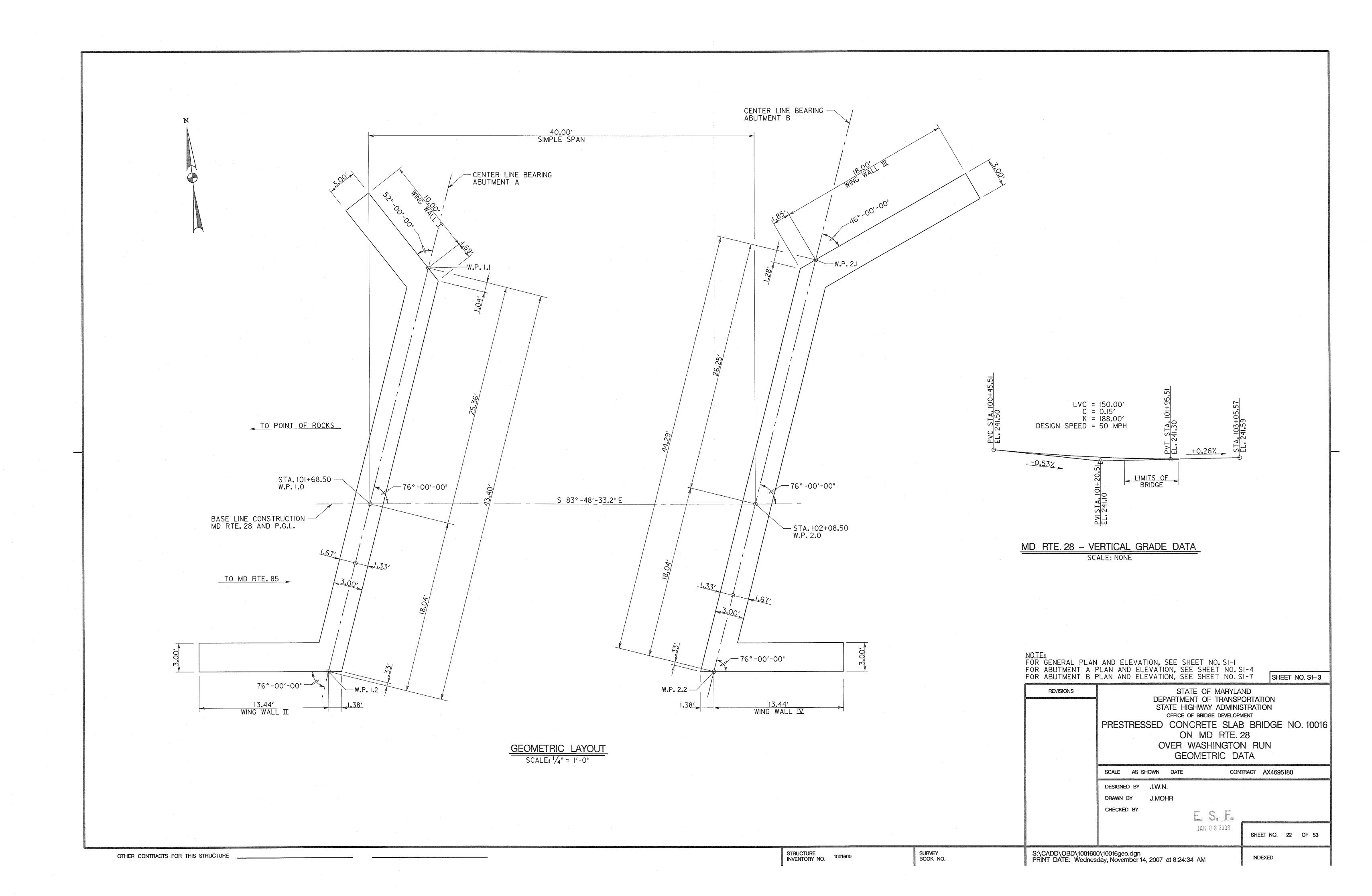
USGS GAGE DATA ANALYSISGAGING STATION NO.

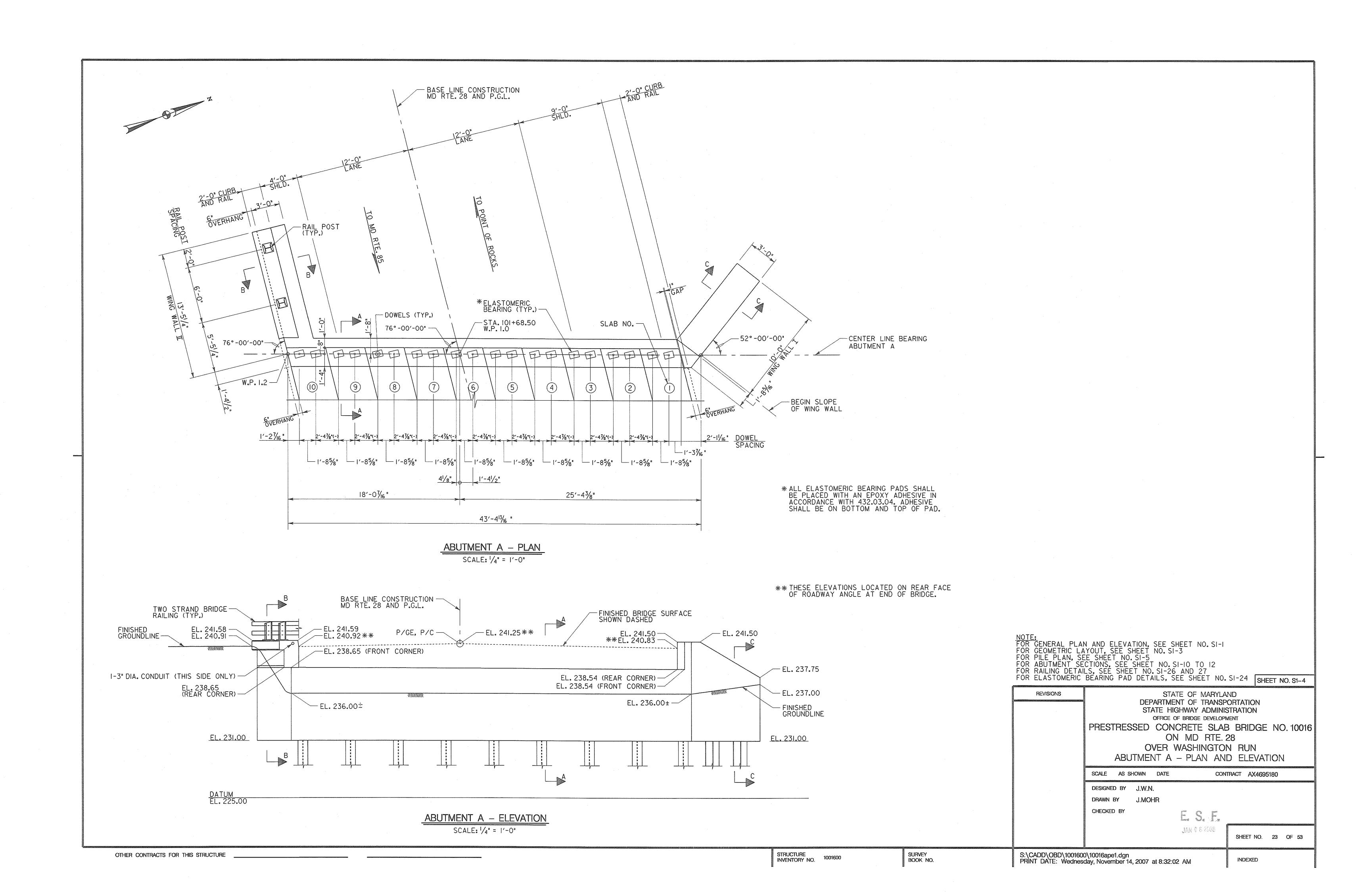
FILE LOCATION: STATE HIGHWAY ADMINISTRATION

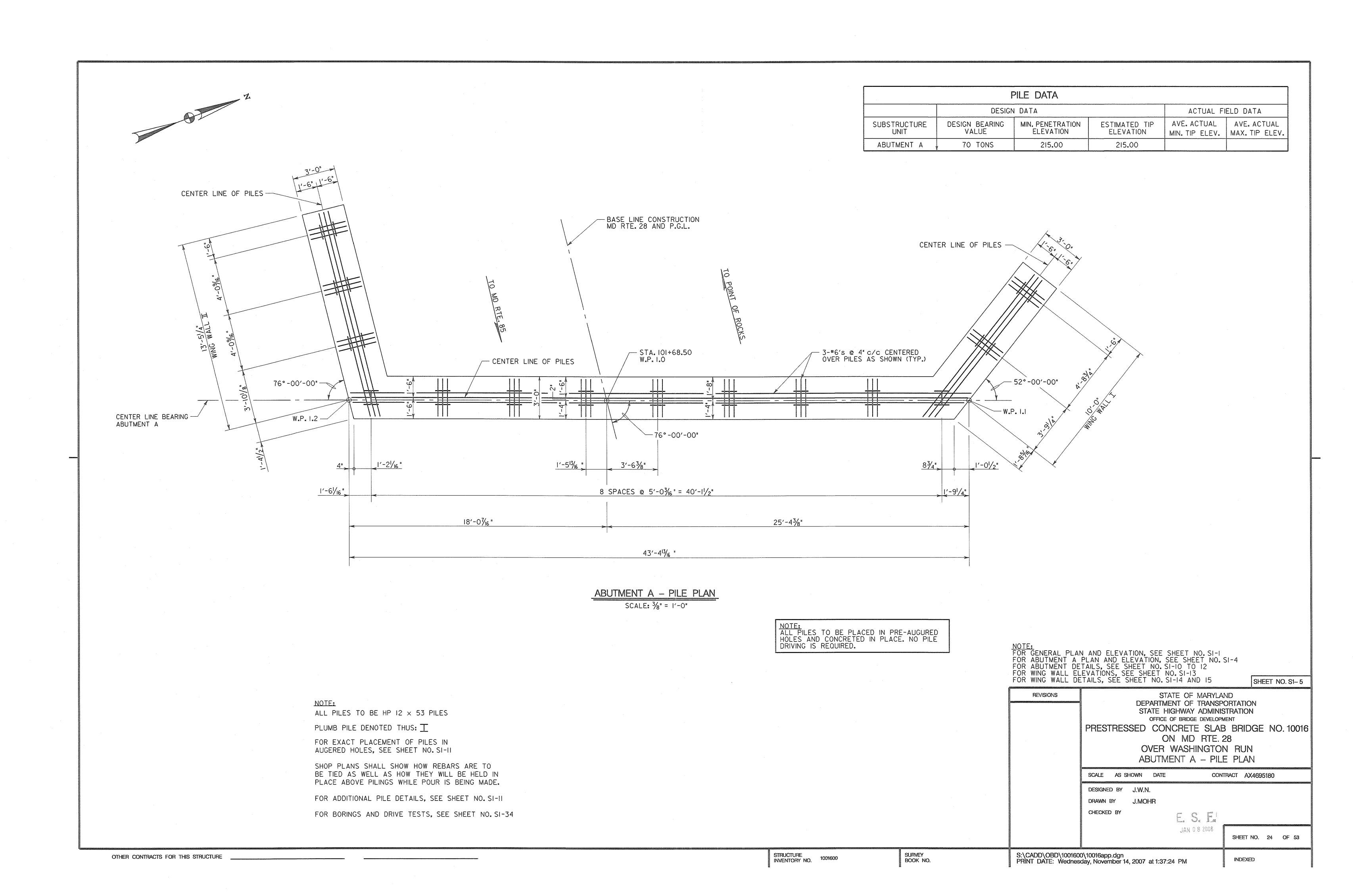
|   | NUOUS RECORD  |  |
|---|---|--|
| USGS REGRESSION   | EQUATIONS   |  |
|   |   | APPLICATION OF HYDROLOGY METHODS IN MD.  |
| <ul> <li>RCN (EXISTING-H</li> </ul>   | OMOGENEOUS WATERSHED) 64  |  |
| <ul> <li>RCN (UI TIMATE)</li> </ul>   | HOMOGENEOUS WATERSHED) 64   |  |
|   |   | (CFS) METHOD USED BY FEMA  |
|   | TEATY DISCHARGE   |  |
| HAS FLOOD ROUTING BEEN  | USED IN DETERMINING FLOOD   |  |
| DISCHARGES?   | YES $igthered{	imes}$ NO $igcup_{}$   |  |
| METHOD SELECTEDAT-  | IN METHOD   |  |
| /. COMPUTED FLOOD   | DISCHARGES  |  |
|   | FL  | OOD DISCHARGE  |
| RETURN PERIOD (YEARS)   | BASED ON EXISTING   | (CFS)  BASED ON ULTIMATE  WATERSHED DEVELOPMENT  |
|   | WATERSHED DEVELOPMENT   |  |
| 2   | 72<br>250   | 82   |
| 25  | 790   | 820  |
| 50  | 1100  | 1160   |
| 100   | 1500  | 1560   |
| 500   |   |  |
| LUOTODIO EL OCOC  | A.L. / A  |  |
| HISTORIC FLOODS   | N/A   |  |
| I. STREAM MORPHOL   | _OGY  |  |
|   | <del>-</del> "  | LLEY TYPETERRACED_ALLUVIAL   |
|   | · · · · · · · · · · · · · · · · · · ·   | торы (— )   1   1   1   1   1   1   1   1   1  |
| STREAM BED MATERIAL:  | DAVE  | 4 MM   |
| DESCRIPTIONG  | AAVEL DI6_  | 4 MM D50 15 MM D84 33 MM   |
| THIS OCCURS AT THE CSX  | LOGY - A 0.5 SQ. MIPORTION OF THE DR<br>CROSSING OF TUSCARORA CREEK - I MIL   | AINAGE AREA FLOWS TO A BLOCKED RAILROAD CROSSING UPSTREAM FROM BRIDGE 1001600. THE DETERMINATION OURING STORM EVENTS AND WAS REMOVED FROM THE  |
| ANALYSIS. THE TOTAL ARE   | A CONTRIBUTING DURING STORM EVENTS  | IS MODELED AS 2.0 SQ. MI A SUB-DIVIDED TR-20 MODE  |
| WAS DEVELOPED WITH A T  |   |  |
| AREA. THE DRAINAGE AREA   | OTAL OF 4 SUB-WATERSHEDS. THE REMA  |  |
|   |   | INING THREE AREA COMPRISE THE 2.0 SQ. MI. CONTRIBUT  |
| GLENN MOGLEN OF THE MD  | WAS FOUND TO CONTAIN 24% LIMESTONE  | NING THREE AREA COMPRISE THE 2.0 SQ. MI. CONTRIBUT   |
|   | WAS FOUND TO CONTAIN 24% LIMESTONE HYDROLOGY PANEL, ABOUT THE ACCURA  | INING THREE AREA COMPRISE THE 2.0 SQ. MI. CONTRIBUTE COVERAGE. THERE IS UNCERTAINTY. ACCORDING TO DRACY OF THE LOCATION OF THIS LIMESTONE. A PROCEDU   |
|   | WAS FOUND TO CONTAIN 24% LIMESTONE  | INING THREE AREA COMPRISE THE 2.0 SQ.MI.CONTRIBUTE COVERAGE. THERE IS UNCERTAINTY. ACCORDING TO DR.  |
| WAS FOLLOWED AND OUTLI  | WAS FOUND TO CONTAIN 24% LIMESTONE HYDROLOGY PANEL, ABOUT THE ACCURA NED IN THE REPORT TO ACCOUNT FOR T   | INING THREE AREA COMPRISE THE 2.0 SQ.MI.CONTRIBUTE COVERAGE. THERE IS UNCERTAINTY, ACCORDING TO DR. ACCY OF THE LOCATION OF THIS LIMESTONE. A PROCEDULY HIS UNCERTAINTY.   |
| WAS FOLLOWED AND OUTLI  | WAS FOUND TO CONTAIN 24% LIMESTONE HYDROLOGY PANEL, ABOUT THE ACCURA NED IN THE REPORT TO ACCOUNT FOR T   | INING THREE AREA COMPRISE THE 2.0 SQ.MI. CONTRIBUTE COVERAGE. THERE IS UNCERTAINTY. ACCORDING TO DR. ACCY OF THE LOCATION OF THIS LIMESTONE. A PROCEDUMENTAL STATE OF THE LOCATION OF THE EXISTING BRIDGE NO. 1001600  |
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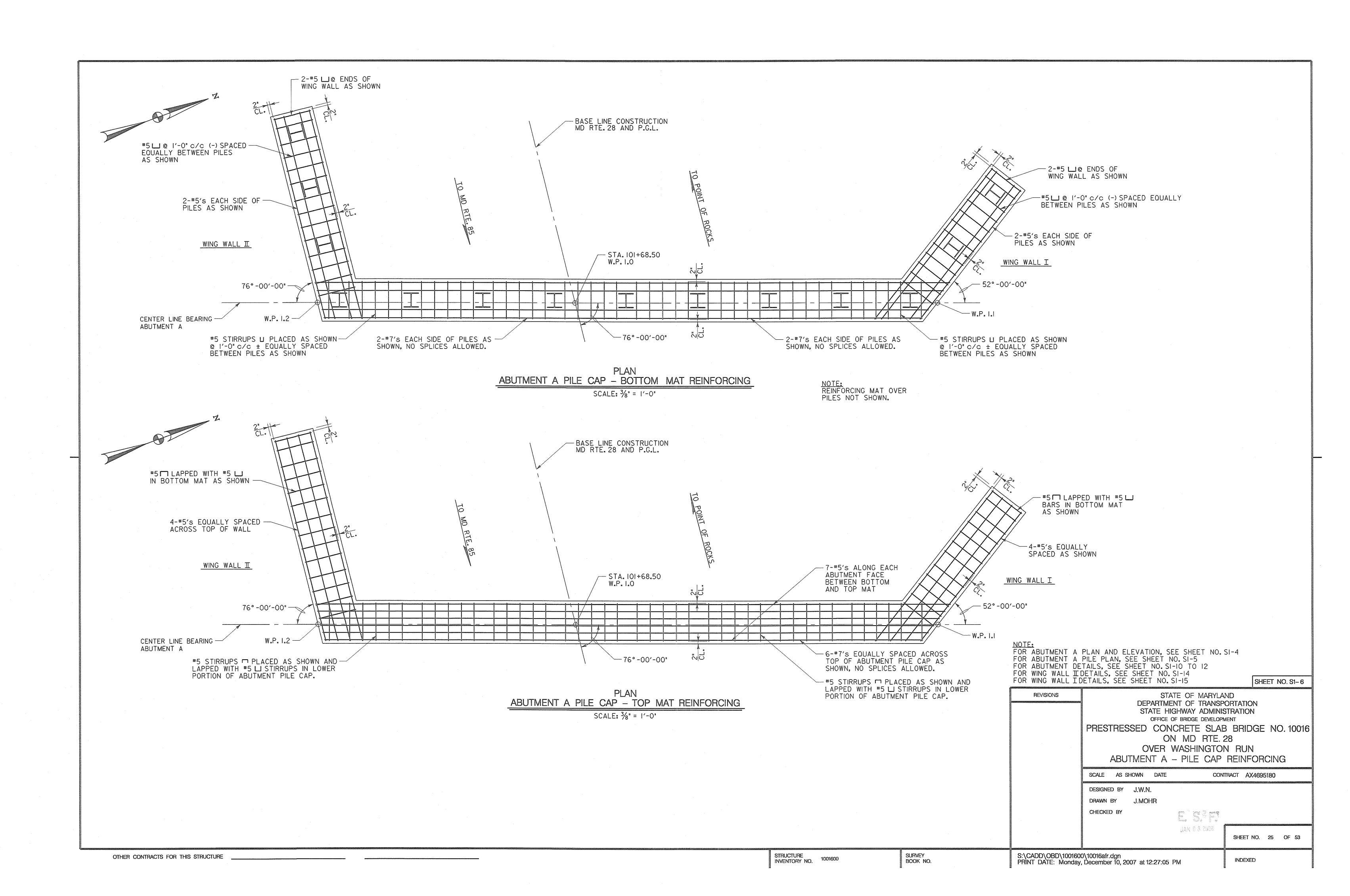
# HYDRAULIC DATA

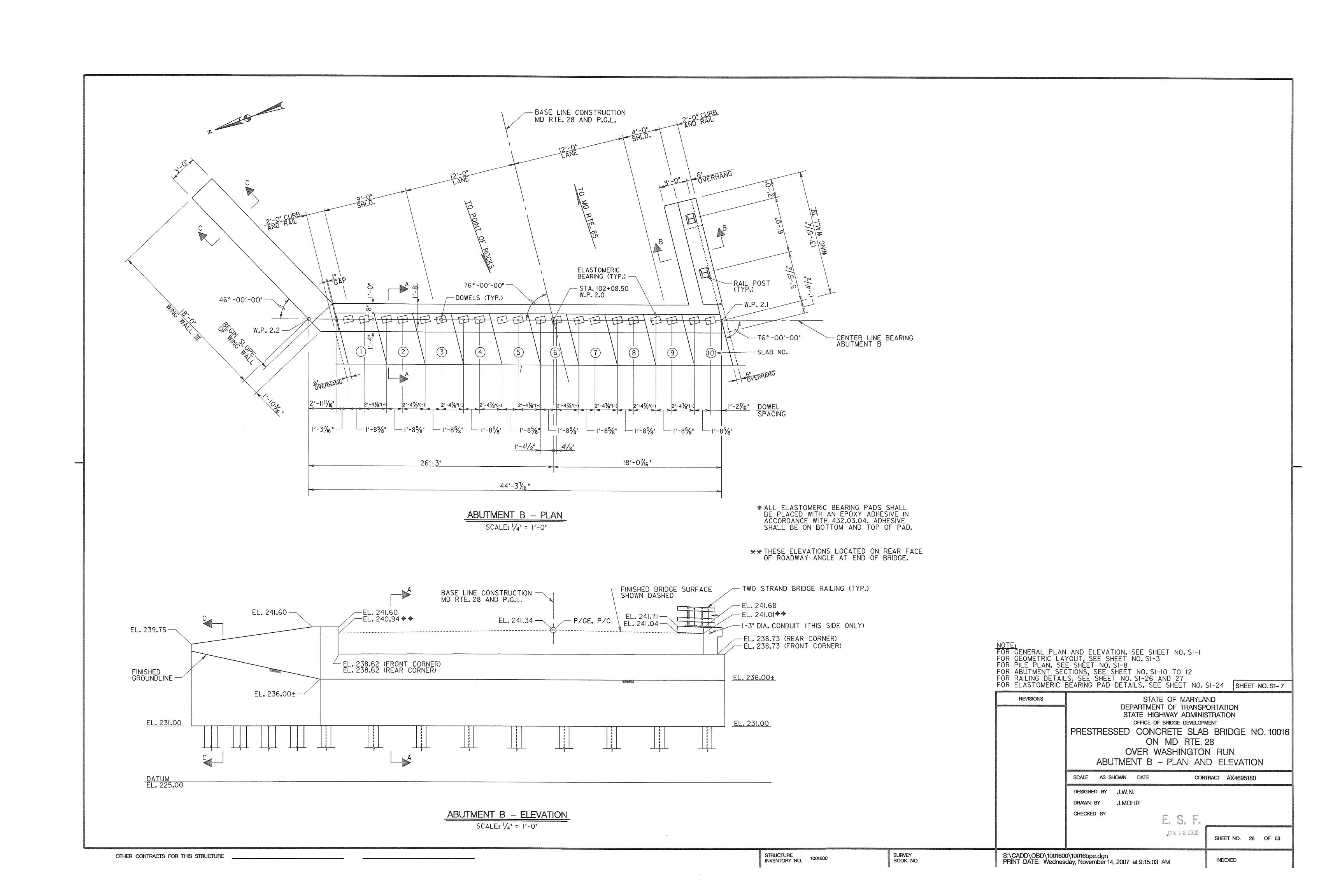
| NO 55505   |  |  |                                  | Same Control (min. )                   |              |           |         |   |  |                             |                  |           |                                  |                          |          |                                       |                            | IV. ROADWAY AND   | STRUCTURE  | DATA  |                      |  |
|--|--|--|----------------------------------|--|--------------|-----------|---------|---|--|-----------------------------|------------------|-----------|----------------------------------|--------------------------|----------|---------------------------------------|----------------------------|---|--|---|----------------------|--|
| SOURCE: <u>NO REPORT</u><br>PREPARED BY:   SH  |  | <u>-D - IN</u><br>CONSUL                 |                                  |  | <u>CEMEN</u> |           |         |   |  |                             |                  |           |                                  | DATE:                    | š        | · · · · · · · · · · · · · · · · · · · |                            | ITEM  |  | EXISTIN<br>STRUCTU  |                      | PROPOSED<br>STRUCTURE  |
| FILE LOCATION:   |  |  |                                  |  |              | 71        | RAT     | $ING^{2}_{-}$   |  |                             |                  |           |                                  |                          |          |                                       |                            | NAME OF WA  |  |   | WASHINGTO            |  |
| METHOD(S) OF ANALYSI   | S: <u>HEC-</u> F                           | RAS                                      |                                  |  | -            |           |         |   |  |                             |                  |           |                                  |                          |          |                                       |                            | OVERTOPPING   |  | 1932  |                      | 2008   |
| HYDRAULIC DATA   | APPROA                                     | ACH SE                                   | ECTION                           | 2148 L                                 | OCATE        | D 45      | 50 F    | T UP  | STRE   | AM                          |                  |           |                                  |                          |          |                                       |                            | OVERTOPPING<br>(DESCRI  | LOCATION<br>IBE)   | AT WEST ABI   | **                   | AT WEST ABUTMEN  |
|  |  | 4  | 1 4                              | 1 4                                    |              | CHANN     | C 1     | 5   | LE   | EFT OVE<br>LOOKII<br>DOWNST | RBANK            | 5         | R                                | IGHT OV<br>LOOK<br>DOWNS | VERBANK  | ( 5                                   | DISCHARGE<br>OVER ROAD     | INCIPIE<br>OVERTOPPING FL(<br>((OVERTOPPING Q <   | NT<br>DW CONDITION<br>LOO YR ELOOD)  | 1260 CF   | FS                   | 1260 CFS   |
| FLOW CONDITIONS  | CHANNEL<br>CROSS-SECTION                   | STRUCTURE<br>WATERWAY<br>AREA            | ENERGY<br>SLOPE                  | WATER<br>SURFACE<br>ELEVATION          |              | CHANN     | E.L.    |   |  | DOWNST                      | REAM             |           |                                  | DOWNS                    | STREAM   |                                       | OVER ROAD                  | FREEBO  |  | 0   |                      | 0  |
|  |  |  |                                  |  | Q            | W         | ٧       | D   | Q  | W                           | ٧                | D         | Q                                | W                        | ٧        | D                                     |                            | TOTAL STRUCTURE W   | ATERWAY AREA   | 3 150.0   |                      | 149.0  |
| <sup>Q</sup> DESIGN  | APPROACH<br>(DESCRIBE LO-<br>CATION BELOW) | N/A                                      | 0.0019                           | 241.3                                  | 430          | 20.4      | 3.9     | 5.4   | 584  | 209                         | 1.6              | 1.8       | 146                              | 95                       | 1.2      | 1.2                                   | N/A                        | STRUCTURE DE  | ESCRIPTION 14  | 37 FT. TOTAL 1<br>2 SPAN BRIDGE, 6.9 F                                | ENGTH<br>T MAX CLEAR | SINGLE 37 FT SPAN<br>6.0 FT MAX CLEARANCE  |
| DESCRIBE 50-YEAR DESIGN  | CATION BELOW)                              |  |                                  |  |              |           |         | 495.49  |  |                             |                  |           |                                  |                          |          |                                       |                            | INLET TREA  | ATMENT 15  | N/A   |                      | N/A  |
| STORM FOR THE ROADWAY  | UPSTREAM<br>AT STRUCTURE                   | 149.0                                    | 0.0033                           | 240.3                                  | 736          | 17.3      | 6.0     | 7.1   | 190  | 298                         | 3.7              | 4.2       | 234                              | 236                      | 3.6      | 4.0                                   | 0                          | OUTLET TRE  | EATMENT 15   | N/A   | ·                    | N/A  |
|  | DOWNSTREAM<br>AT STRUCTURE                 | 149.0                                    | 0.0108                           | 238.6                                  | 787          | 17.9      | 8.7     | 1 1   | 183  | 280                         | 3.6              | 2.5       | 190                              | 228                      | 3.5      | 2.5                                   | N/A                        | MANNINGS "N   |  |   |                      |  |
|  | APPROACH 8                                 |  |                                  | -                                      |              |           | <u></u> | ***************************************   |  |                             |                  |           |                                  |                          |          |                                       |                            | V. SURVEY BOOK  |  |   |                      |  |
| Q 100 DESCRIBE   | APPROACH<br>(DESCRIBE LO-<br>CATION BELOW) | N/A                                      | 0.0005                           | 243.05                                 | 347          | 20.4      | 2.4     | 7.1   | 894  | 245                         | 1.2              | 3.2       | 319                              | 126                      | 6 1.0    | 2.5                                   | N/A                        | REFERENCE DAT   | TUM FOR E  | LEVATIONS _   | NAVD                 | 88   |
| DESCRIBE.  | UPSTREAM<br>AT STRUCTURE                   | 149.0                                    | 0.00004                          | 243.03                                 | 134          | 17.3      | 0.8     | 9.8   | 695  | 385                         | 0.4              | 4.4       | 731                              | 236                      | 6 0.5    | 6.2                                   | 0                          | VI.FLOOD PLAIN N  | /ANAGEMEN  | T DATA  |                      |  |
|  | DOWNSTREAM<br>AT STRUCTURE                 | 149.0                                    | 0.0015                           | 239.12                                 | 342          | 17.9      | 3.4     | 7<br>5.6  | 836  | 298                         | 1.2              | 2.3       | 382                              | 228                      | 8 1.0    | 1.7                                   | N/A                        | DATE OF FLOOD INSU  | RANCE STUDY  | N/A commun  | NITY PANEL I         | NO. <u>240027 025</u> 0  |
|  | AT STRUCTURE                               | 143.0                                    | 0.0013                           | 233.12                                 | 714          |           | J. 1    |   |  |                             |                  |           | 302                              |                          | 7.0      |                                       |                            | PROJECT LOCATION (  |  |   |                      |  |
| <sup>Q</sup> INCIPIENT OVERTOPPING, <sup>Q</sup> 500<br>OR OTHER DISCHARGE                       | APPROACH (DESCRIBE LO-CATION BELOW)        | N/A                                      | 0.0013                           | 241.7                                  | 405          | 20.4      | 3.4     | 5.8   | 672  | 217                         | 1.5              | 2.1       | 182                              | 106                      | 5 1.2    | 1.5                                   | N/A                        | BEYOND FEMA   |  |   |                      |  |
| DESCRIBE 1260 CFS, INCIPIENT OVERTOPPING   |  |  |                                  |  |              |           | -       |   |  |                             |                  |           |                                  |                          |          |                                       |                            |   |  |   |                      |  |
|  | UPSTREAM<br>AT STRUCTURE                   | 149.0                                    | 0.0029                           | 240.79                                 | 776          | 17.3      | 6.0     | 7.6   | 215  | 312                         | 3.8              | 4.7       | 268                              | 236                      | 6 3.6    | 4.5                                   | 0                          | FEMA HAZARD  REGULATORY FLOODW  | -  |   | P F2 LABE12HI        | EU   |
|  | DOWNSTREAM<br>AT STRUCTURE                 | 149.0                                    | 0.0113                           | 238.74                                 | 842          | 17.9      | 9.1     | 5.2   | 205  | 285                         | 3.8              | 2.6       | 213                              | 228                      | 8 3.8    | 2.6                                   | N/A                        | MAXIMUM CHANGE IN BRIDGE DUE TO HIGHW   | WATER SURFACE  | ELEVATION UPSTR   |                      |  |
| BRIDGE SCOUR DATA SCOUR EVALUATION S' PREPARED BY: SH FILE LOCATION:                             | TUDY TITLE                                 | ::                                       | METANT: _                        |  |              |           |         |   | (  | 8L<br>NOT                   | ES:              | 'S INDIC. | ATE THA                          |                          |          |                                       |                            | DESCRIBE TYPE OF S WITH NFIP STANDARDS DATE COMMUNITY ACK  IS THE PROJECT CON PART 650 A, LOCATIVE FLOOD PLAINS (23 CF)  IS THE PROJECT CON | S NAME NOWLEDGEMENT  SISTENT WITH THE NAME OF AND HYDRAU  FR 650 A). Y  SISTENT WITH THE | FORM ISSUED:  HE CODE OF FEDER LIC DESIGN OF ENC YN YES  HE ANNOTATED | N/A RAL REGULAT      | •  |
| (DESCRIBE SPECIAL CONDITIONS SUCH AS OVERTOPPING, LOW TAILWATER, INFLUENCE OF CONFLUENCES, ETC.) | DISCHARGE  RETURN MAGNITUDE PERIOD         | LONG TERM DEGRADATION / AGGRADATION (FT) | (LOOKING DO                      | JR DEPTH<br>OWNSTREAM) (FT)<br>MAIN RT | 15250,53     |           |         | OF SCOUR<br>CLEAR WAT   | ***  | I. PARAI                    | METER            | S COMP    | UTED AS<br>WITHOUT               |                          |          | TERSHE                                | ED                         | CODE OF MARYLAND  |  |   |                      |  |
| DESIGN FLOOD   | (YEARS) (CFS)<br>50-YR. 1,260              | 1.3 FT.                                  | 3.0                              | 2.9 2.9                                | GRAVEL       |           | 1 1     | VE BED  |  |                             |                  |           | ND ITEM I                        | II3 RATI                 | ING REF  | ER TO                                 | FEDERAL                    | VII. COMMENTS:TI  |  |   |                      | . WAIVER OF<br>E STRUCTURALLY  |
| FOR SCOUR INCIPIENT OVERTOPPING  | PLUS 1,250                                 | DEGREDATION                              | 2.9                              |  | UIAVEE       |           |         | VI 020  |  |                             |                  |           | NDITIONS                         |                          |          |                                       | CHARGE (O),                | IN-KIND REPLACE   |  |   |                      |  |
| CHECK FLOOD<br>FOR SCOUR 100-YR.   | 100-YR. 1,560                              | 1.3 FT.<br>DEGREDATION                   | 4.1                              | 4.1                                    | GRAVEL       |           | LI      | VE BED  |  | DEPF<br>ASSI                | RESSED<br>JMPTIO | CULVE     | ERTS, IND                        | ICATE U                  | JNDER C  | OMMENT                                |                            |   |  |   |                      | NOBEMBER 21, 20  |
| OTHER  |  |  |                                  |  |              |           |         |   | THE PROPERTY OF THE PROPERTY O | 4. FOR                      |                  | :RTS, U   | SE THESE                         |                          |          |                                       |                            | SIGNED BY CHRI  |  |   |                      |  |
| TOTAL SCOUR: ESTIM   | ATED TOTAL SCOUR AT                        | SUBSTRUCTURE/                            | CHANNEL ELEM                     | MENTS (INCLUDES<br>JR, PLUS LOCAL      | LONG TERM DE | EGRADATIO | N/AGGR/ | DATION  | OT A THE PROPERTY OF THE PROPE | 0 W/                        | ATER-S           | SURFACE   | V AT CUL<br>E ELEVAT<br>FOR CUL  | ION AT                   | CULVER   |                                       | T AND OUTLET               | WETLANDS AND  | WATERWAYS  | DIVISION.   |                      |  |
| LOCATION OF CHANNEL OR SUBSTRUCTU  | IRE ELEMENT                                | ELE<br>DESIGN FL                         |                                  | TOM OF STREAM CHECK FLOOR              |              |           | COUNTER | FT) IO<br>MEASURES<br>Ø NEW   | THE PROPERTY OF THE PROPERTY O | W =                         | FLOW<br>CHANN    | OR DIS    | CHARGE (<br>TH OR FL<br>TY (FPS) |                          | AIN WIDT | H (FT)                                |                            | ,   |  |   |                      |  |
| CHANNEL THALWEG ABUTMENT: EAST   |  | 229.00                                   | )                                | 227.43                                 |              |           | CLASS : | C   |  |                             |                  |           | OW (FT)<br>ECORD OL              | ITI FT N                 | VFI OCIT | Y HFRF                                |                            |   |  |   |                      | SHEET NO. S  |
| ABUTMENT: WEST PIER NO.  |  | 228.64                                   | 1                                | 227.00                                 |              |           | CLASS : | <u>.</u>  |  |                             |                  |           | RECORD -                         |                          |          |                                       |                            | REVISIONS   |  | STATE<br>DEPARTMENT   | OF MARYLA OF TRANSI  |  |
| PIER NO.   |  |  |                                  |  |              |           |         |   |  |                             |                  |           | N SHOULE<br>MANUAL               | D BE SE                  | ELECTED  | AS PE                                 | ER GUIDANCE IN             |   |  | STATE HIGHV   |                      | ISTRATION  |
| PIER NO.<br>PIER NO.   |  |  |                                  |  |              |           |         |   | -  | IN A                        | BSCOU            | R OUTP    | UT) - NO                         | T ABUTI                  | MENT S   | COUR                                  | ROXIMATE LINE 121          |   | PRESTRES   | SSED CONCR  |                      | B BRIDGE NO.1  |
| PIER NO.   |  |  |                                  |  |              |           |         |   |  |                             |                  |           | NOTE THIS                        |                          |          |                                       | R, ENTER BEDROCK<br>MMENTS |   |  | OVER WA   |                      |  |
| PIER NO.   |  |  |                                  |  |              |           |         |   | The state of the s | II. RECO                    |                  | CIPIENT   | OVERTOR                          | PPING DI                 | ISCHARG  | E (Q) AI                              | ND RECURRENCE              |   | HY   | DRAULIC ANI   | ) HYDRO              | DLOGIC DATA  |
| PIER NO.   |  |  |                                  |  |              |           |         |   |  |                             |                  |           | CE BETWI<br>DESIGN D             |                          |          | RFACE                                 | ELEVATION AND              |   | SCALE AS S   | HOWN DATE   | COI                  | NTRACT AX4695180   |
| PIER NO.   |  |  |                                  |  |              |           |         | -   |  |                             |                  |           | LOW ARE<br>500 YEAR              |                          |          | CTURE                                 | (DOWNSTREAM                |   | DESIGNED BY  | J.W.N.  |                      |  |
| PIER NO.   |  |  | ,                                |  |              |           |         |   |  | 14. FOR<br>ENT              | ER TY            | PE. SPA   | AN LENGT                         | H AND I                  | MAXIMU   | M VERT                                | ICAL CLEARANCE             |   | DRAWN BY   | J.MOHR  | usse                 |  |
| PIER NO. PIER NO.  |  |  |                                  | ·                                      |              |           |         |   |  | DES                         | CRIBE            | ANY SP    | BER OF C                         | ATURES                   | UNDER    | COMME                                 |                            |   | CHECKED BY   |   |                      | , DECOMPOSITION OF THE PROPERTY OF THE PROPERT |
| PIER NO.   |  |  |                                  |  |              |           | ,       | Wind the second |  | PR0                         | TECTIO           | NC        |                                  |                          |          | 「/OUTLI                               | ET AND EROSION             |   |  | J.P.  | <b>N 0 8</b> 2002    | SHEET NO. 21 OF  |
|  |  |  | nodeskastitispoorasak (bosontoe) |  |              |           |         | STRUCTU   | RE   | 16. COM                     |                  | E "N" VA  | ALUE OF :                        | SI                       | URVEY    |                                       |                            | S:\CADD\OBD\100160<br>PRINT DATE: Wednes  | 00\10016hh.dgn   |   |                      | INIDOVED   |
|  |  |  |                                  |  |              |           |         | NVENTOR   | RY NO.   | เบบเปป                      | ~                |           |                                  | B                        | OOK N    | U.                                    |                            | PRINT DATE: Wednes  | sday, November 14  | 4, 2007 at 8:23:37 A  | M                    | INDEXED  |

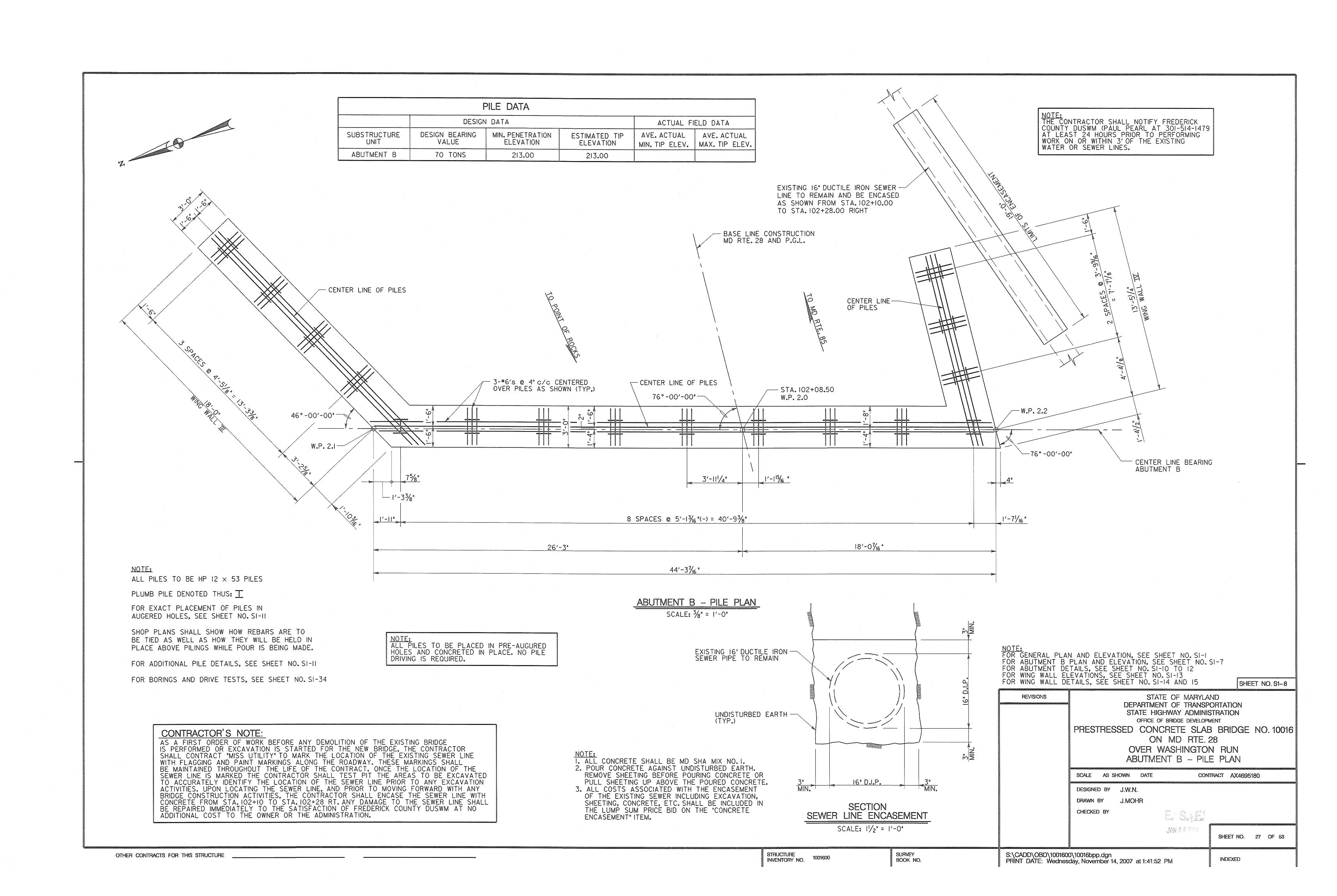


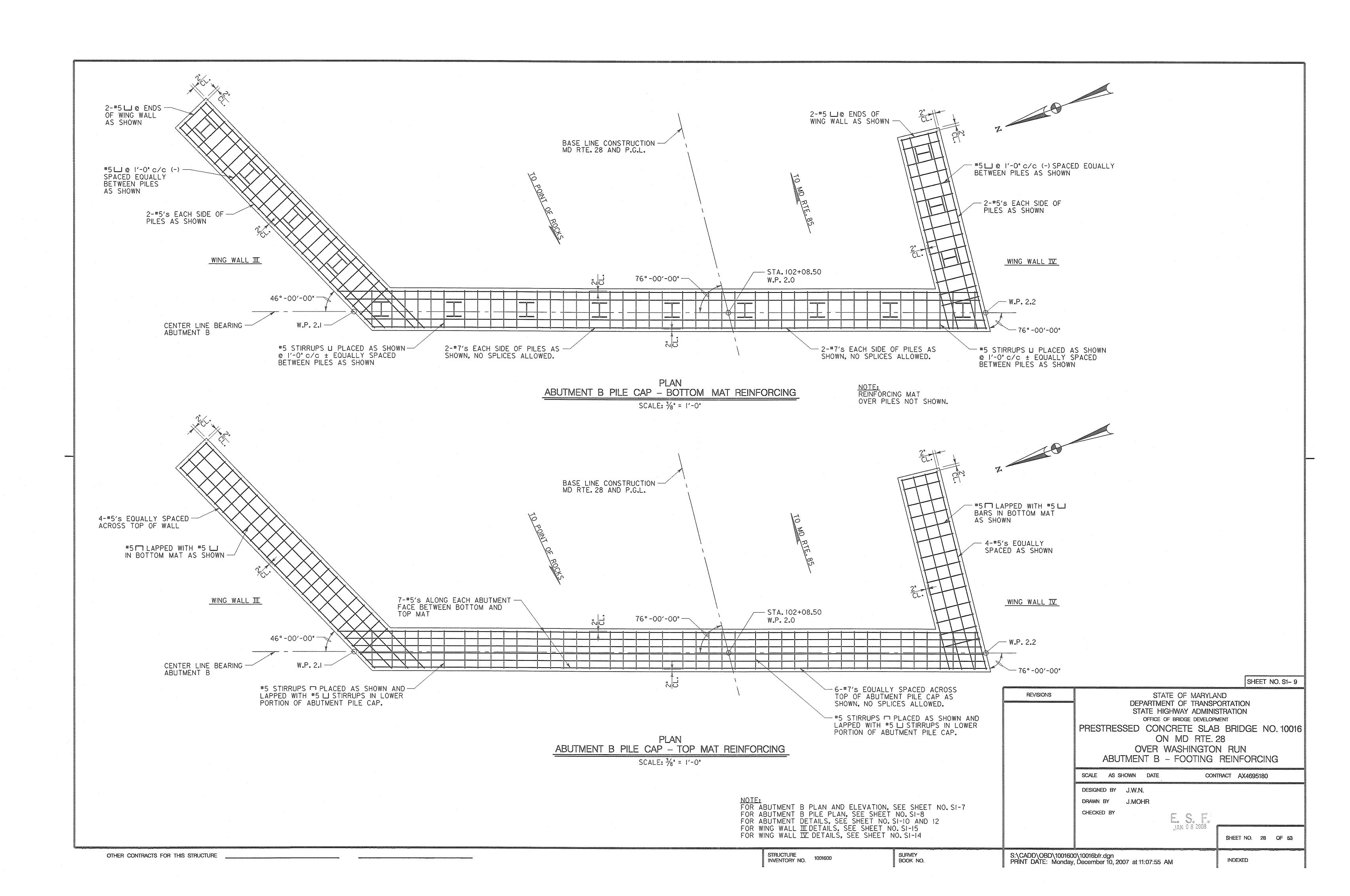


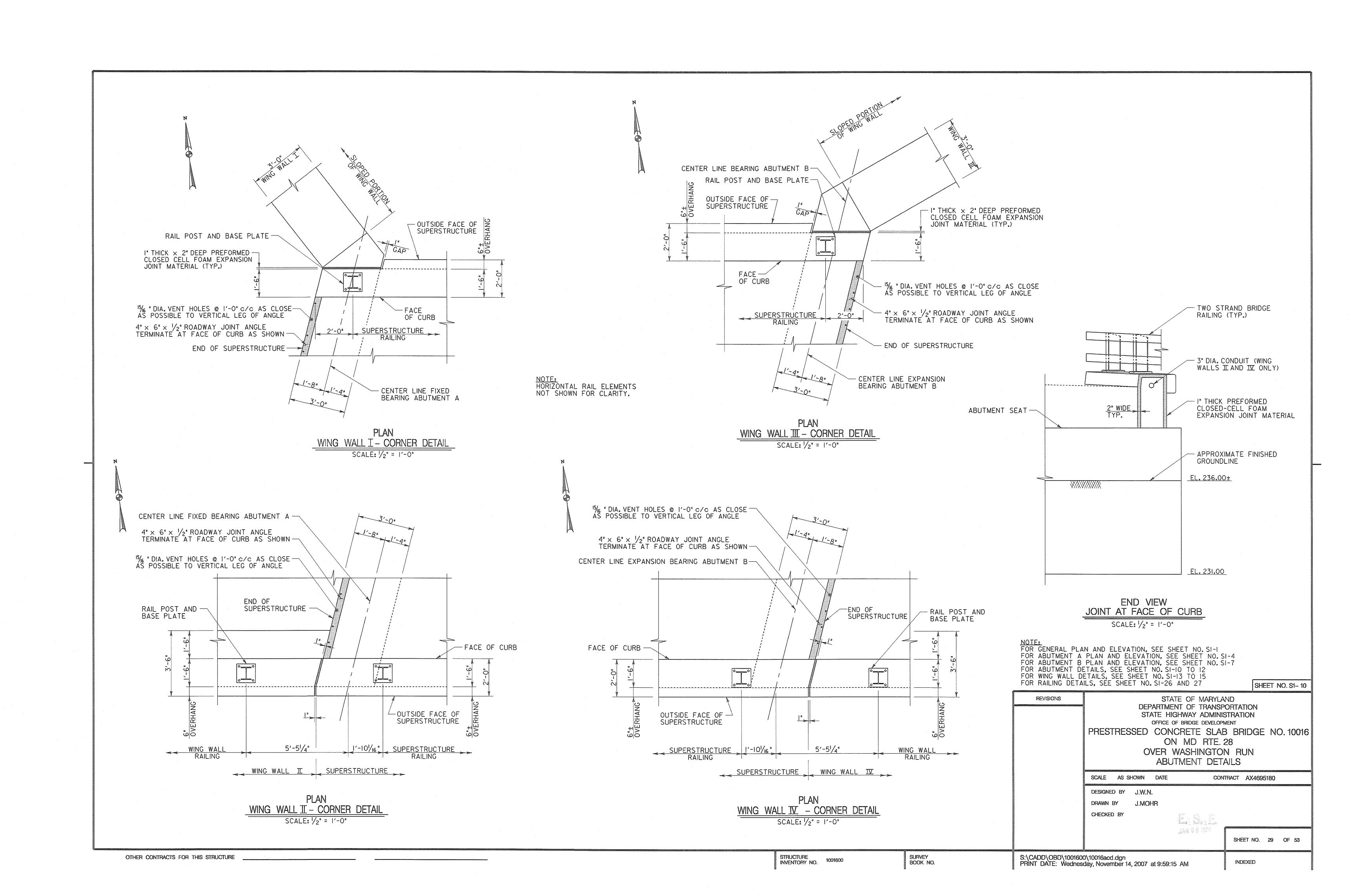


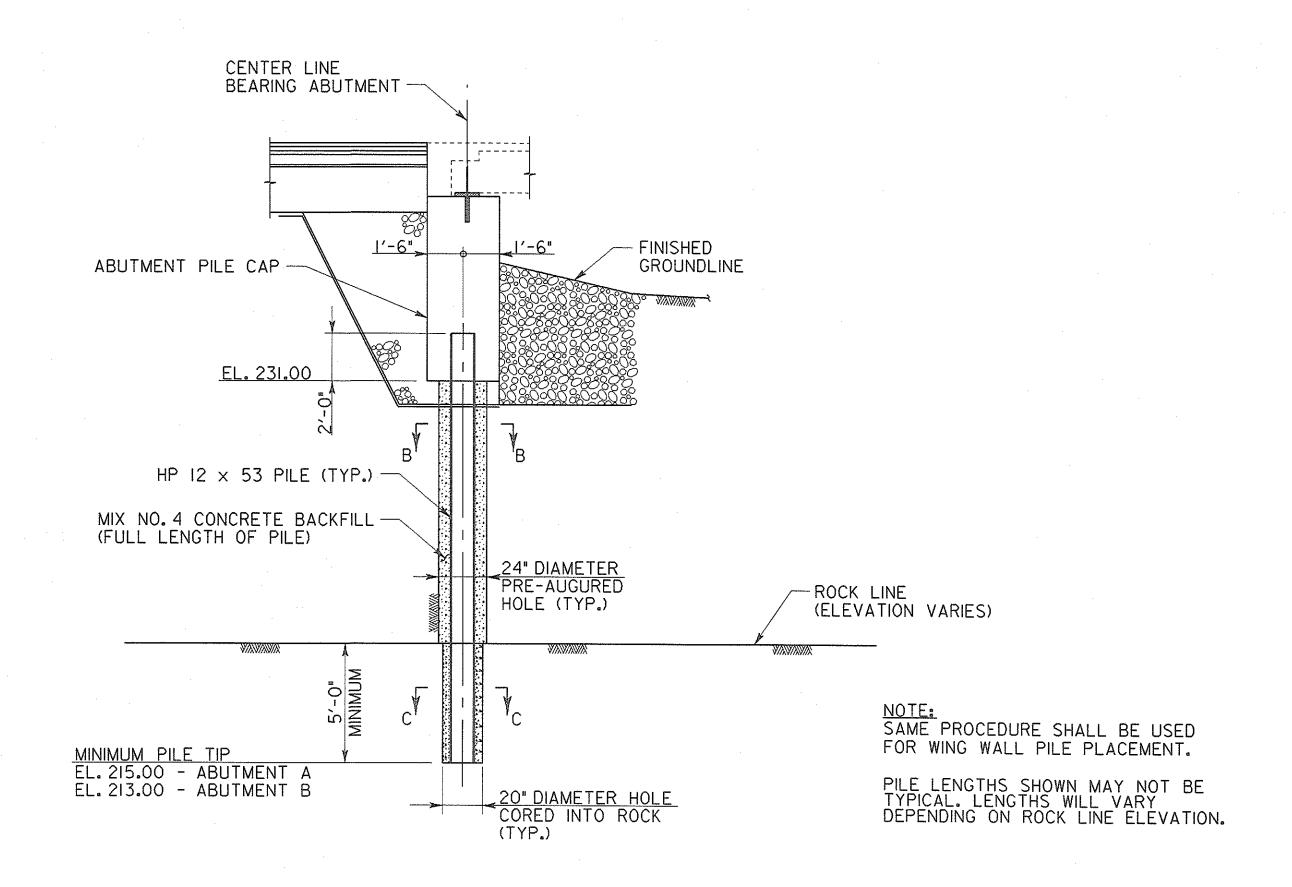




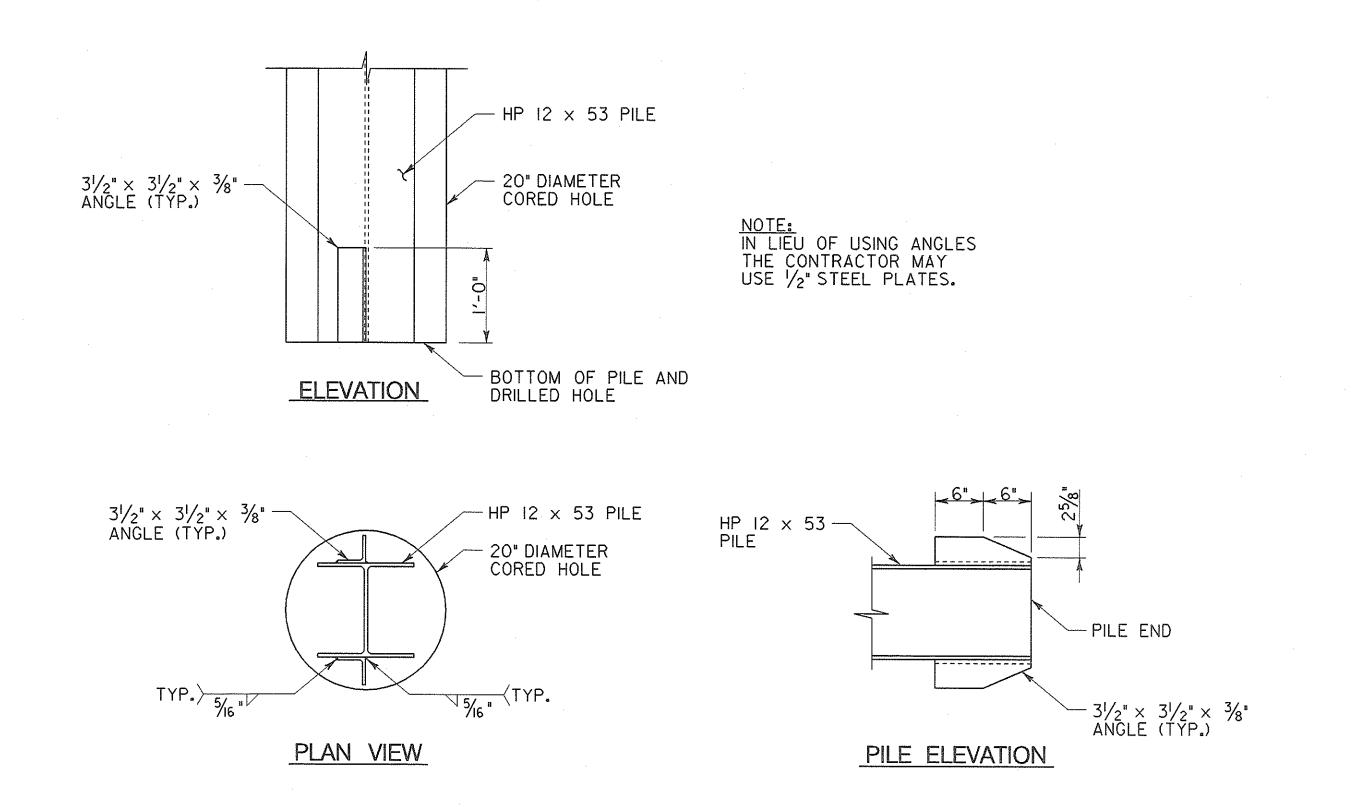




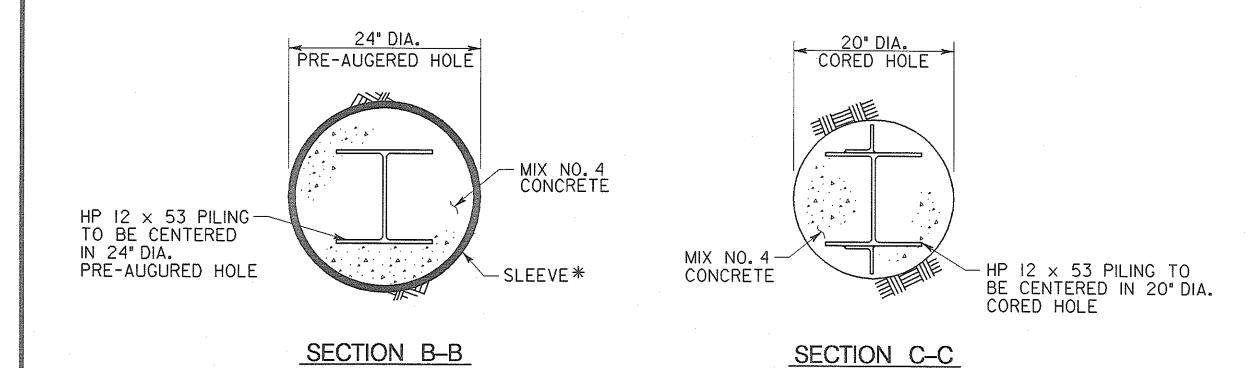




**SECTION** ABUTMENT - PILE PLACEMENT DETAIL SCALE:  $\frac{1}{4}$ " = 1'-0"



PILE CENTERING DEVICE SCALE: |"=|'-0"



\*THE 24" DIAMETER PRE-AUGERED HOLE MAY REQUIRE THE USE OF A SLEEVE DURING THE AUGERING OPERATION TO PREVENT THE SOIL FROM CAVING-IN. IF A SLEEVE IS USED, IT SHALL BE REMOVED DURING THE CONCRETE PLACEMENT OPERATION.

SCALE: |"=|'-0"

DURING THE PLACEMENT OF CONCRETE THE FOLLOWING PROCEDURE SHALL BE USED:

I. CLEAN ALL LOOSE DEBRIS FROM BOTTOM OF HOLE. 2. PLACE 2'-0" CONCRETE IN BOTTOM OF HOLE. 3. SET PILE IN PLACE. CENTERED IN HOLE. 4. PLACE REMAINDER OF CONCRETE TO TOP OF HOLE.

NOTE:
CONTRACTOR SHALL USE STANDOFFS, ETC. TO MAINTAIN PILE LOCATION IN CENTER OF HOLE DURING CONCRETE PLACEMENT. THESE DEVICES SHALL BE INDICATED ON SHOP DRAWAINGS AND SHALL BE SUBMITTED FOR REVIEW AND APPROVAL.

SCALE: |"=|'-0"

### PILE PLACEMENT PROCEDURE:

I) THE CONTRACTOR SHALL PRE-AUGER A 24" DIAMETER HOLE AT EACH PILE LOCATION UNTIL ROCK IS ENCOUNTERED. A SLEEVE SHALL BE INSTALLED, IF REQUIRED, AND LEFT IN PLACE UNTIL THE PILE IS IN PLACE AND THE CONCRETE IS POURED. THE SLEEVE SHALL BE EXTRACTED AS THE CONCRETE IS POURED SO THAT THE ENTIRE VOID IS FILLED AND CONCRETE IS IN CONTACT WITH SIDE OF THE HOLE, HOLES SHALL CONFORM TO THE APPLICABLE PORTIONS OF 412.03.04. THE SERVICES OF A GEOTECHNICAL ENGINEER WILL NOT BE REQUIRED.

2) AFTER AUGERING THE HOLE TO ROCK, THE CONTRACTOR SHALL CORE A 20" DIAMETER SOCKET A MINIMUM OF 5 FEET INTO THE ROCK LAYER OR TO A MINIMUM TIP ELEVATION OF 215.00 AT ABUTMENT A AND ELEVATION 213.00 AT ABUTMENT B. WHICHEVER IS LOWER. THE MINIMUM PILE TIP ELEVATION IS ACCEPTABLE IF AT LEAST 5 FEET OF THE BOTTOM OF PILE HAS BEEN CORED INTO ROCK.

3) UPON COMPLETING THE 5 FEET OF ROCK CORING, THE CONTRACTOR SHALL PLACE 2'-O" OF CONCRETE IN THE BOTTOM OF THE HOLE, PLACE A PILE TO THE BOTTOM OF THE CORED HOLE PRIOR TO THE CONCRETE SETTING OR BECOMING NON-PLASTIC, AND BACKFILL THE REMAINDER OF THE HOLE WITH MIX NO. 4 CONCRETE TO THE BOTTOM OF FOOTING AS SHOWN IN THE PILE PLACEMENT DETAIL. THE TOP OF THE PILE SHALL BE SUPPORTED AS NEEDED TO MAINTAIN THE FINAL ALIGNMENT OF THE PILES IN THE PILE CAP. IF A SLEEVE WAS INSTALLED FOR AUGERING THE 24" HOLE, IT SHALL BE EXTRACTED WHILE THE CONCRETE IS BEING POURED (NOTE: THE SLEEVE MUST BE EXTRACTED, UNDER NO CIRCUMSTANCES IS THE SLEEVE TO REMAIN IN PLACE).

THE COST OF PRE-AUGERING THE HOLE, INSTALLING SLEEVES AS NECESSARY, CORING THE ROCK, BACKFILLING THE HOLE WITH CONCRETE, REMOVING THE SLEEVES, AND ALL OTHER PROCEDURES REQUIRED TO INSTALL THE PILES WILL NOT BE MEASURED FOR PAYMENT BUT THE COST WILL BE INCIDENTAL TO THE PRICE PER LINEAR FOOT FOR THE PERTINENT PILING ITEM. THE LINEAR FOOT PRICE WILL INCLUDE ALL MATERIAL, PILE CENTERING DEVICES, LABOR, SETUPS, AUGERING, CORING, REMOVAL AND DISPOSAL OF EXCAVATED MATERIAL, PILING, SLEEVING, CONCRETE, ETC. TO COMPLETE THE PILE PLACEMENT AS SHOWN. THERE WILL BE NO SEPERATE PAY ITEM FOR THE ROCK SOCKET.

STRUCTURE

FOR GENERAL PLAN AND ELEVATION, SEE SHEET NO. SI-I
FOR ABUTMENT PLAN AND ELEVATIONS, SEE SHEET NO. SI-4 AND 7
FOR ABUTMENT PILE PLANS, SEE SHEET NO. SI-5 AND 8
FOR WING WALL ELEVATIONS, SEE SHEET NO. SI-I3
FOR WING WALL DETAILS, SEE SHEET NO. SI-I4 AND I5
SHE

SHEET NO. S1-11

**REVISIONS** STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT PRESTRESSED CONCRETE SLAB BRIDGE NO. 10016 ON MD RTE. 28 OVER WASHINGTON RUN ABUTMENT PILE PLACEMENT DETAILS SCALE AS SHOWN DATE CONTRACT AX4695180 DESIGNED BY J.W.N. J.MOHR DRAWN BY CHECKED BY JAN 08 2008

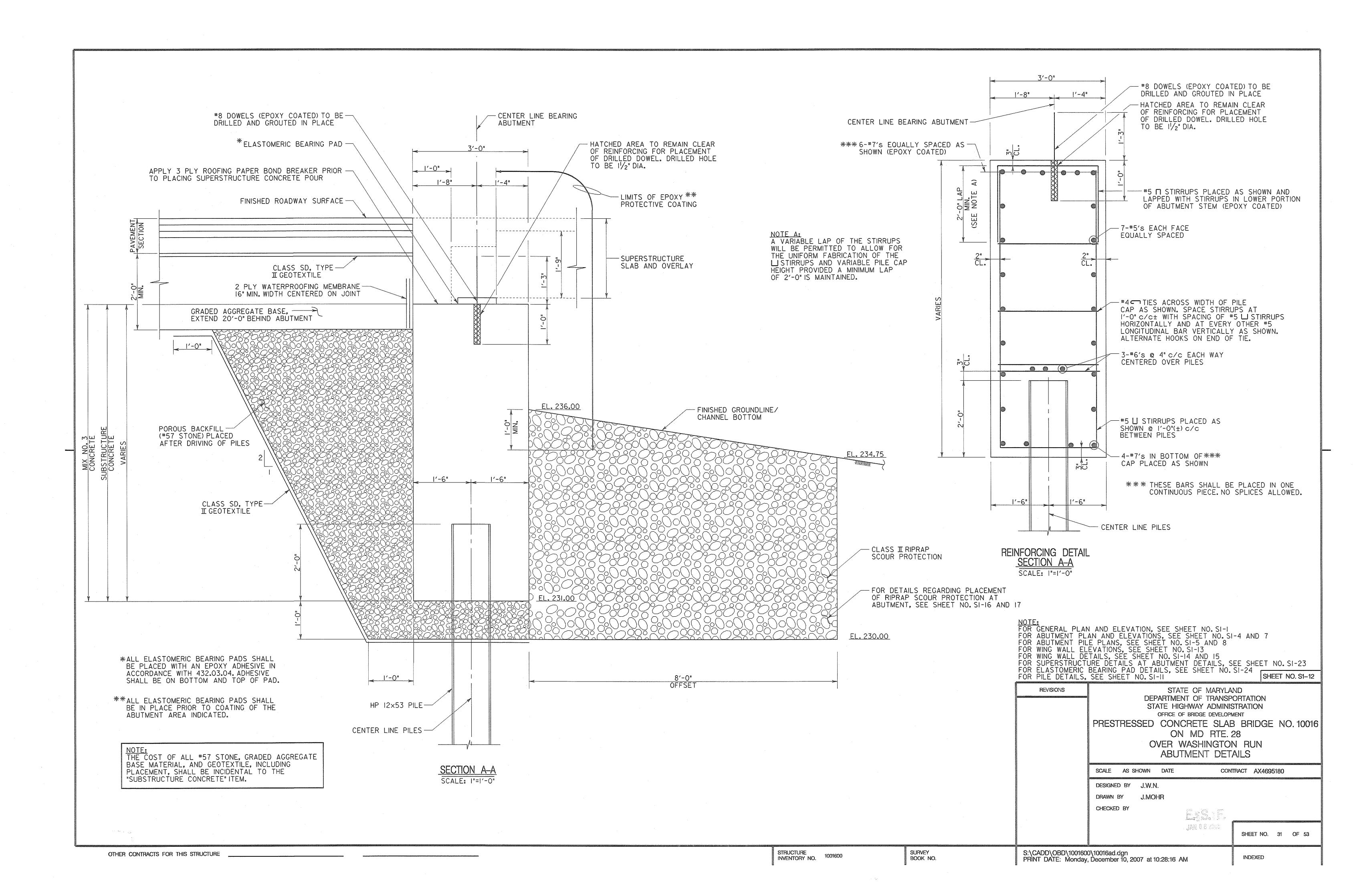
OTHER CONTRACTS FOR THIS STRUCTURE INVENTORY NO. 1001600

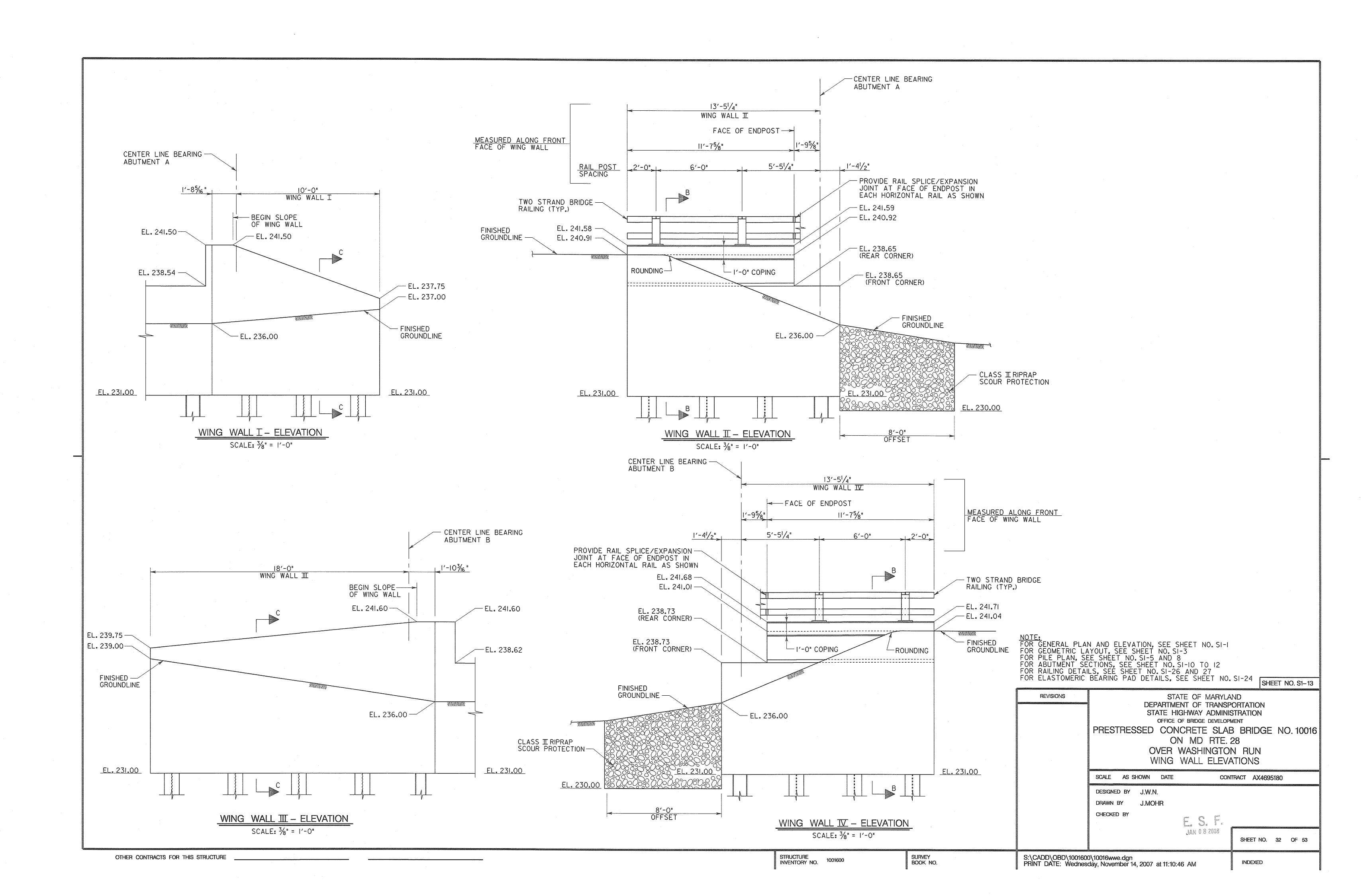
BOOK NO.

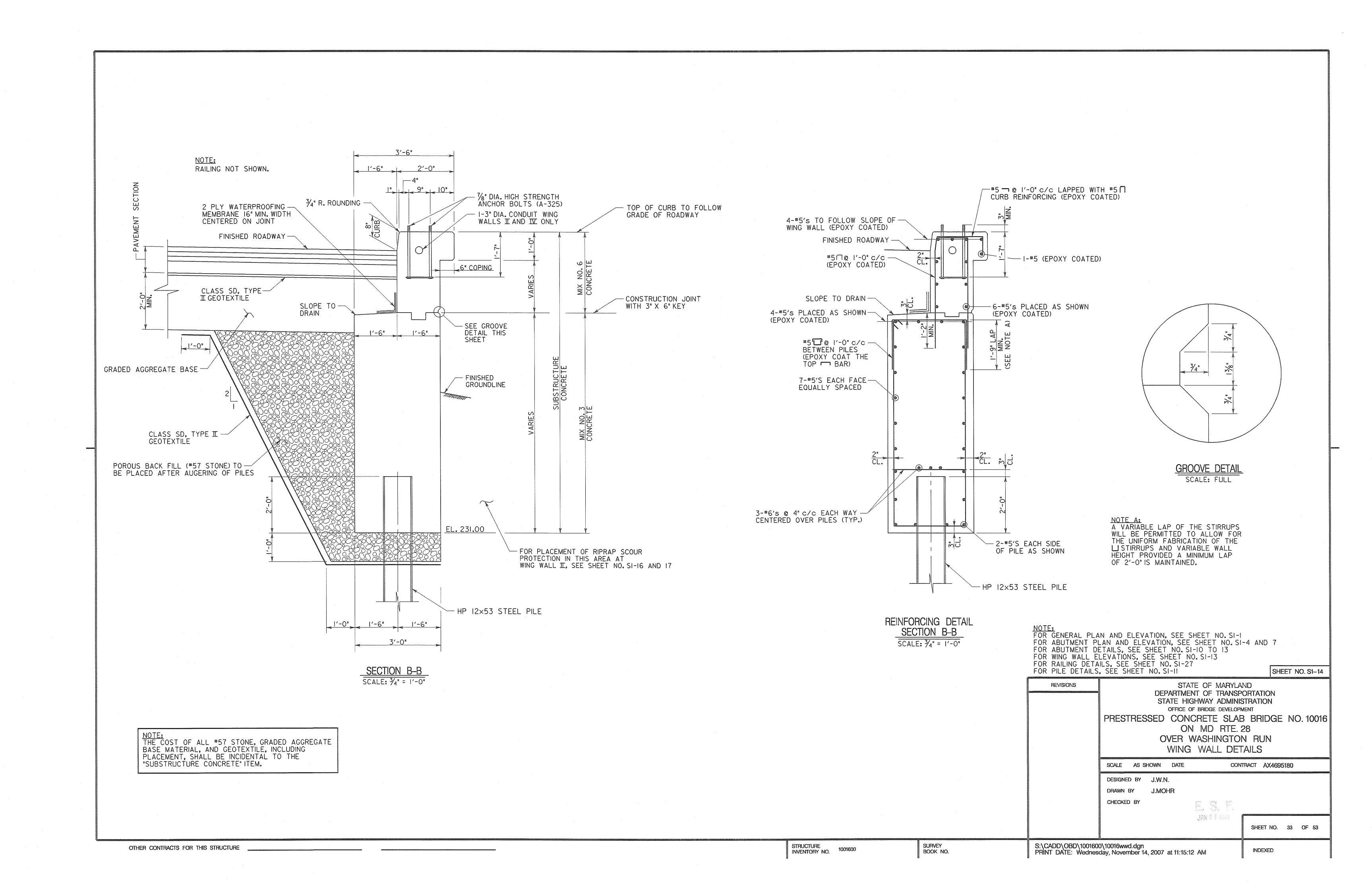
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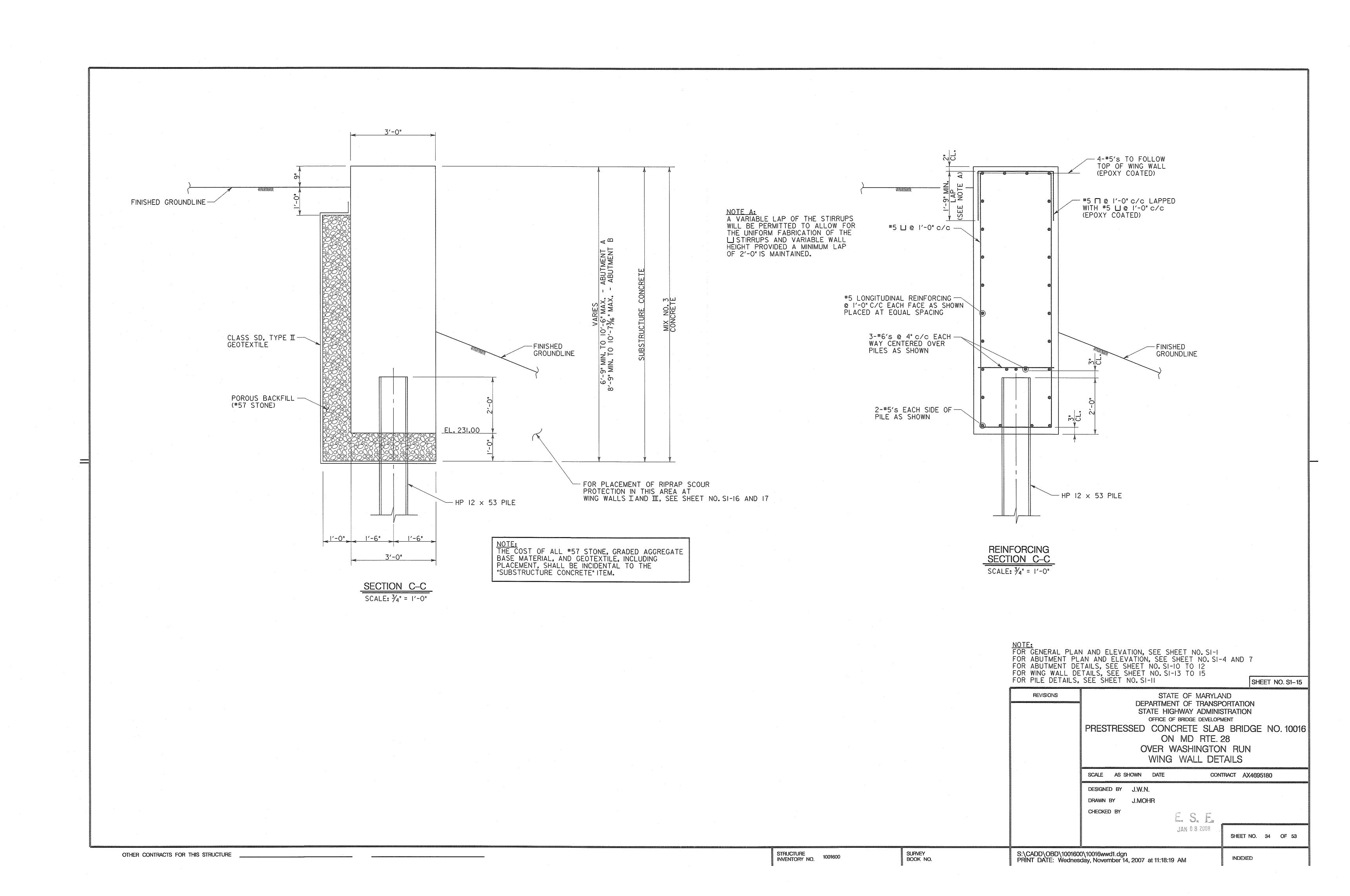
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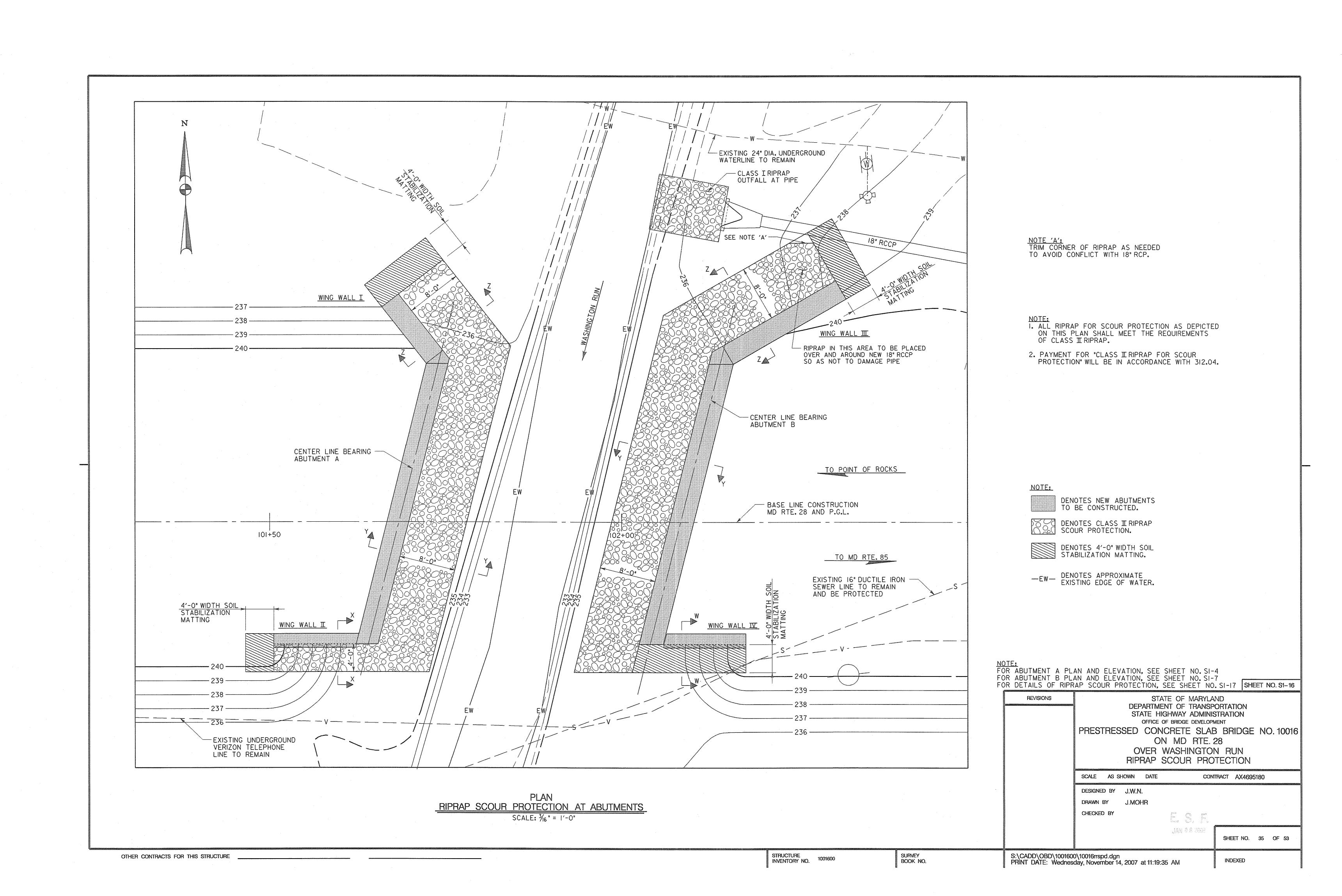
SHEET NO. 30 OF 53

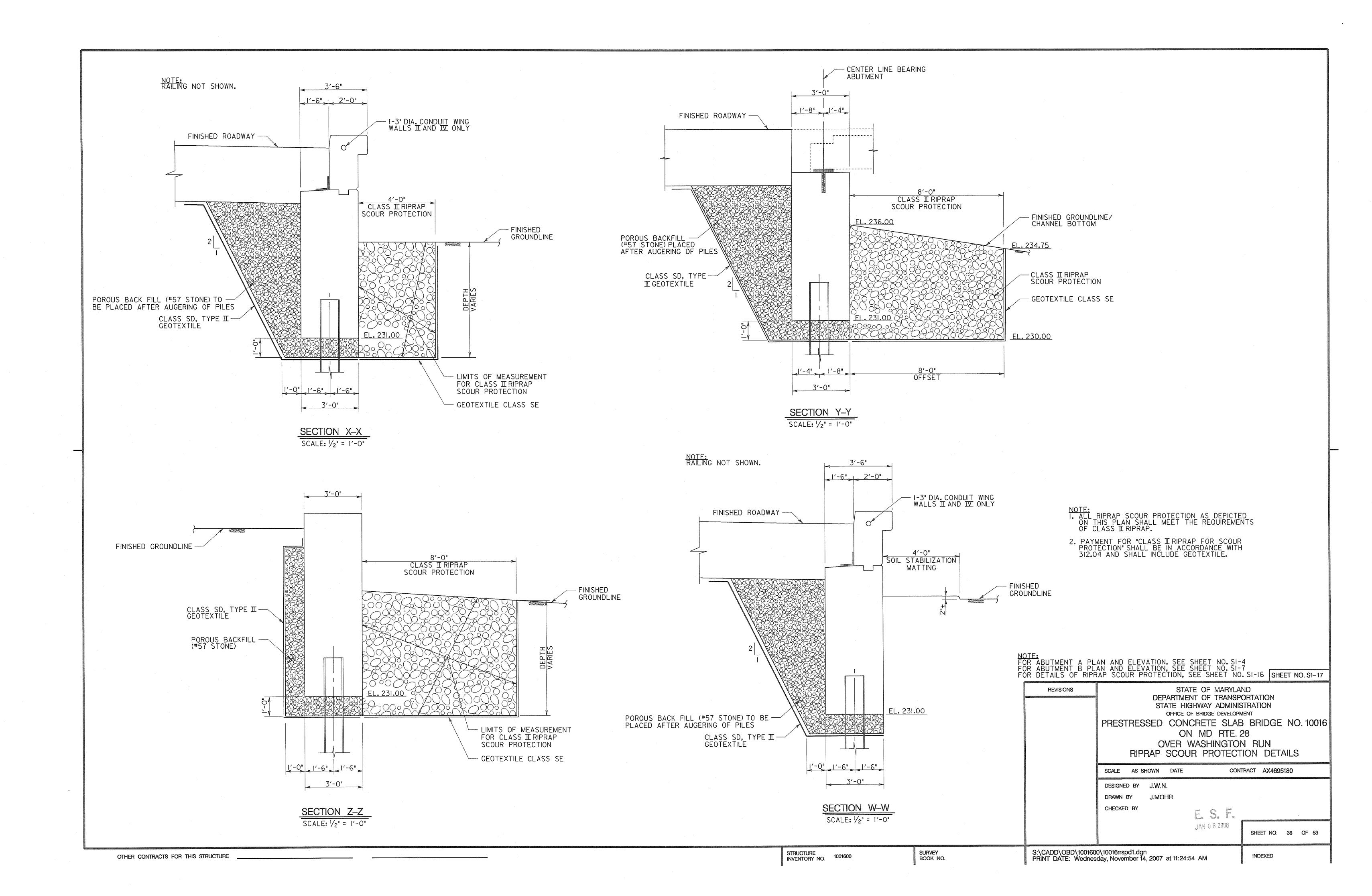


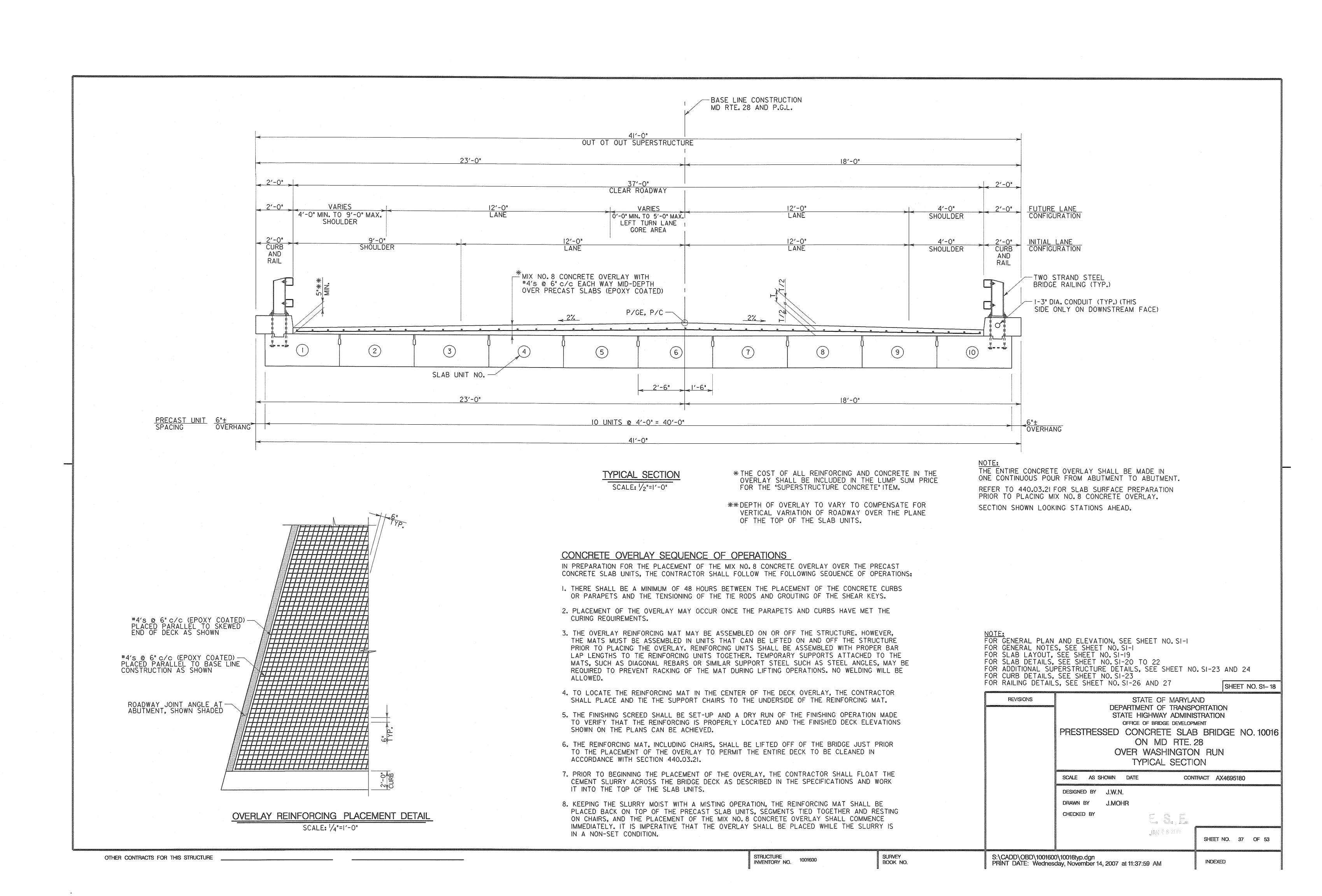


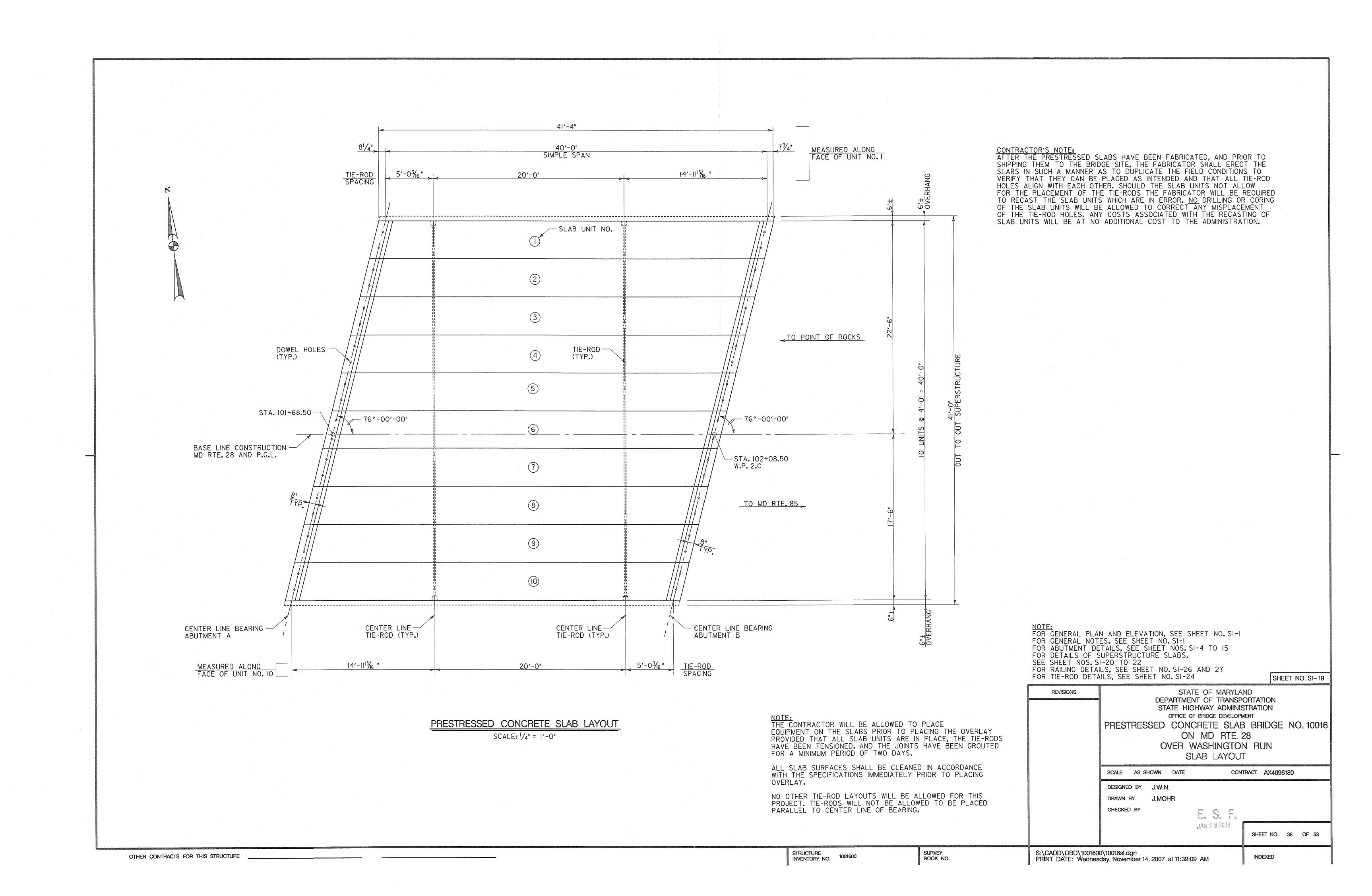


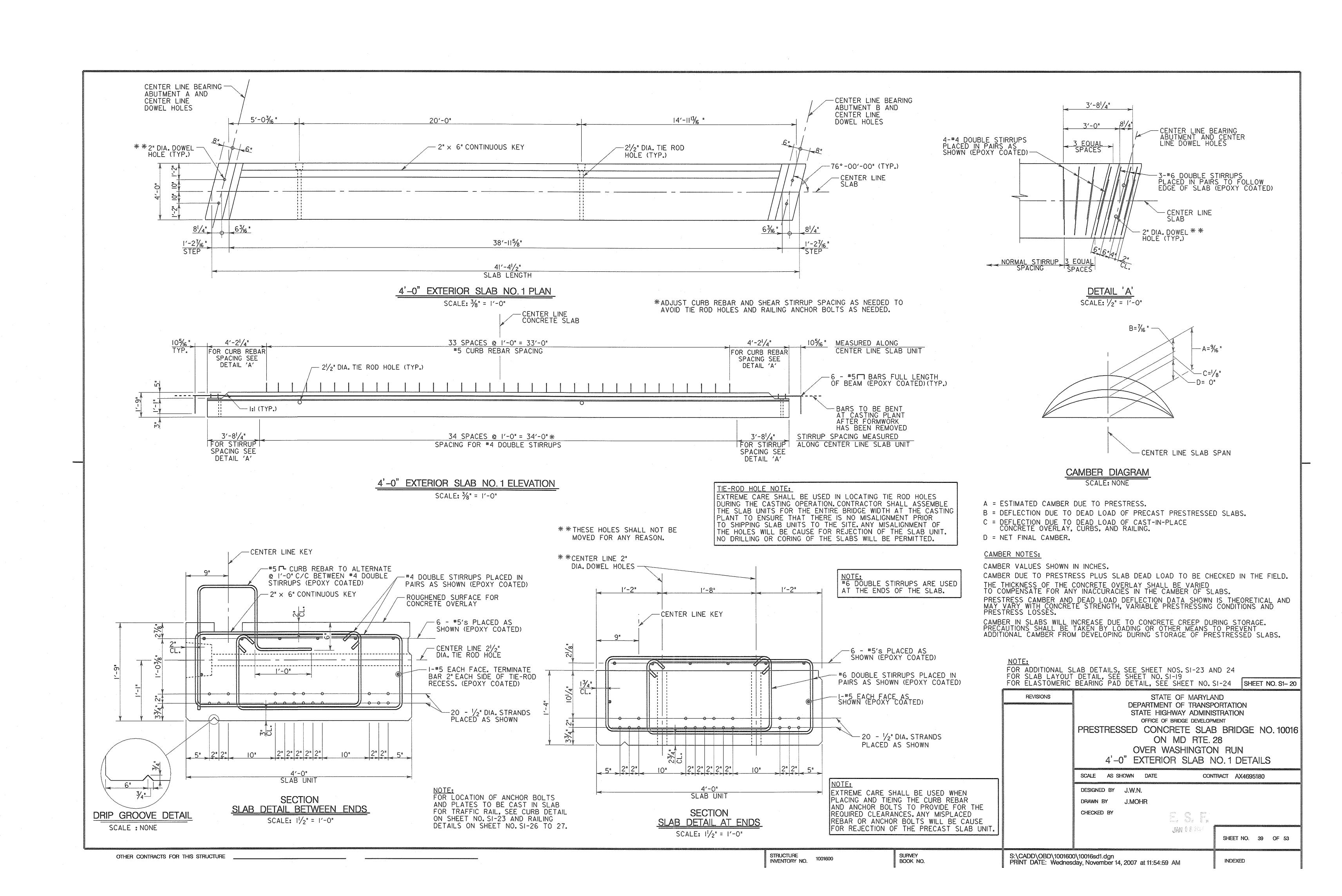


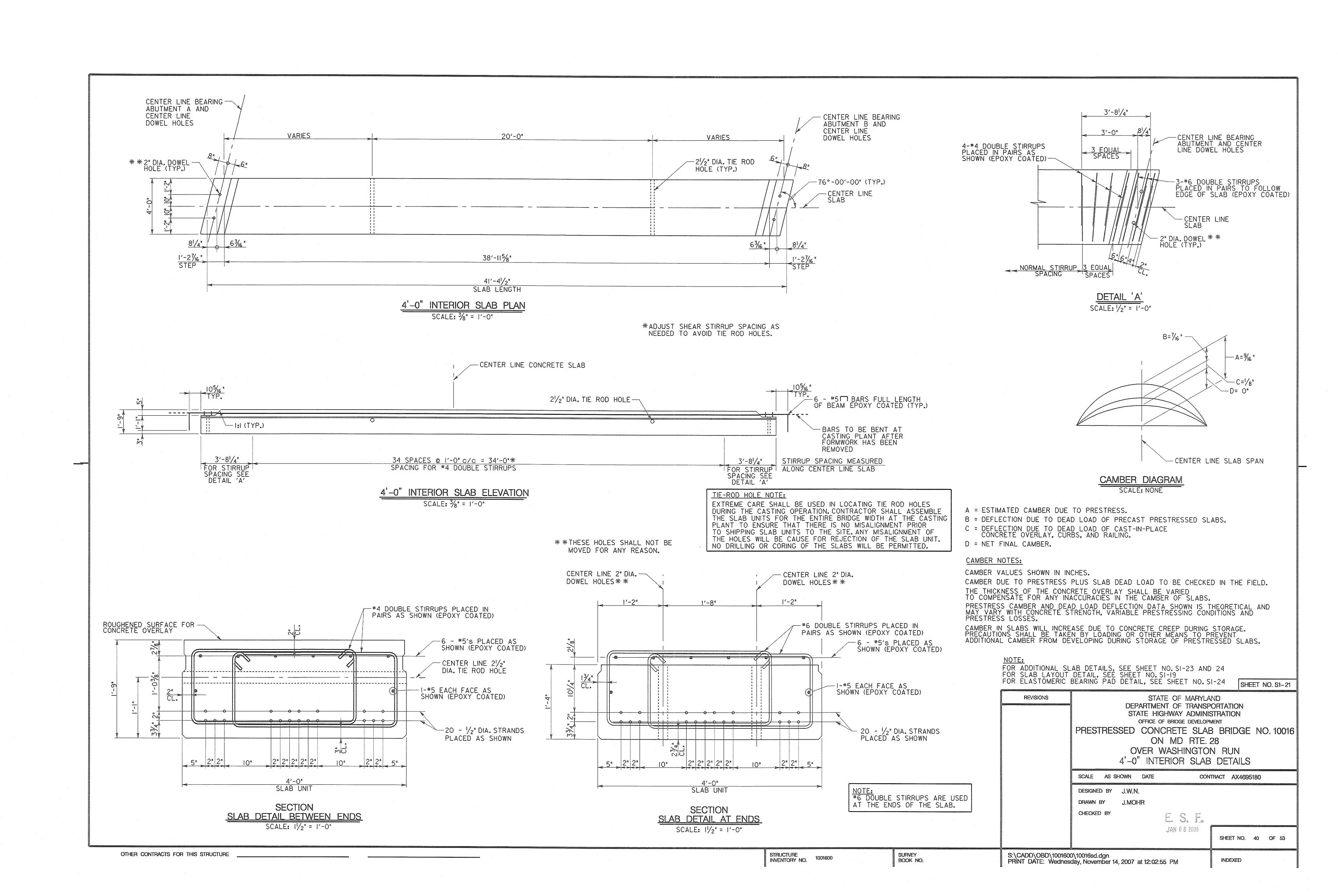


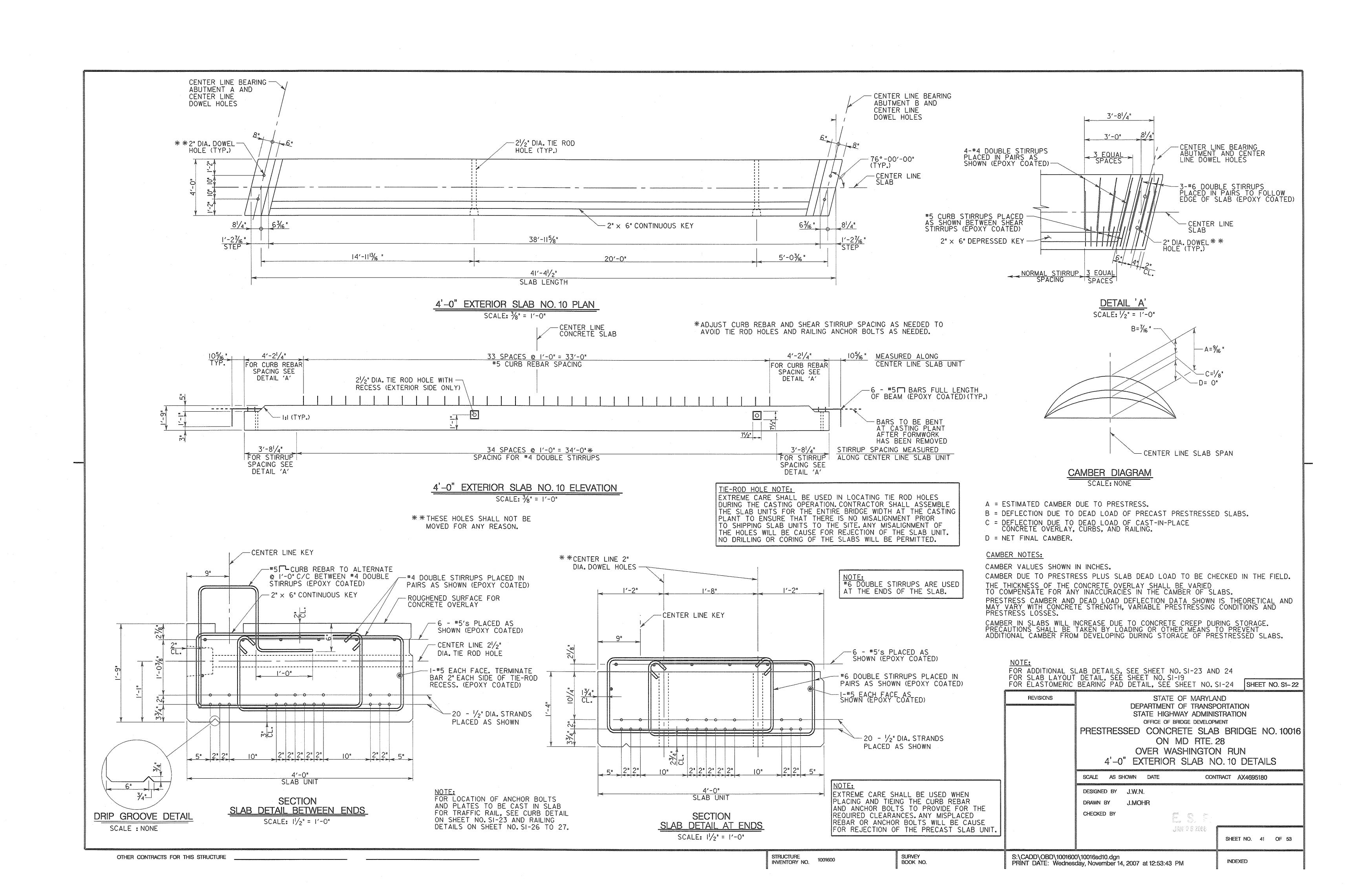


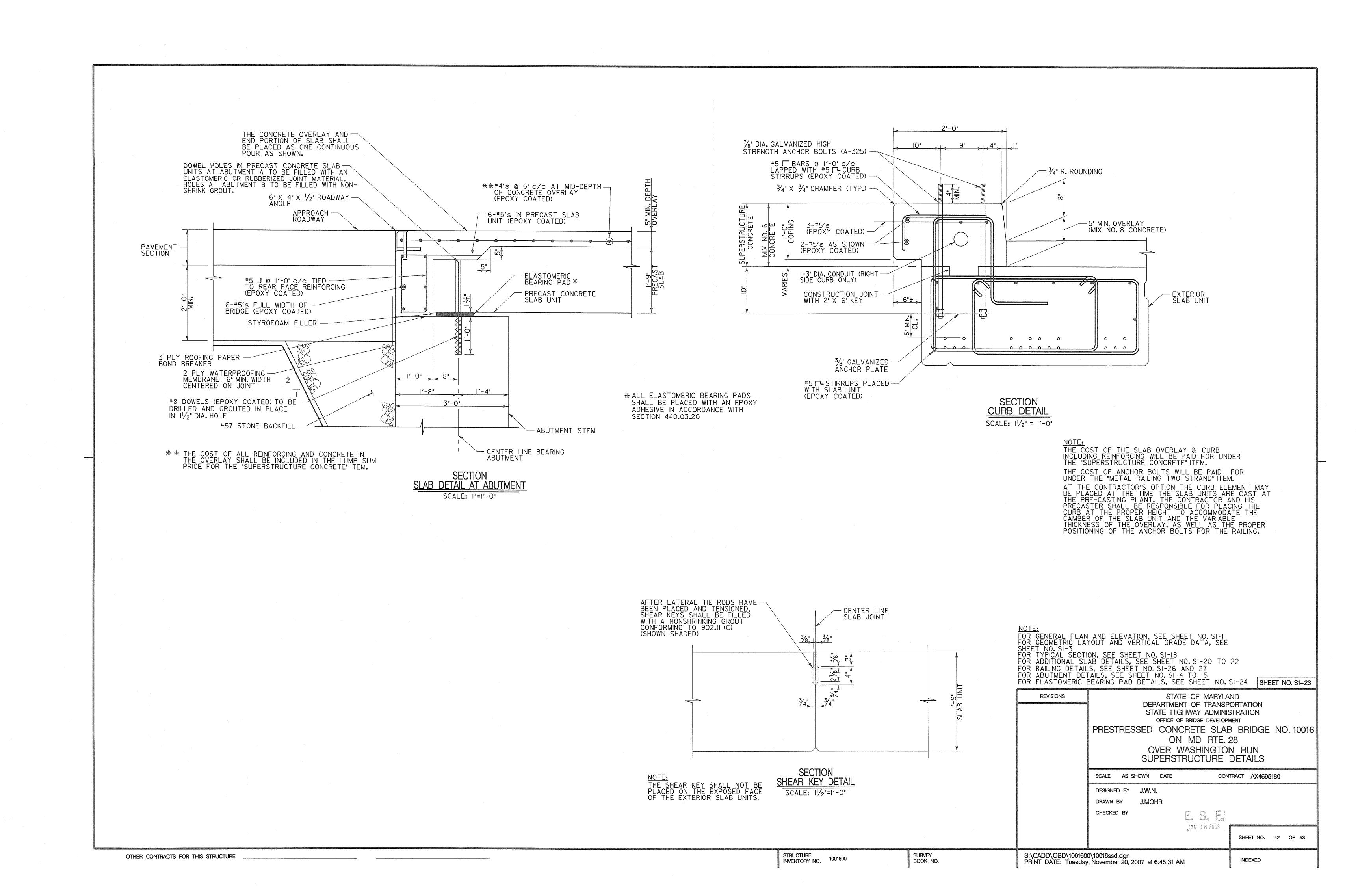


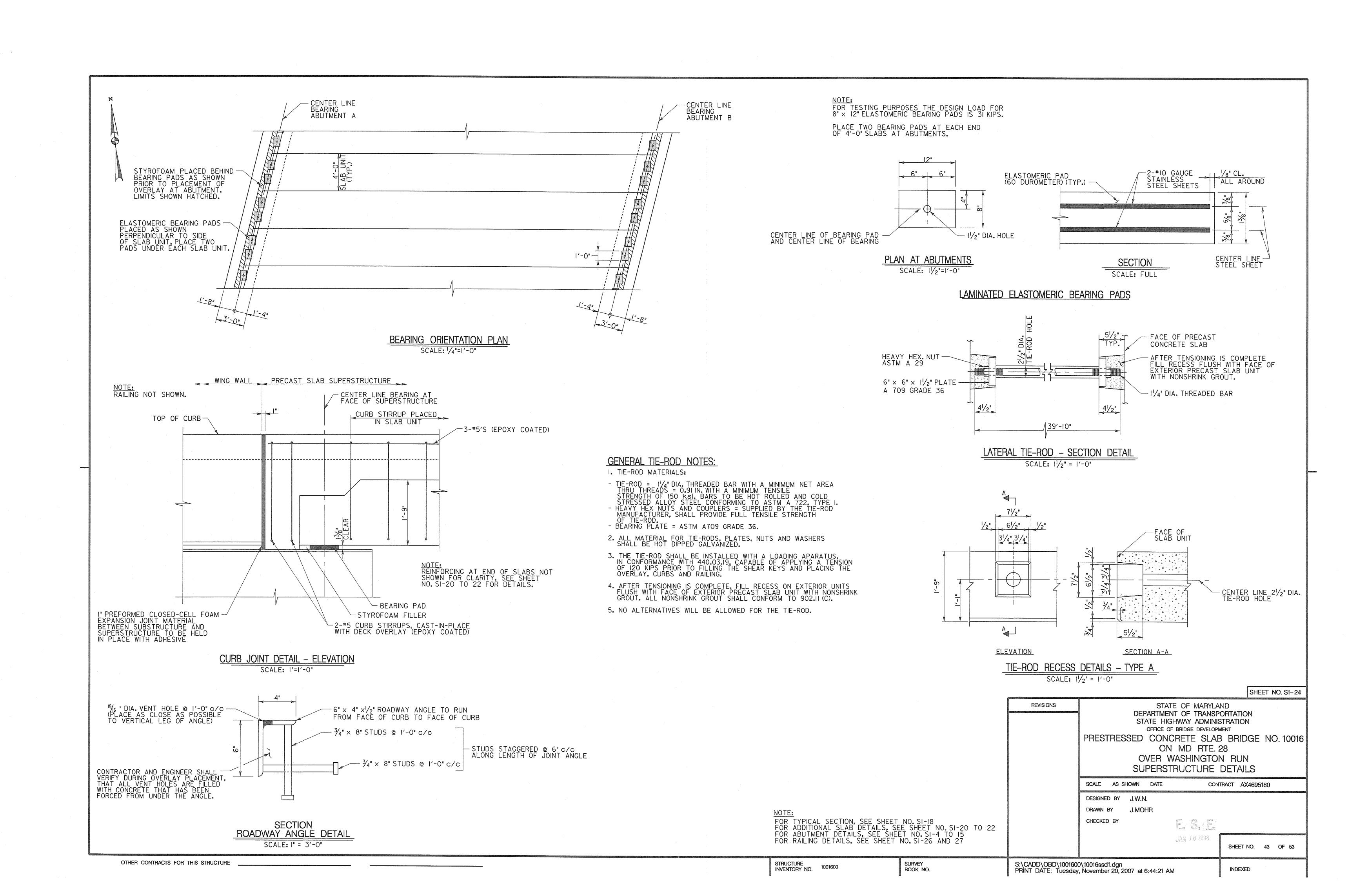


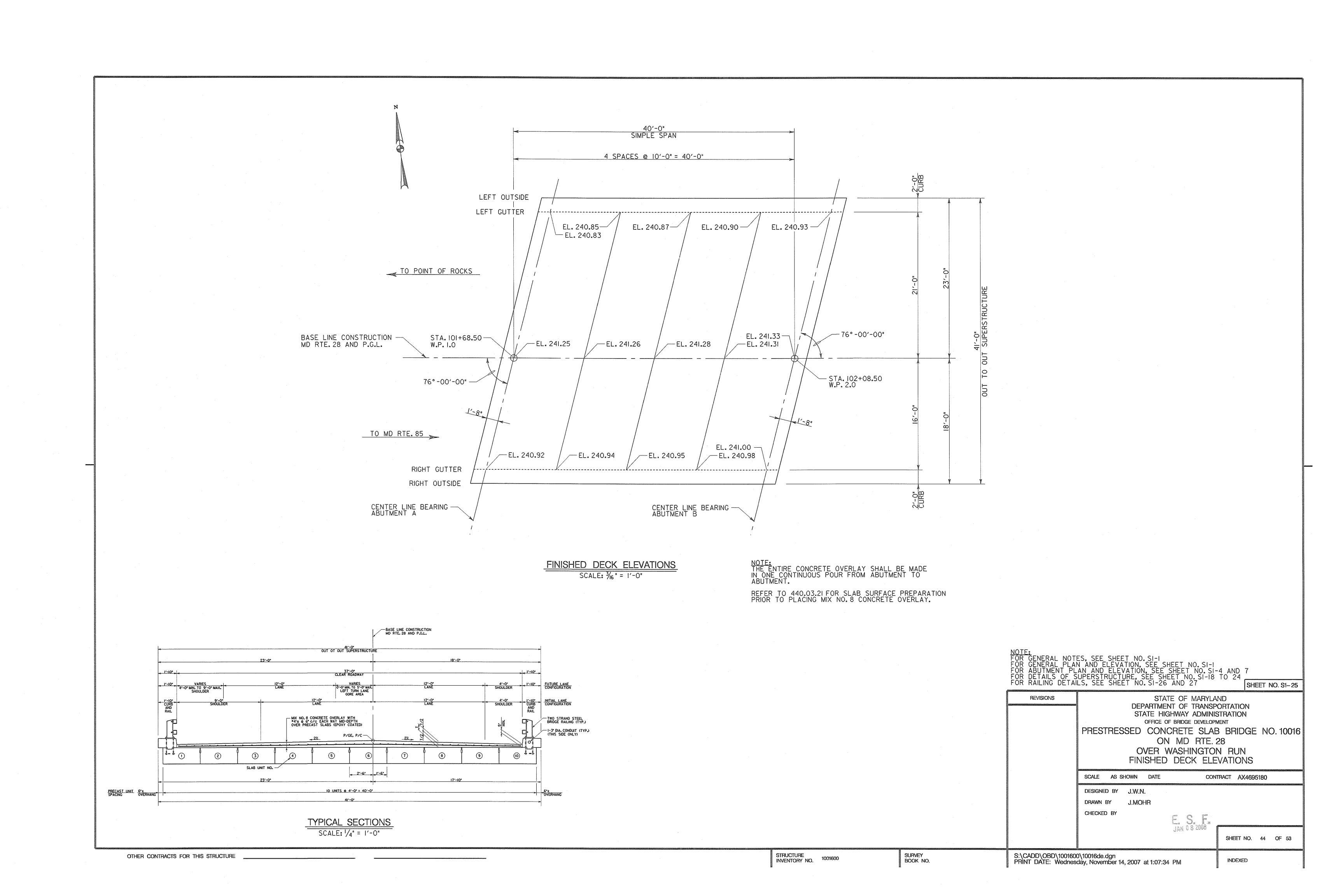


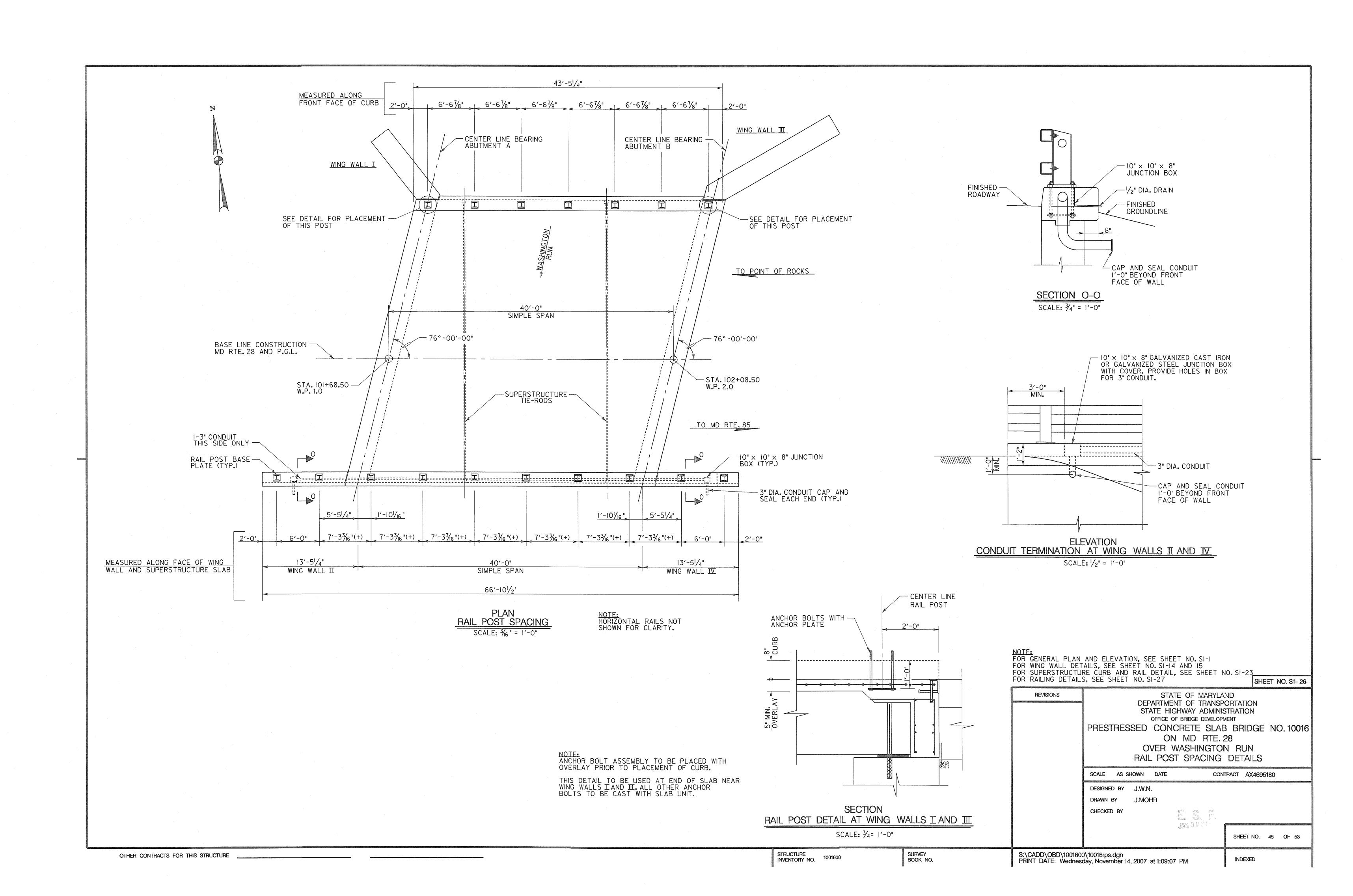


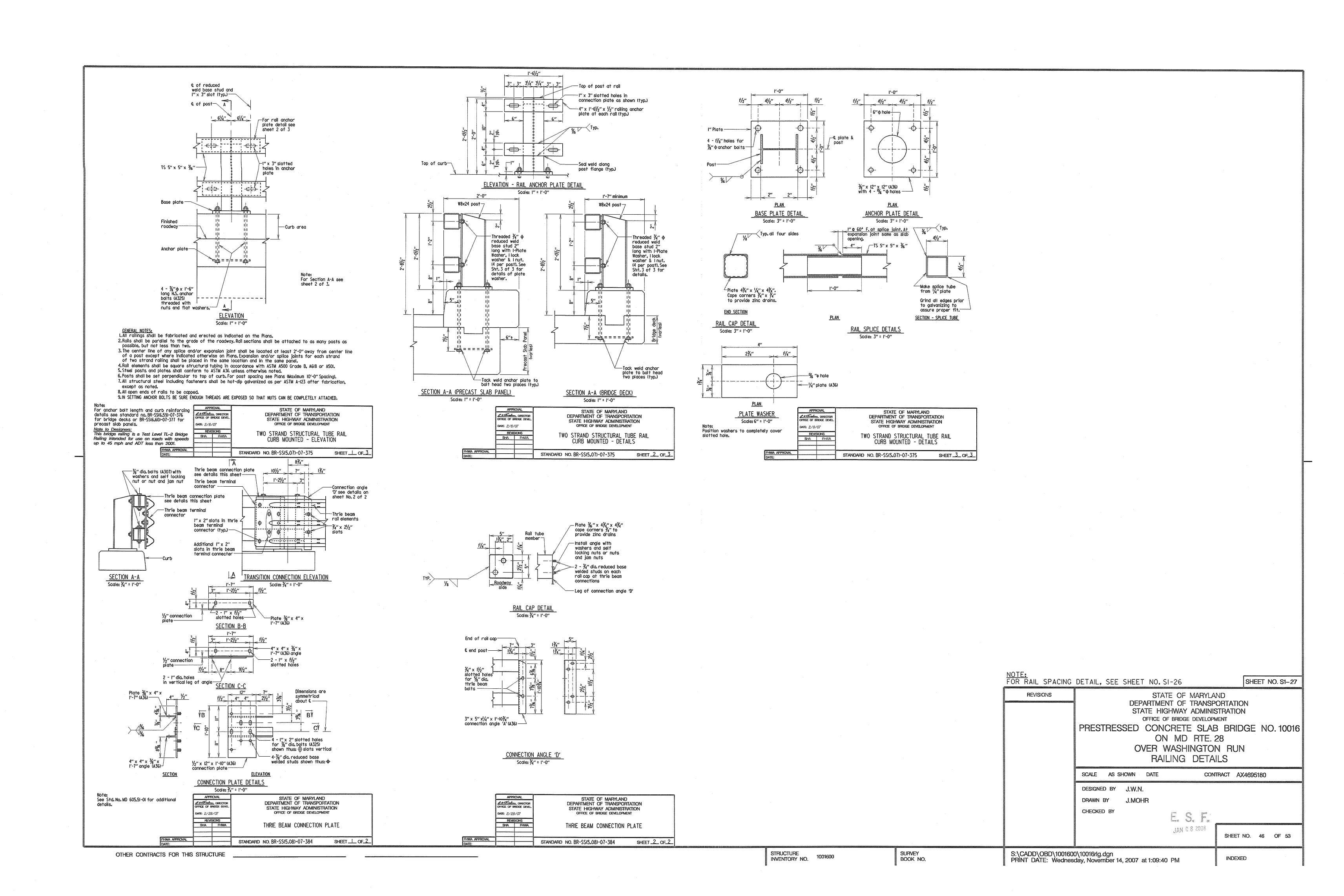


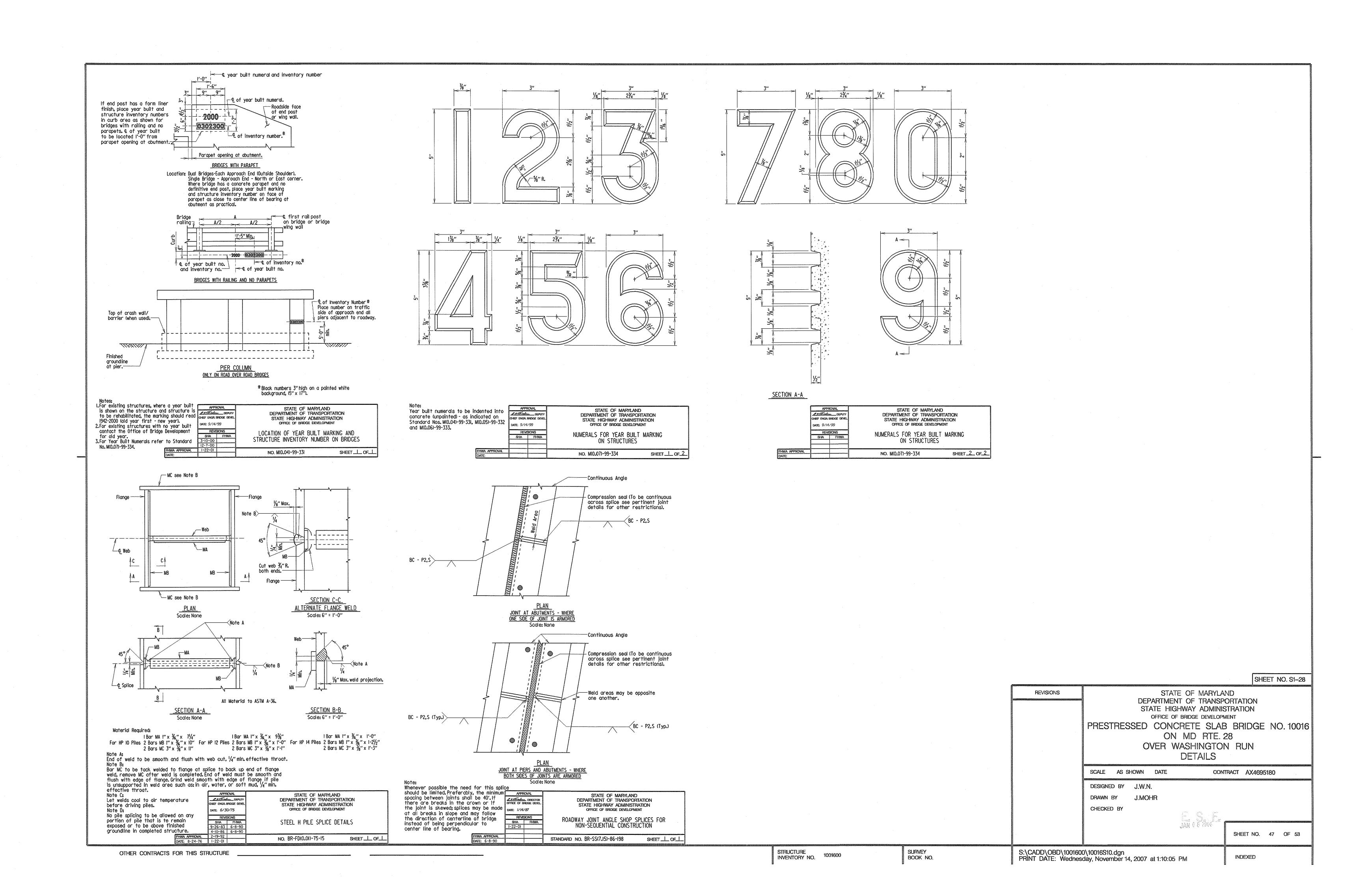


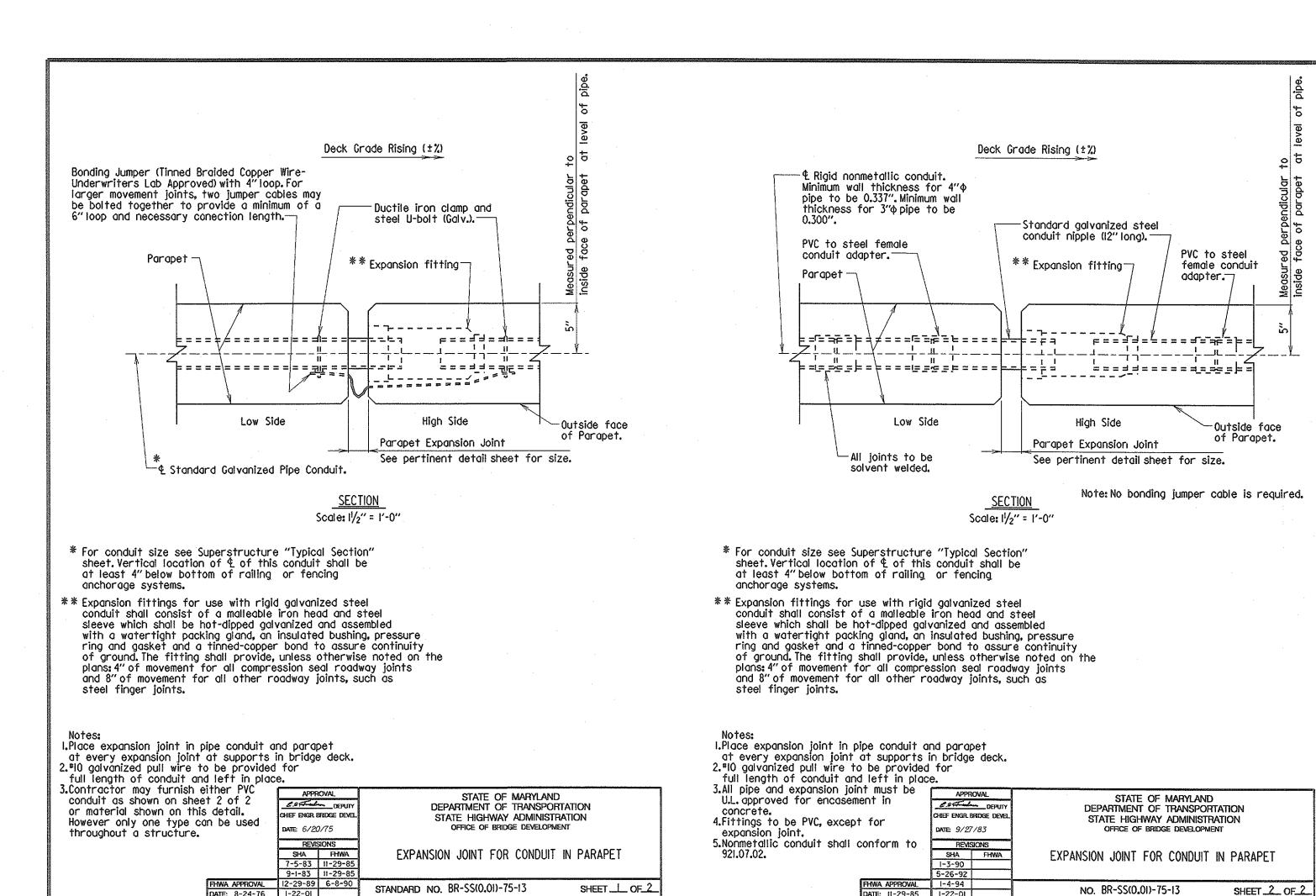


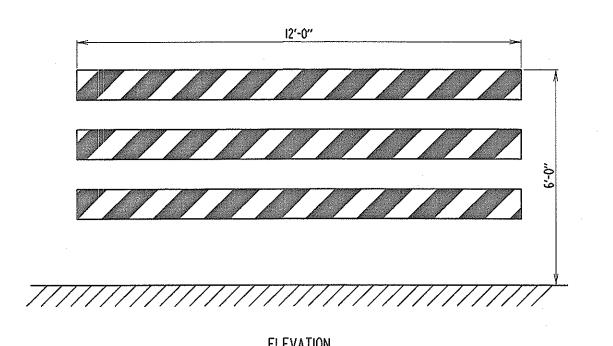










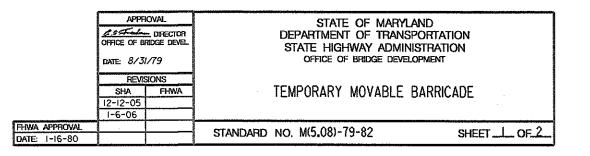


I. Type III Barricade shall conform to NCHRP Report 350 and the MUTCD except that all barricades to close structures shall be 12 ft.long by 6 ft.high.

2. Striping shall be reflectorized alternate orange and white colors. Right (R) Barricade shown. (L) barricade shall have stripes sloping in opposite direction.

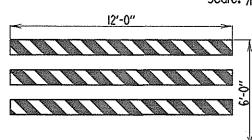
3. Barricade shall be lighted if required by location.

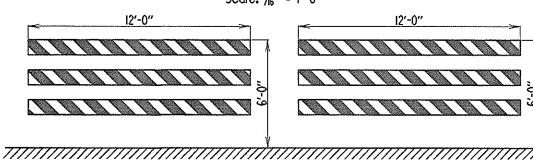
4. Type III Barricades shall be selected from the Preapproved List maintained by the Office of Materials and Technology. Procedures for adding products to the prequalified list may be obtained from the Office of Materials and Technology.



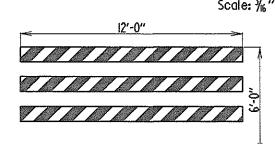
12'-0" 

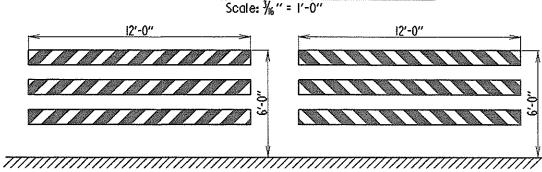
ELEVATION-ROAD CLOSED TRAFFIC TURNING LEFT Scale: 1/6" = 1'-0"



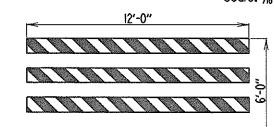


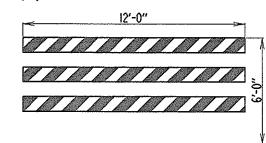
## ELEVATION-ROAD CLOSED TRAFFIC TURNING RIGHT



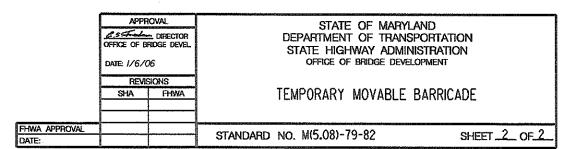


#### ELEVATION-ROAD CLOSED TRAFFIC TURNING EITHER DIRECTION Scale: 1/6" = 1'-0"





Scale: ¾6" = 1'-0"



SHEET NO. S1-29

STATE OF MARYLAND REVISIONS DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT PRESTRESSED CONCRETE SLAB BRIDGE NO. 10016 ON MD RTE. 28 OVER WASHINGTON RUN DETAILS SCALE AS SHOWN DATE CONTRACT AX4695180 DESIGNED BY J.W.N. J.MOHR DRAWN BY CHECKED BY JAN 0 8 2008 SHEET NO. 48 OF 53

INVENTORY NO. 1001600

S:\CADD\OBD\1001600\10016S10a.dgn PRINT DATE: Wednesday, November 14, 2007 at 1:10:30 PM

OTHER CONTRACTS FOR THIS STRUCTURE

BOOK NO.

INDEXED

| BAR        | * L00  | * LOCATION CATEGORY |         |  |  |
|------------|--------|---------------------|---------|--|--|
| SIZE       | А      | В                   | С       |  |  |
| *4         | 2'-5'' | 1'-9"               | l'-5"   |  |  |
| #5         | 3′-0′′ | 2'-2''              | 1′-9"   |  |  |
| <b>*</b> 6 | 3′-7′′ | 2'-7"               | 2'-1"   |  |  |
| <b>*</b> 7 | 4′-10″ | 3′-6″               | 2′-10″  |  |  |
| #8         | 6′-5′′ | 4'-7''              | 3'-8"   |  |  |
| #9         | 8′-1″  | 5′-9′′              | 4′-8′′  |  |  |
| <b>#10</b> | 10'-3" | 7′-4′′              | 5′-11′′ |  |  |
| #[         | 12'-7" | 9'-0"               | 7′-3″   |  |  |

### \* LOCATION CATEGORY

- A Bars in horizontal layers in top of pour with 12" or more of concrete

| below them s       | ucn as in: tootings, pier caps, etc.     |
|--------------------|--|
| B - All bars not i | in Category A spaced less than 6" apart. |
|                    | in Category A spaced 6" or more apart.   |

| BAR        | * L00  | CATION CATEG | ORY          | 3 Times 6 Times |                   | . c/c              |
|------------|--------|--------------|--------------|-----------------|-------------------|--------------------|
| SIZE       | А      | В            | B C Diameter | Diameter        | Bar :<br>Diameter | = Spacing          |
| #4         | 2'-1!" | 2'-7''       | 2'-l''       | 11/2"           | 3"                | 3!/2"              |
| #5         | 3′-8″  | 3′-3"        | 2'-7"        | 11/8"           | 3¾"               | 4¾"                |
| #6         | 4′-5″  | 3′-10″       | 3′-1′′       | 21/4"           | 41/2"             | 5 <sup>1</sup> /4" |
| <b>*</b> 7 | 5′-11″ | 5′-3″        | 4'-2"        | 25/8′′          | 51/4′′            | 61/8"              |
| *8         | 7′-9″  | 6′-10″       | 5′-6″        | 3"              | 6"                | 7"                 |
| #9         | 9'-10" | 8′-8′′       | 6′-11″       | 33/8′′          | 6¾"               | 7%"                |
| *10        | 12'-5" | 11'-0"       | 8′-10"       | 3¾"             | 75%**             | 8¾"                |
| 811        | 15′-3″ | 13'-6"       | 10'-10"      | 4!/4"           | 81/2"             | 9%"                |

### \* LOCATION CATEGORY

I. When bar lap is not specified

2. These bar laps do not apply

when bar is in lightweight

required for this material.

the General Notes indicate

concrete. Greater lengths are

3. These bar laps only apply where

Reinforcing Steel Design, fs=

shall be used.

24,000 p.s.i."

\* LOCATION CATEGORY:

on the Plans, the above dimensions

A - Bars in horizontal layers in top of pour with 12" or more of concrete

\_\_\_\_\_DEPUTY

CHIEF ENGR BRIDGE DEVEL

DATE: 2/2/90

below them such as in: footings, pier caps, etc. B - All bars not in Category A spaced less than 6" apart. C - All bars not in Category A spaced 6" or more apart.

| BAR         | * LOCATION CATEGORY |             |        |  |
|-------------|---------------------|-------------|--------|--|
| SIZE        | А                   | В           | C      |  |
| #4          | 2'-11"              | 2'-1''      | l'-8'' |  |
| #5          | 3′-7"               | 2′-7′′      | 2'-1'' |  |
| <b>*</b> 6  | 4'-4''              | 3′-1′′      | 2′-6″  |  |
| <b>#</b> 7  | 5′-9′′              |             | 3′-4′′ |  |
| #8          | 7′-7′′              | 0000        | 4'-4'' |  |
| #9          | 9′-7′′              | Does<br>Not | 5′-6′′ |  |
| <b>*</b> 10 | 12'-2"              | Exist       | 7′-0′′ |  |
| #11         | 14'-11''            |             | 8′-7′′ |  |

#### \* LOCATION CATEGORY

I. When bar lap is not specified

2.These bar laps do not apply

when bar is in lightweight

concrete.Greater lengths are

3. These bar laps only apply where

"Reinforcing Steel Design, fs=

required for this material.

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on the Plans, the above dimensions

A - Bars in horizontal layers in top of pour with 12" or more of concrete

65 Thulum DEPUTY

iate:2/2/90

CASE NO.2 - For bars coated with epoxy not in Case No.1.

NO. M(6.07)-81-127

STATE OF MARYLAND

DEPARTMENT OF TRANSPORTATION

STATE HIGHWAY ADMINISTRATION

OFFICE OF BRIDGE DEVELOPMENT

BAR LAP DIMENSIONS FOR

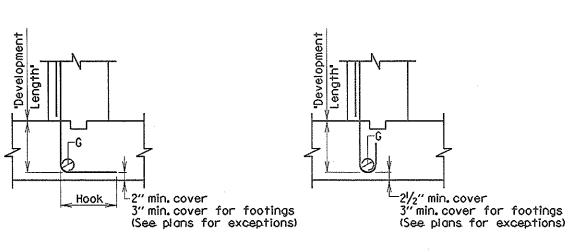
GRADE 60 REINFORCING STEEL

IN MIX NO.3 (3500 P.S.I.) CONCRETE

EPOXY COATED REINFORCING CASE NO.2

SHEET 3 OF 3

- below them such as in: footings, pier caps, etc. B - All bars not in Category A spaced less than 6" apart.
- C All bars not in Category A spaced 6" or more apart.



STANDARD 90° HOOK

STANDARD 180° HOOK

| BAR        | * LOCATION CATEGORY |        |                |  |
|------------|---------------------|--------|----------------|--|
| SIZE       | D                   | E      | F              |  |
| 84         | 8"                  | 11"    | 9"             |  |
| <b>*</b> 5 | 9"                  | '- ''  | 11"            |  |
| <b>#</b> 6 | ''                  | 1'-4'' | '- ''          |  |
| <b>2</b> 7 | '- "                | 1'-6"  | 1'-3"          |  |
| <b>*</b> 8 | 1'-3"               | 1'-9"  | 1'-5"          |  |
| *9         | 1′-5″               | 1'-1 " | 1'-7"          |  |
| <b>#10</b> | 1'-7"               | 2'-2"  | 1'-9"          |  |
| 8          | l'-9"               | 2'-5"  | <b>'-</b>   '' |  |

#### APPROVAL 1-22-01 DATE: 6-8-90 9-20-05

| RECOMME     | ENDED END                             | HOOKS ALL              | GRADES               |
|-------------|---------------------------------------|------------------------|----------------------|
| BAR<br>SIZE | Finished<br>bend<br>diameter<br>G in. | 180<br>Degree<br>hooks | 90<br>Degre<br>hook: |
| 84          | 3"                                    | 6"                     | 8"                   |
| #5          | 3¾"                                   | 7′′                    | 10"                  |
| 88          | 41/2"                                 | 8"                     | 1′-0′′               |
| #7          | 51/4"                                 | 10"                    | 1'-2"                |
| 8*          | 6"                                    | II"                    | 1'-4''               |
| #9          | 91/2"                                 | 1'-3"                  | 1'-7''               |
| *10         | 10¾"                                  | 1'-5"                  | 1'-10'               |
| <b>\$  </b> | 1′-0″                                 | 1'-7"                  | 2'-0'                |
|             |                                       |                        |                      |

#### \* LOCATION CATEGORY:

- D-All bars terminating with a standard 180°hook with side cover (normal to plane of hook) not less than 2½, and for 90°deg, hook, cover on bar extension beyond hook not less than 2".
- E- All bars <u>not</u> in Category D. F- All bars with hook enclosed vertically or horizontally within ties or stirrup-ties spaced along the full development length not greater than 3d where d is the diameter of the hooked bar.

I. When development length is not specified on the Plans, the above dimensions shall be used. 2. These development lengths do not apply when bar is

in lightweight concrete or any other strength of concrete. 3. These development lengths only apply where the General Notes indicate "Reinforcing

Steel Design, fs= 24,000 p.s.i.'
4.If depth of member does not allow bar development length indicated in Categories A, B, and C: Std. No. M(6.14)-90-214; then hook shall be added to all bars not conforming.

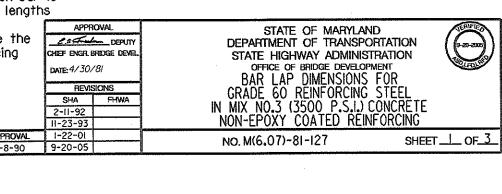
as per D,E & F.

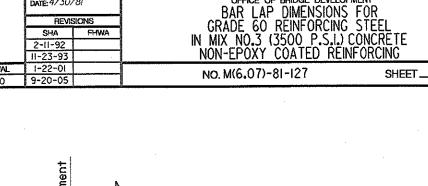
DATE: 4/4/86

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT DEVELOPMENT LENGTH DIMENSIONS OF HOOKED BARS FOR GRADE 60 REINFORCING STEEL IN MIX NO. 3 (3500 P.S.I.) CONCRETE



| Note: I. When bar lap is not specified on the above dimensions shall be used.                 |                          |   |               |
|---|--------------------------|---|---------------|
| <ol><li>These bar laps do not apply when t<br/>in lightweight concrete. Greater len</li></ol> |                          |   |               |
| are required for this material.  3.These bar laps only apply where th                         | APPROVAL.  DEPUTY        | STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION            | TERV<br>D-20- |
| General Notes indicate "Reinforcing<br>Steel Design, fs= 24,000 p.s.i."                       | CHEF ENGR. BRIDGE DEVEL. | STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT |               |
| 3.33. 333.3., .3 E 11,000 prom  | REVISIONS                | BAR LAP DIMENSIONS FOR                                    | _             |





-2" min. cover

STANDARD STRAIGHT BAR

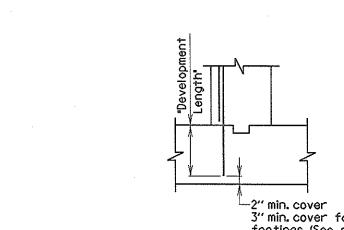
\*4 1′-5″ 1′-0″ 1′-0″ #5 | 1'-9'' | 1'-3" | 1'-0" \*6 2'-2" l'-6" l'-3" \*7 2'-11" 2'-1" 1'-8" \*8 3'-9" 2'-9" 2'-2" \*9 4'-9" 3'-5" 2'-9" \*IO 6'-I" 4'-4" 3'-6"

#II 7'-5" 5'-4" 4'-3"

LOCATION CATEGORY

3" min, cover for footings (See plans

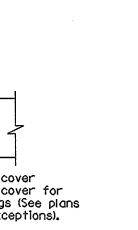
for exceptions).



|          | 1   |
|----------|---|
|          | -2" min. cover 3" min. cover for footings (See plans for exceptions). |
| STANDARD | STRAIGHT BAR  |

| 2" min. cover 3" min. cover footings (See for exception | plans |
|---|-------|
| STANDARD STRAIGHT BAR                                   |       |

| BAR        | * LOCA | ATION CATE | GORY   |
|------------|--------|------------|--------|
| SIZE       | Α      | В          | С      |
| 84         | 1'-9"  | 1'-3"      | 1'-0"  |
| <b>*</b> 5 | 2'-2"  | 1'-6"      | 1′-3′′ |
| #6         | 2'-7"  | 1'-10"     | 1'-6"  |
| <b>27</b>  | 3′-5″  |            | 2'-0"  |
| <b>\$8</b> | 4'-6"  | Does       | 2'-7"  |
| <b>89</b>  | 5′-7″  | Not        | 3′-3″  |
| *10        | 7'-2"  | '-2" Exist |        |
|            | 4      | 4          |        |



CASE NO.1 - For bars coated with epoxy with cover less than 3 times the bar diameter or clear spacing between bars less than 6 times the bar diameter.

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION

STATE HIGHWAY ADMINISTRATION

SHEET\_2\_OF\_3\_

OFFICE OF BRIDGE DEVELOPMENT
BAR LAP DIMENSIONS FOR
GRADE 60 REINFORCING STEEL
IN MIX NO.3 (3500 P.S.I.) CONCRET
EPOXY COATED REINFORCING CASE

NO. M(6.07)-81-127

| BAR        | * LOCATION CATEGORY |        |        | 3 Times<br>Bar | 6 Times<br>Bar = | . c/c   |
|------------|---------------------|--------|--------|----------------|------------------|---------|
| SIZE       | Α                   | 8      | С      | Diameter       | Diameter         | Spacing |
| <b>84</b>  | 1'-9"               | 1'-6"  | 1′-3″  | 11/2"          | 3"               | 31/2"   |
| <b>*</b> 5 | 2'-2''              | '-  "  | 1'-6"  | 1%"            | 3¾"              | 43/8"   |
| . *6       | 2'-7''              | 2'-3"  | 1'-10" | 21/4"          | 41/2"            | 51/4"   |
| <b>#</b> 7 | 3′-6″               | 3′-1″  | 2'-6"  | 25/8"          | 51/4"            | 61/8"   |
| #8         | 4'-7''              | 4'- "  | 3′-3″  | 3′′            | 6"               | 7"      |
| <b>#9</b>  | 5'-9"               | 5′-1″  | 4'- "  | 33/8"          | 6¾"              | 71/8"   |
| *10        | 7'-4''              | 6'-6"  | 5′-2″  | 33/4"          | 75/8"            | 8¾"     |
| #          | 9'-0"               | 7′-11″ | 6'-4"  | 41/4"          | 81/2"            | 9%"     |

A-Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in:

footings, pier caps, etc.
B- All bars not in Category A spaced less than 6" apart.
C- All bars not in Category A spaced 6" or more apart.

# LOCATION CATEGORY:

- A-Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in:
- footings,pier caps,etc. B-All bars not in Category A spaced less than 6" apart. C-All bars not in Category A spaced 6" or more apart.

\*II 8'-I0'' 5'-I''

I. When development length is not specified CASE NO.2 - For bars coated with epoxy not in Case No.1. on the Plans, the above dimensions

shall be used. 2. These development lengths do not apply

when bar is in lightweight concrete or any other strength of concrete. STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION 3. These bar laps only apply where CHIEF ENGR. BRIDGE DEVEL STATE HIGHWAY ADMINISTRATION the General Notes indicate DEVELOPMENT LENGTH DIMENSIONS FOR GRADE 60 REINFORCING STEEL
IN MIX NO.3 (3500 P.S.I.) CONCRETE EPOXY COATED REINFORCING CASE NO.2 DATE:2/2/90 "Reinforcing Steel Design, fs= 24,000 p.s.i.' FHWA APPROVAL | NO. M(6.14)-90-214 SHEET 3 OF 3 SHEET NO. S1-30

#### REVISIONS STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT PRESTRESSED CONCRETE SLAB BRIDGE NO. 10016 ON MD RTE. 28 OVER WASHINGTON RUN REINFORCING DETAILS SCALE AS SHOWN DATE CONTRACT AX4695180 DESIGNED BY J.W.N. J.MOHR DRAWN BY

CHECKED BY

JAN 0 8 2008

SHEET NO. 49 OF 53

| * | LOCATION | CATEGORY: |
|---|----------|-----------|

- A-Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in:
- footings, pier caps, etc.
  B- All bars not in Category A spaced less than 6" apart.
  C- All bars not in Category A spaced 6" or more apart.

#### I. When development length is not specified on the Plans, the above dimensions shall be used. 2. These development lengths do not apply when bar is in lightweight concrete or any other

strength of concrete. 3. These development lengths only apply where the Steel 4.lf dep

| Consent Marine Indicate Pointage   |  |   |        |
|--|--|---|--------|
| General Notes indicate "Reinforcing Steel Design, fs= 24,000 p.s.i."  4.If depth of member does not allow bar development length indicated in Categories A, B, and C: then hook shall be added to all bars not conforming, as per D, E, and F per Std. No. M(6.08)-86-178. | APPROVAL  LECTURE DEPUTY CHEF ENGR. BRIDGE DEVEL DATE: 2/2/90  REVISIONS SHA FHWA II-23-93 I-22-01 | STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT DEVELOPMENT LENGTH DIMENSIONS FOR GRADE 60 REINFORCING STEEL IN MIX NO.3 (3500 P.S.L.) CONCRETE NON-EPOXY COATED REINFORCING |        |
| FHWA APPROVAL  | 9-20-05  | NO. M(6.14)-90-214 SHEET.   | 1 of 3 |
| (DAIE:   | 8 I I  |   |        |

24,000 p.s.i."

I. When development length is not specified on the Plans, the above dimensions shall be used.

CASE NO.1 - For bars coated with epoxy with cover less than 3 times the bar diameter or clear spacing between bars less than 6 times the bar diameter. 2. These development lengths do not apply when bar is in lightweight concrete APPROVAL STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION or any other strength of concrete. 3. These bar laps only apply where CHIEF ENGR. BRIDGE DEVE STATE HIGHWAY ADMINISTRATION the General Notes indicate OFFICE OF BRIDGE DEVELOPMENT
DEVELOPMENT LENGTH DIMENSIONS FOR
GRADE 60 REINFORCING STEEL
IN MIX NO.3 (3500 P.S.I.) CONCRETE
EPOXY COATED REINFORCING CASE NO.I DATE: 2/2/90 "Reinforcing Steel Design, fs= SHEET 2 OF 3 NO. M(6.14)-90-214

SURVEY BOOK NO.

S:\CADD\OBD\1001600\10016s13.dgn

PRINT DATE: Wednesday, November 14, 2007 at 1:10:55 PM

OTHER CONTRACTS FOR THIS STRUCTURE

INVENTORY NO.

INDEXED

|            |                     | •      |        |  |
|------------|---------------------|--------|--------|--|
| BAR        | * LOCATION CATEGORY |        |        |  |
| SIZE       | Α                   | В      | С      |  |
| #4         | 2′-5″               | 1'-9"  | 1′-5″  |  |
| *5         | 3'-0"               | 2′-2″  | 1'-9"  |  |
| #6         | 3′-7′′              | 2′-7″  | 2'-1"  |  |
| <b>*</b> 7 | 4'-4"               | 3′-1″  | 2′-6″  |  |
| *8         | 5′-8″               | 4'-1"  | 3′-3"  |  |
| #9         | 7′-2″               | 5′-1″  | 4'- '' |  |
| #10        | 9'-1"               | 6′-6″  | 5′-2″  |  |
| 83]        | 11'-1"              | 7′-11″ | 6'-4'' |  |

### \* LOCATION CATEGORY

- A Bars in horizontal layers in top of pour with 12" or more of concrete
- below them such as in: footings, pier caps, etc. B - All bars not in Category A spaced less than 6 inches apart. C - All bars not in Category A spaced 6 inches or more apart.

| BAR        | * L00   | CATION CATEG | ORY     | 3 Times           | 6 Times c/c     |         |
|------------|---------|--------------|---------|-------------------|-----------------|---------|
| SIZE       | Α       | В            | С       | - Bar<br>Diameter | Bar<br>Diameter | Spacing |
| <b>*</b> 4 | 2'-11"  | 2′-7′′       | 2'-1"   | 11/2"             | 3"              | 31/2"   |
| <b>*</b> 5 | 3′-8″   | 3′-3′′       | 2'-7"   | 17/8"             | 3¾"             | 43/8"   |
| <b>*</b> 6 | 4′-5″   | 3′-10′′      | 3′-1″   | 21/4"             | 41/2"           | 51/4"   |
| <b>#</b> 7 | 5′-3″   | 4'-7''       | 3′-8′′  | 25/8"             | 51/4"           | 61/8"   |
| #8         | 6′-10″  | 6′-1"        | 4'-10'' | 3"                | 6"              | 7"      |
| #9         | 8′-8′′  | 7′-8′′       | 6′-1″   | 33%"              | 6¾"             | 71/8"   |
| <b>*10</b> | II'-0'' | 9′-8′′       | 7′-9′′  | 3¾"               | 75/8"           | 81/8"   |
| #11        | 13′-6″  | '-  ''       | 9′-6"   | 41/4"             | 81/2"           | 9¾"     |

### \* LOCATION CATEGORY

- A Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in: footings, pier caps, etc.
- B All bars not in Category A spaced less than 6 inches apart. C - All bars not in Category A spaced 6 inches or more apart.

| BAR        | * LOCATION CATEGORY |                      |        |  |
|------------|---------------------|----------------------|--------|--|
| SIZE       | Α                   | В                    | С      |  |
| #4         | 2'-9"               | 2'-0"                | 1'-7"  |  |
| <b>*</b> 5 | 3′-6"               | 2'-6"                | 2′-0″  |  |
| #6         | 4'-2"               | 3'-0"                | 2′-5″  |  |
| <b>*</b> 7 | 4'-11"              | Does<br>Not<br>Exist | 2′-10″ |  |
| <b>*</b> 8 | 6′-6″               |                      | 3′-9"  |  |
| #9         | 8'-2"               |                      | 4'-8'' |  |
| *10        | 10′-5″              |                      | 6′-0′′ |  |
| <u>#  </u> | 12'-9"              |                      | 7'-4'' |  |

### \* LOCATION CATEGORY

the above dimensions shall be used.

General Notes indicate "Reinforcing

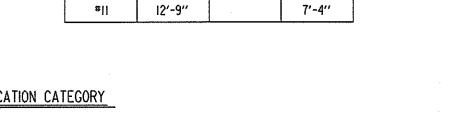
are required for this material.

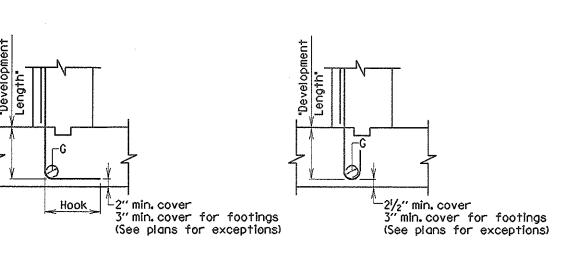
Steel Design, fs= 24,000 p.s.i."

2. These bar laps do not apply when bar is

in lightweight concrete. Creater lengths

3. These bar laps only apply where the





STANDARD 90° HOOK

\*4 7" 9"

**"**6 | 10" | 1'-2"

\*8 | 1'-1" | 1'-6" \*9 l'-3" l'-9" \$10 |'-4" |'-II" \*!I I'-6" 2'-2"

STANDARD 180° HOOK

| \$ 100 | TION OATE  | 200V  |
|--------|------------|-------|
| LUCA   | ATION CATE | JURY  |
| D      | Ε          | F     |
| 7′′    | 9"         | 8"    |
| 8"     | 1'-0"      | 9"    |
| 10"    | 1′-2′′     | ll"   |
| II"    | 1'-4"      | ['- " |
| l'-l"  | 1'-6"      | 1'-3" |
| 1'-3"  | 1'-9"      | 1′-5″ |
| l'-4'' | l'-II''    | 1'-7" |
| 1′-6″  | 2'-2"      | 1'-9" |

| RECOMMENDED END HOOKS ALL GRADES |                                       |                        |                       |  |  |
|----------------------------------|---------------------------------------|------------------------|-----------------------|--|--|
| BAR<br>SIZE                      | Finished<br>bend<br>diameter<br>G in. | 180<br>Degree<br>hooks | 90<br>Degree<br>hooks |  |  |
| <b>*</b> 4                       | 3″                                    | 6"                     | 8"                    |  |  |
| <b>*</b> 5                       | 3¾′′                                  | 7''                    | 10"                   |  |  |
| <b>≈</b> 6                       | 41/2"                                 | 8"                     | 1′-0″                 |  |  |
| 87                               | 51/4"                                 | 10"                    | 1'-2"                 |  |  |
| <b>8</b> 8                       | 6"                                    | 11"                    | 1'-4''                |  |  |
| <b>8</b> 9                       | 91/2"                                 | 1'-3"                  | l'-7''                |  |  |
| s10                              | 10¾"                                  | 1′-5″                  | 1'-10"                |  |  |
| *                                | 1'-0''                                | 1′-7′′                 | 2'-0"                 |  |  |

- \* LOCATION CATEGORY:
- D- All bars terminating with a standard 180° hook with side cover (normal to plane of hook) not less than 2½ in., and for 90°deg. hook, cover on bar extension beyond hook not less than 2 in.
- F- All bars with hook enclosed vertically or horizontally within ties or stirrup-ties spaced along the full development length not greater than 3d where d is the diameter of the hooked bar.

# I. When development length is not specified on the plans, the above dimensions shall be used.

- 2. These development lengths do not apply when bar is in lightweight concrete or any other strength of concrete.
- 3. These development lengths only apply where the General Notes indicate "Reinforcing
- Steel Design, fs= 24,000 p.s.i." 4.If depth of member does not allow bar development length indicated in Categories A, B, and C: Std. No. M(6.16)-90-216; then hook shall be added to all bars not conforming,

as per D,E & F.

DEPARTMENT OF TRANSPORTATION CHIEF ENGR BRIDGE DEVE STATE HIGHWAY ADMINISTRATION DATE: 4/4/86 OFFICE OF BRIDGE DEVELOPMENT

DEVELOPMENT LENGTH DIMENSIONS OF HOOKED BARS FOR GRADE 60 REINFORCING STEEL IN MIX NO. 6 (4500 P.S.I.) CONCRETE NO. M(6,10)-86-180 SHEET \_\_\_\_ OF\_\_\_

STATE OF MARYLAND

A - Bars in horizontal layers in top of pour with 12" or more of concrete

I. When bar lap is not specified on the plans, CASE NO.2 - For bars coated with epoxy not in Case No.1.

STATE OF MARYLAND

DEPARTMENT OF TRANSPORTATION

STATE HIGHWAY ADMINISTRATION

GRADE 60 REINFORCING STEEL IN MIX NO. 6 (4500 P.S.I.) CONCRE

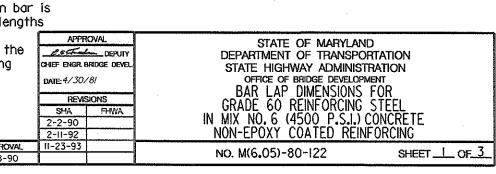
NO. M(6.05)-80-122

EPOXY COATED REINFORCING CASE NO.2

SHEET <u>3</u> OF <u>3</u>

below them such as in: footings, pier caps, etc. B - All bars not in Category A spaced less than 6 inches apart. C - All bars not in Category A spaced 6 inches or more apart.

I. When bar lap is not specified on the plans, the above dimensions shall be used. 2. These bar laps do not apply when bar is in lightweight concrete. Greater lengths are required for this material. STATE OF MARYLAND 3. These bar laps only apply where the L.S. C. Tandam DEPUTY DEPARTMENT OF TRANSPORTATION General Notes indicate 'Reinforcing STATE HIGHWAY ADMINISTRATION Steel Design, fs= 24,000 p.s.i. DATE: 4/30/8/



" min. cover

STANDARD STRAIGHT BAR

\*4 1'-5" 1'-0" 1'-0"

\*4 | 1'-5" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-3" | 1'-0" | 1'-3" | 1'-0" | 1'-3" | 1'-0" | 1'-3" | 1'-6" | 1'-3" | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-10" | 1'-6" | 1'-10" | 1'-6" | 1'-10" | 1'-6" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-10" | 1'-1

\*II 6'-7" 4'-8" 3'-9"

A-Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in:

DEPUTY

CHIEF ENGR. BRIDGE DEVEL

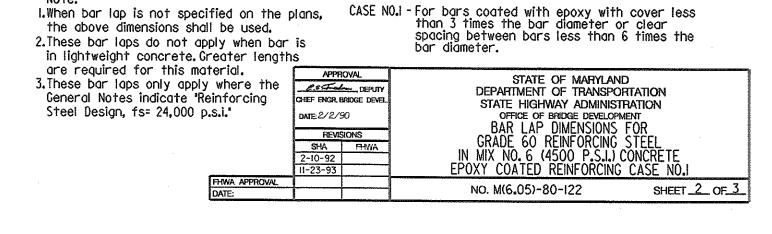
\* LOCATION CATEGORY

A B C

3" min. cover for

for exceptions).

footings (See plans



-2" min. cover

STANDARD STRAIGHT BAR

1'-9" 1'-6" 1'-3" 11/2" 3"

2'-2" | 1'-1|" | 1'-6" | 1\[\frac{1}{8}\]" | 3\[\frac{3}{4}\]" | 2'-7" | 2'-3" | 1'-10" | 2\[\frac{1}{4}\]" | 4\[\frac{1}{2}\]"

\*7 3'-1" 2'-9" 2'-2" 25%" 51/4" 61/8"

\*8 4'-1" 3'-7" 2'-10" 3" 6" 7"

\*9 5'-1" 4'-6" 3'-7" 33/8" 63/4" 73/8"

\*10 6'-6" 5'-9" 4'-7" 33/4" 75%" 87/8"

#II 7'-II" 7'-0" 5'-7" 4<sup>1</sup>/<sub>4</sub>" 8<sup>1</sup>/<sub>2</sub>" 9<sup>7</sup>/<sub>8</sub>"

A-Bars in horizontal layers in top of pour with 12" or more of concrete below them such as in:

LOCATION CATEGORY

\* LOCATION CATEGORY:

footings,pier caps,etc.

I. When development length is not specified on the

plans, the above dimensions shall be used.

B- All bars not in Category A spaced less than 6" apart. C- All bars not in Category A spaced 6" inches or more apart.

3" min. cover for

for exceptions).

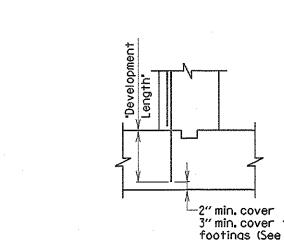
footings (See plans

|Diameter | Diameter | Spacing

CASE NO.1 - For bars coated with epoxy with cover less than 3 times the bar diameter or clear

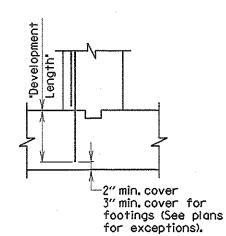
spacing between bars less than 6 times the

SHEET 2 OF 3



DATE: 2/2/90

| BAR         | * LOCA | * LOCATION CATE |        |  |
|-------------|--------|-----------------|--------|--|
| SIZE        | Α      | В               | C      |  |
| <b>#</b> 4  | l'-8'' | 1'-2"           | 1'-0'' |  |
| <b>\$</b> 5 | 2'-1"  | 1′-6″           | 1'-2"  |  |
| <b>*</b> 6  | 2'-5"  | 1'-9"           | 1′-5′′ |  |
| #7          | 2'-11" |                 | 1′-8′′ |  |
| #8          | 3'-10" | Does            | 2′-3′  |  |
|             | T .    | 1               |        |  |



| STANDARD | STRAIGHT | BAR |
|----------|----------|-----|
|          |          |     |

| BAR         | * LOCATION CATEGORY |                      |        |  |
|-------------|---------------------|----------------------|--------|--|
| SIZE        | A                   | В                    | С      |  |
| <b>\$4</b>  | 1′-8″               | 1'-2"                | 1'-0"  |  |
| <b>\$</b> 5 | 2'-1"               | 1'-6"                | 1'-2"  |  |
| ¤6          | 2'-5"               | 1'-9"                | 1′-5″  |  |
| 87          | 2'-11"              |                      | 1'-8'' |  |
| <b>₽8</b>   | 3′-10″              | Does<br>Not<br>Exist | 2′-3′′ |  |
| 89          | 4'-10"              |                      | 2'-9"  |  |
| <b>*</b> 10 | 6'-2"               |                      | 3'-6"  |  |
| #           | 7'-6"               |                      | 4'-4'' |  |

- **LOCATION CATEGORY:** A-Bars in horizontal layers in top of pour with 12 or more of concrete below them such as in: footings, pier caps, etc.

  B- All bars not in Category A spaced less than 6 apart.

  C- All bars not in Category A spaced 6 inches or more apart.

SHEET NO. S1-31

REVISIONS STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT PRESTRESSED CONCRETE SLAB BRIDGE NO. 10016 ON MD RTE. 28 OVER WASHINGTON RUN REINFORCING DETAILS

SCALE AS SHOWN DATE CONTRACT AX4695180 DESIGNED BY J.W.N.

DRAWN BY J.MOHR CHECKED BY

JAN 0 8 2008 SHEET NO. 50 OF 53

CASE NO.2 - For bars coated with epoxy not in Case No.1.

I. When development length is not specified on the plans, the above dimensions shall be used. 2. These development lengths do not apply when bar is

in lightweight concrete or any other strength of concrete. 3. These development lengths only apply where the General Notes indicate "Reinforcing Steel Design, fs= 24,000 p.s.i."

STATE OF MARYLAND P.S. Frake DEPUTY CHIEF ENGR. BRIDGE DEVEL. DATE: 2/2/90

DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION DEVELOPMENT LENGTH DIMENSIONS FOR GRADE 60 REINFORCING STEEL
IN MIX NO.6 (4500 P.S.I.) CONCRETE EPOXY COATED REINFORCING CASE NO.2 NO. M(6.16)-90-216 SHEET 3 OF 3

2. These development lengths do not apply when bar is in lightweight concrete or any other STI ENGTH OT CONCRETE.

3. These development lengths only apply STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION where the General Notes indicate CHEF ENGR BRIDGE DEVEL STATE HIGHWAY ADMINISTRATION DEVELOPMENT LENGTH DIMENSIONS FOR GRADE 60 REINFORCING STEEL IN MIX NO.6 (4500 P.S.I.) CONCRETE EPOXY COATED REINFORCING CASE NO.1 "Reinforcing Steel Design, fs= DATE:2/2/90 24,000 p.s.i. NO. M(6.16)-90-216

bar development length indicated in STATE HIGHWAY ADMINISTRATION OFFICE OF BRIDGE DEVELOPMENT
DEVELOPMENT LENGTH DIMENSIONS FOR
GRADE 60 REINFORCING STEEL
IN MIX NO.6 (4500 P.S.I.) CONCRETE
NON-EPOXY COATED REINFORCING DATE: 2/2/90 Categories A, B, and C; then hook shall be added to all bars not conforming, as per D, E, and F per Std. No. M(6.10)-86-180. NO. M(6,16)-90-216 SHEET\_L\_OF\_3\_

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION

OTHER CONTRACTS FOR THIS STRUCTURE

I. When development length is not specified on the plans, the above dimensions shall be used.

3. These development lengths only apply where the

in lightweight concrete or any other

General Notes indicate "Reinforcing

4.If depth of member does not allow

Steel Design, fs= 24,000 p.s.i."

strength of concrete.

2. These development lengths do not apply when bar is

footings, pier caps, etc.

B- All bars not in Category A spaced less than 6" apart.

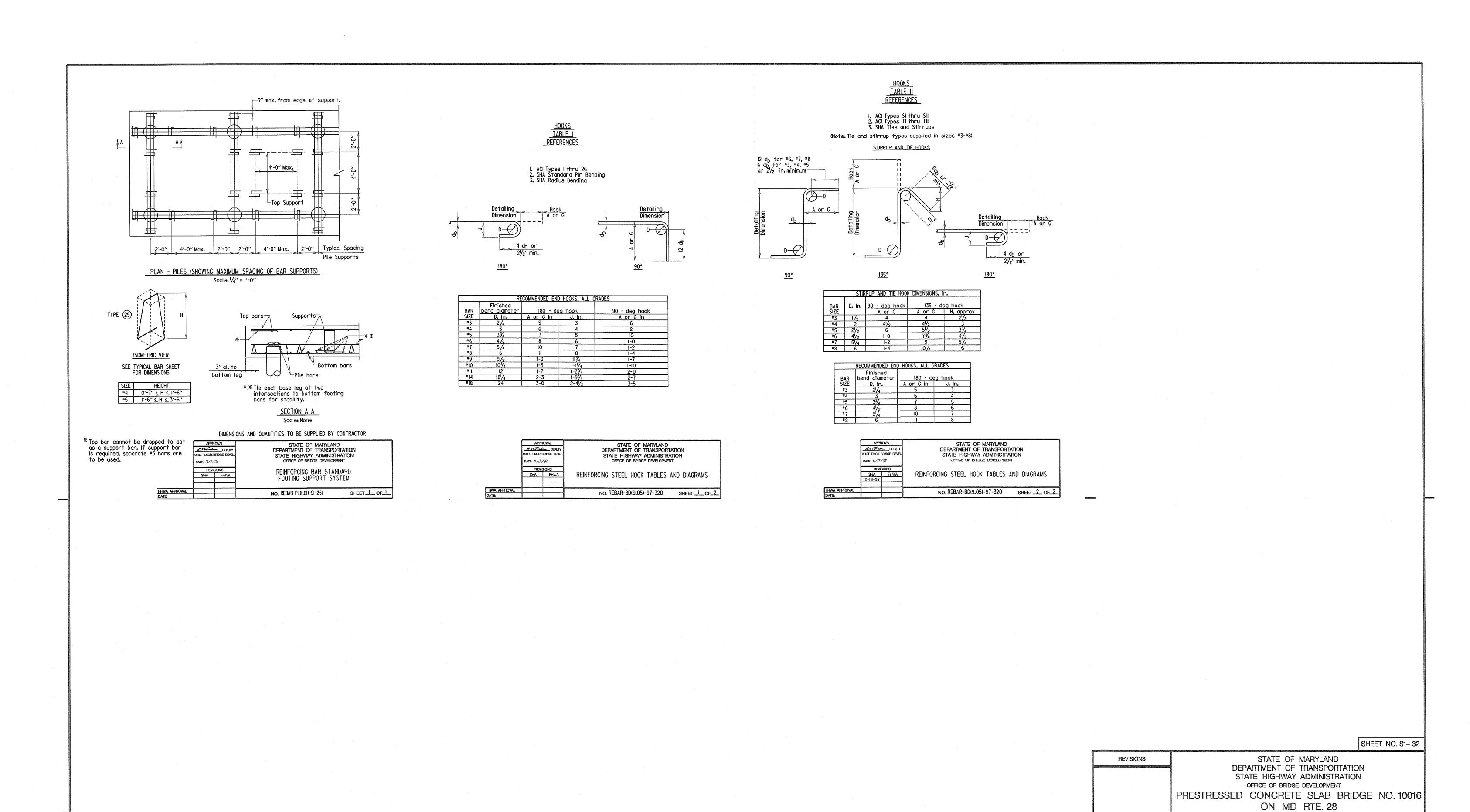
C- All bars not in Category A spaced 6" inches or more apart.

**■ LOCATION CATEGORY:**

STRUCTURE 1001600 INVENTORY NO.

SURVEY BOOK NO. S:\CADD\OBD\1001600\10016s14.dgn PRINT DATE: Wednesday, November 14, 2007 at 1:11:16 PM

INDEXED



SCALE AS SHOWN DATE CONTRACT AX4695180

DESIGNED BY J.W.N.
DRAWN BY J.MOHR
CHECKED BY

STRUCTURE
INVENTORY NO. 1001600

SURVEY
BOOK NO.

DOSIGNED BY J.W.N.
DRAWN BY J.MOHR
CHECKED BY

SHEET NO. 51 OF 53

OTHER CONTRACTS FOR THIS STRUCTURE

