



PLANS OF PROPOSED IMPROVEMENTS ON THE

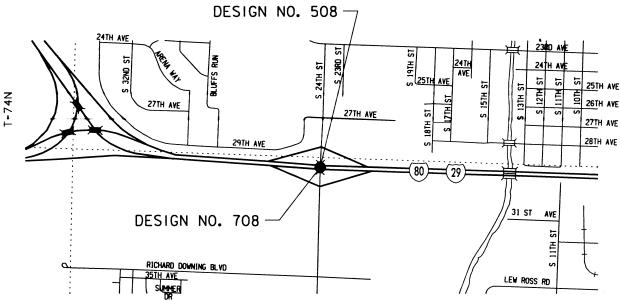
INTERSTATE ROAD SYSTEM POTTAWATTAMIE COUNTY

BRIDGE REPLACEMENT-STEEL GIRDER ON 24TH STREET OVER 1-80

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

VALUE ENGINEERING SAVES. REFER TO THE GENERAL NOTES IN THESE PLANS.

HIGHWAYS FOR LIFE



R-44W LOCATION MAP (PART OF CITY OF COUNCIL BLUFFS)

ALL WORKING DRAWINGS INCLUDING SHOP DRAWINGS AND FALSEWORK DRAWINGS WILL BE REVIEWED BY: HDR ENGINEERING BRIDGE SECTION 8404 INDIAN HILLS DRIVE

PROJECT DIRECTORY NAME: P8921997 / 7802901004

ENGLISH

IOWA DOT * OFFICE OF BRIDGES AND STRUCTURES

POTTAWATTAMIE COUNTY

INDEX OF SEALS

TYPE

STRUCTURAL DESIGN

SOILS DESIGN

NAME

HUSSEIN H. KHALIL

ROBERT L. STANLEY

SHEET NO.

SPS.01

FILE NO. 30169

PROJECT NUMBER IM-080-1(308)2--13-78

PROJECT NUMBER IM-080-1(308)2--13-78 R.O.W. PROJECT NUMBER

TOTAL SHEETS

PROJECT IDENTIFICATION NUMBER 04-78-029-010-02

INDEX OF SHEETS DESCRIPTION TITLE SHEET 2-3 GENERAL NOTES BRIDGE ESTIMATE SHEET 5-63 BRIDGE DESIGN NO. 508 64 WALL ESTIMATE SHEET TERRACE WALLS DESIGN NO. 708 65-81 SOIL PROFILE SHEET SPS.01-SPS.03

ENGLISH STANDARD BRIDGE PLANS										
STANDARD	ISSUED	REVISED								

REVISIONS

TRAFFIC ESTIMATE (24TH STREET)

A.A.D.T. = 15,000 VPD (2004) A.A.D.T. = 24,400 VPD (2030) D.H.V. = 2,930 VPH (2030) 19% TRUCKŚ

STRUCTURAL DESIGN



hereby certify that this engineering document was prepared by me or under my direct personal supervision and that am a duly licensed Professional Engineer under the laws

Hussein H. Khalil

SHEET NUMBER I

My license renewal date is December 31, 2008

Pages or sheets covered by this seal: SHEETS | THRU 81 OF 81

OMAHA, NE 68114

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SPECIFICATIONS:

SUPERSTRUCTURE DESIGN: AASHTO LRFD SERIES OF 2004.

SUBSTRUCTURE DESIGN: AASHTO STANDARD SERIES OF 2002.

CONSTRUCTION:

IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, AND SPECIAL PROVISIONS, INCLUDING DEVELOPMENTAL SPECIFICATIONS FOR CONSTRUCTION PROGRESS SCHEDULE, DEVELOPMENTAL SPECIFICATION FOR HIGH PERFORMANCE CONCRETE FOR STRUCTURES (COUNCIL BLUFFS INTERSTATE SYSTEM), DEVELOPMENTAL SPECIFICATIONS FOR "COLORED SEALER COATING FOR STRUCTURAL CONCRETE", DEVELOPMENTAL SPECIFICATION FOR DISC BEARING ASSEMBLY, SPECIAL PROVISIONS FOR A+B BIDDING, SPECIAL PROVISIONS FOR "PRECAST POST-TENSIONED SLAB PANELS", SPECIAL PROVISIONS FOR "STONE VENEER", AND SUPPLEMENTAL SPECIFICATIONS FOR "CLEANING AND SURFACE PREPARATION OF GALVANIZED SURFACES" SHALL APPLY TO THE CONSTRUCTION WORK ON THIS PROJECT.

DESIGN STRESSES:

SUPERSTRUCTURE:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES

REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5, f'c = 3,500 PSI, EXCEPT AS NOTED.

CONCRETE FOR THE PRECAST DECK PANELS IS TO BE HIGH PERFORMANCE CONCRETE AND IS TO HAVE STRENGTH AS NOTED ON DESIGN SHEET 32. SHEAR STUD POCKETS, TRANSVERSE JOINTS AND GIRDER HAUNCHES (f'c = 6,000 PSI). STRUCTURAL STEEL IN ACCORDANCE WITH SECTION 6, ASTM A709 GRADE 50W AND GRADE HPS70W (AASHTO M270 GRADE 50W AND GRADE HPS70W).

SUBSTRUCTURE:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2002.

REINFORCING STEEL IN ACCORDANCE WITH SECTION 8, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 8, f'c = 3,500 PSI, EXCEPT AS NOTED.

ALL PIER CONCRETE ABOVE THE FOOTING AND ALL ABUTMENT CONCRETE IS TO BE HIGH PERFORMANCE CONCRETE AND IS TO HAVE A MINIMUM f'c = 5,000 PSI.

ALL PILING SHALL BE GRADE 50.

GENERAL NOTES:

THE BRIDGE SUPERSTRUCTURE IS DESIGNED FOR HL93 LOADING.

THE BRIDGE SUBSTRUCTURE IS DESIGNED FOR HS-25 LOADING.

THE DESIGN OF THE SUPERSTRUCTURE IS BY THE LOAD AND RESISTANCE FACTOR DESIGN METHOD. THE DESIGN OF SUBSTRUCTURE IS BY THE LOAD FACTOR DESIGN METHOD, EXCEPT THE PILE BEARING VALUE IS BY THE SERVICE LOAD DESIGN METHOD.

THIS DESIGN IS FOR THE REPLACEMENT OF THE EXISTING 215'x53' CONTINUOUS I-BEAM BRIDGE, DESIGN NO. 6665. PLANS FOR THE EXISTING STRUCTURE WILL BE MADE AVAILABLE TO THE CONTRACTOR. CONTACT THE OFFICE OF CONTRACTS HIGHWAY DIVISION - IOWA D.O.T. - AMES.

THE LUMP SUM BID FOR "REMOVAL OF EXISTING BRIDGE" SHALL INCLUDE REMOVAL OF THE EXISTING 215' x 53' CONTINUOUS PRESTRESSED I-BEAM BRIDGE.

STAGED REMOVALS ARE REQUIRED. REMOVALS SHALL BE IN ACCORDANCE WITH SECTION 2401 OF THE STANDARD SPECIFICATIONS EXCEPT THE EXISTING ABUTMENTS AND PIERS SHALL BE REMOVED TO THE ELEVATION AS SHOWN ON DESIGN SHEET 7.

FAINT LINES ON PLANS INDICATE THE EXISTING STRUCTURE.

THE CITY AND UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE BRIDGE CONTRACTOR OF THE CONSTRUCTION STARTING DATE.

GENERAL NOTES (CON'T.):

THE BRIDGE CONTRACTOR IS ENCOURAGED TO TAKE FULL ADVANTAGE OF SPECIFICATION 1105.15--VALUE ENGINEERING INCENTIVE PROPOSAL. PAMPHLET AND CONCEPTUAL PROPOSAL FORM WILL BE AVAILABLE AT THE PRECONSTRUCTION

THE CONTRACTOR SHALL NOTE THE STANDARD ABUTMENT DETAILS HAVE BEEN MODIFIED TO OFFSET THE ABUTMENT FOOTING FROM THE WINGWALL AND THE ABUTMENT FOOTING FROM THE BACKWALL TO AID IN TYING THE REINFORCING STEEL BETWEEN THE FOOTING TO WINGWALL AND THE FOOTING TO BACKWALL.

THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENTS THE INTENDED METHOD FOR THE DEMOLITION OF THE EXISTING BRIDGE AND THE ERECTION OF THE STEEL GIRDERS AND THE PRECAST PANELS. AT A MINIMUM THE SUBMITTAL SHALL INCLUDE THE START & FINISH TIME ON A DAILY BASIS FOR EACH OF THE WORK ACTIVITIES STATED PREVIOUSLY. ALSO, TYPE OF EQUIPMENT AND METHODS OF REMOVAL AND ERECTION.

CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD WILL REQUIRE THE USE OF A CLASS BR CONCRETE IN ACCORDANCE WITH ARTICLE 2513.03B OF THE STANDARD SPECIFICATION, CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. CLASS D CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS (CAST-IN-PLACE OR SLIPFORMED METHOD). CONCRETE CURING COMPOUNDS CONTAINING PARAFIN SHALL NOT BE USED.

THIS STRUCTURE SHALL BE BUILT WITH WEATHERING STEEL. ALL STRUCTURAL STEEL, EXCEPT AS NOTED, SHALL CONFORM TO ASTM A709 GRADE HPS 50W AND GRADE HPS 70W. PAINTING REQUIREMENTS FOR THIS STRUCTURE SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS 2408.30.

TEMPORARY SHORING (SHEET PILE OR OTHER) SHALL BE REQUIRED AS NECESSARY TO PREVENT THE EARTH UNDER THE TRAFFIC LANE FROM SLOUGHING IN DURING

THE CONTRACTOR SHALL SUBMIT A TEMPORARY SHORING PLAN TO THE ENGINEER FOR APPROVAL. THE TEMPORARY SHORING PLAN SHALL BE DESIGNED AND CERTIFIED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF IOWA, THE CONTRACTOR SHALL SUBMIT 6 COPIES OF PLANS FOR TEMPORARY SHORING. THE ENGINEER WILL BE ALLOWED 30 CALENDAR DAYS TO REVIEW THE TEMPORARY SHORING PLAN. THE CONTRACTOR SHALL NOT PROCEED WITH INSTALLATION OF THE TEMPORARY SHORING WITHOUT NOTICE TO PROCEED FROM THE ENGINEER.

THE TEMPORARY SHORING SUBMITTAL SHALL INCLUDE:

- DESIGN CALCULATIONS (INCLUDING A GLOBAL STABILITY ANALYSIS)
- SOIL PROPERTIES
- . SHORING MATERIAL PROPERTIES
- SHORING PLAN LAYOUT (SHOWING LOCATION OF TRAFFIC)
- · SHORING DETAILS

NOTE:

POLLUTION PREVENTION PLAN

SHOWN IN IM-080-I(334)2--I3-78.

TEMPORARY SHORING SHALL BE PAID FOR AS A LUMP SUM INCLUDING ALL COST FOR DESIGNING, FURNISHING, INSTALLING AND REMOVAL. ALL MATERIAL USED FOR SHORING SHALL REMAIN THE PROPERTY OF THE CONTRACTOR, SHORING IS TO BE REMOVED ONLY AFTER BACKFILLING HAS BEEN COMPLETED. IN ADDITION TO THE REQUIREMENTS NOTED ABOVE, ARTICLE 1107.07 OF THE STANDARD SPECIFICATIONS STILL APPLIES.

CONCRETE SEALER IS TO BE APPLIED TO THE EXPOSED BRIDGE SEAT SURFACE AT THE ABUTMENTS.

SUBSTRUCTURE CONCRETE SHALL BE PROTECTED FROM STAINING BY A WRAPPING OF POLYETHYLENE OR SIMILAR MATERIALS WHICH SHALL BE LEFT IN PLACE AND KEPT IN A SERVICEABLE CONDITION UNTIL AFTER THE DECK HAS BEEN PLACED. IF SUBSTRUCTURE CONCRETE IS STAINED, THE STAINS SHALL BE REMOVED BY METHODS APPROVED BY THE ENGINEER. ALL COSTS ASSOCIATED WITH THE PROTECTION AND ANY REQUIRED CLEANING OF THE SUBSTRUCTURE CONCRETE SHALL BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL STEEL".

LONGITUDINAL GROOVING WILL NOT BE AS PART OF THIS PROJECT, BUT WILL BE INCLUDED IN ANOTHER PROJECT ASSOCIATED WITH THIS CONTRACT.

TEMPORARY CONCRETE BARRIERS REQUIRED FOR PHASING ARE NOT PAID FOR AS A PART OF THIS PROJECT, BUT WILL BE PAID FOR IN ANOTHER PROJECT ASSOCIATED WITH THIS CONTRACT.

THE BRIDGE FLOOR SURFACING CONCRETE SHALL NOT BE PLACED AFTER OCTOBER 31 AND PRIOR TO APRIL I, WITHOUT WRITTEN APPROVAL OF THE ENGINEER. ALL OTHER LIMITATIONS OF OPERATIONS IN ARTICLE 2413.10 SHALL APPLY.

GENERAL NOTES (CON'T.):

2325 PPM.

A SCRAPE SAMPLE WAS TAKEN FROM AREAS OF THE EXISTING BRIDGE TO GET AN INDICATION OF THE EXISTENCE OF AND LEVEL OF TOTAL CHROMIUM AND TOTAL LEAD. SAMPLES WERE TAKEN FROM THE ABUTMENT BEARINGS AND EXPANSION DEVICES. ANALYSIS OF TOTAL LEAD ON ABUTMENT BEARING WAS 4690 PARTS PER MILLION (PPM). ANALYSIS OF TOTAL CHROMIUM ON ABUTMENT BEARING WAS 2220 PPM. ANALYSIS OF TOTAL LEAD ON EXPANSION DEVICE SAMPLE WAS 3575 PARTS PER MILLION (PPM). ANALYSIS OF TOTAL CHROMIUM ON EXPANSION DEVICE SAMPLE WAS

THESE ANALYSES SHOW THE EXISTENCE OF THESE TWO TOXIC CONSTITUENTS. LEVELS INDICATED BY THESE TESTS COULD CREATE CONDITIONS ABOVE REGULATORY LIMITS FOR HEALTH AND SAFETY REQUIREMENTS. NO OTHER CONSTITUENTS WERE ANALYZED. THE BIDDER SHOULD NOT RELY ON THE DEPARTMENT'S TESTING AND ANALYSIS FOR ANY PURPOSE OTHER THAN AS AN INDICATION OF THE EXISTENCE OF THESE TWO TOXIC CONSTITUENTS.

SEE INDIVIDUAL DESIGN SHEETS FOR SPECIFIC NOTES AND DETAILS DESCRIBING THE FEATURES WHICH INCORPORATE TEXTURED CONCRETE. WORK PERFORMED TO CREATE TEXTURED CONCRETE SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR FORMWORK AND THE FOLLOWING:

FORM THE TEXTURED CONCRETE SURFACES USING A FORM LINER SYSTEM MADE OF HIGH-STRENGTH URETHANE ELASTOMER OR FLEXIBLE FOAM MATERIALS CAPABLE OF WITHSTANDING ANTICIPATED CONCRETE POUR PRESSURES WITHOUT LEAKAGE OR CAUSING PHYSICAL DEFECTS. FORM LINERS SHALL EASILY ATTACH TO FORMS AND BE REMOVABLE WITHOUT CAUSING CONCRETE SURFACE DAMAGE. IF RECOMMENDED BY THE FORM LINER MANUFACTURER, USE STRUCTURAL BACKERS TO PREVENT DEFORMATION OF THE LINER DURING LOADING OF THE FORMS. THE LINERS SHALL BE DESIGNED TO FORM SURFACES CONFORMING TO THE DESIGN INTENT INCLUDING THE SHAPE, LINES AND DIMENSIONS SHOWN IN THE PLANS AND TO AVOID VISIBLE PATTERN REPEATS. MATCH PATTERN FEATURES AT FORM LINER JOINTS TO MINIMIZE PATTERN REPEATS AND MAKE THE FORMED CONCRETE SURFACE APPEAR UNIFORM AND CONTINUOUS WITHOUT VISIBLE SEAMS AND FORM MARKS, WHEN JOINTS ARE UNAVOIDABLE, MAKE JOINTS ALONG MAIN FEATURES OF THE PATTERN IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

FORM LINER EDGES FOLLOWING CURVES ARE TO BE CUT CLEANLY AND PARALLEL TO THE CURVE. USE ADEQUATE BLOCKING, SEALING AND OTHER MEANS IN ORDER TO MAINTAIN THE APPROPRIATE DEPTH AND CHARACTER OF TEXTURE AT CUT EDGES OF LINERS AND TO PREVENT MORTAR LEAKAGE.

LAY OUT INDIVIDUAL SIMULATED STONE LINERS WITHIN FORMS SO THAT NO VERTICAL MORTAR JOINTS ARE ALIGNED ON ADJACENT COURSES. DO NOT MIX FORM LINERS FROM DIFFERENT MANUFACTURERS WHEN FORMING THE TEXTURED CONCRETE ON

DURING LOADING OF FORMS WITH CONCRETE, TAKE EXTRA CARE TO ADEQUATELY VIBRATE CONCRETE IN ORDER TO MAINTAIN ALL INTENDED FEATURES OF THE FORM LINER IN THE FINAL SURFACE AND TO PREVENT VOIDS. FOLLOWING REMOVAL OF FORMS, FINISH MINOR DEFECTS TO BLEND WITH THE BALANCE OF THE SURFACE TEXTURE. THE COMPLETED SURFACE SHALL BE FREE OF BLEMISHES, SURFACE VOIDS AND CONSPICUOUS FORM MARKS TO THE SATISFACTION OF THE ENGINEER. THE CONTRACTOR SHALL CORRECT, AT HIS OWN COST, ANY SURFACE DEFECTS.

VERIFY THAT RELEASE AGENTS USED ARE COMPATIBLE WITH FORM LINER MATERIAL, AND ARE NON-STAINING. APPLY RELEASE AGENT IN ACCORDANCE WITH THE FORM LINER MANUFACTURER'S RECOMMENDATIONS. RELEASE AGENTS MUST ALSO BE COMPATIBLE WITH THE PROPOSED CONCRETE STAINS TO BE USED TO COLOR THE CONCRETE.

IF USED, FORM TIES SHALL BE MADE OF NON-CORROSIVE MATERIALS WHEN THE PORTION PERMANENTLY EMBEDDED IN THE CONCRETE IS LESS THAN 12 INCHES FROM THE FINISHED SURFACE, POSITION FORM TIES AND ACCESSORIES IN STONE PATTERN MORTAR JOINTS AND AT HIGH POINTS OF FINISHED WALL.

STRIP FORMWORK IN ACCORDANCE WITH LINER MANUFACTURER'S RECOMMENDATIONS AFTER THE CONCRETE HAS SUFFICIENT STRENGTH TO AVOID SURFACE DAMAGE. CLEAN AND REPAIR FORM LINER SURFACES PRIOR TO REUSE. DO NOT USE SPLIT, FRAYED, DELAMINATED OR OTHERWISE DAMAGED FORM LINERS.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

GENERAL NOTES

JUNE, 2007

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80) POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 1 OF 62 FILE NO. 30169 DESIGN NO. 508

NOTE:

ROADWAY QUANTIES SHOWN

IN IM-080-1(334)2--13-78.

POTTAWATTAMIE COUNTY

TRAFFIC CONTROL PLAN:

REFER TO THE TRAFFIC CONTROL

PLAN SHOWN IN IM-080-1(334)2--13-78

PROJECT NUMBER IM-080-I(308)2--I3-78

DESIGN TEAM RRP/HHK/ACB

HDR Engineering. Inc.

HR

GENERAL NOTES (CON'T.):

CONSTRUCT A 4-FOOT HIGH, BY 10-INCH WIDE (MIN.), BY 8-FOOT LONG MOCKUP PANEL IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND THESE PLANS. CAST THE MOCKUP PANEL ON SITE, USING THE SAME FORMING METHODS, PROCEDURES, FORM LINER, AND CONCRETE MIXTURE PROPOSED FOR THE PRODUCTION WORK, TEXTURED FACE SHALL BE VERTICAL DURING THE CASTING PROCESS. A SINGLE MAT OF NO. 5 REINFORCING BARS IN TWO DIRECTIONS SHALL BE SET 2 INCHES CLEAR TO THE BOTTOM OF THE TEXTURED FACE. IF THE MOCKUP PANEL IS REJECTED, CONSTRUCT A NEW MOCKUP PANEL AS DIRECTED BY THE ENGINEER. BEGIN CONCRETE TEXTURE PRODUCTION WORK ONLY AFTER THE MOCKUP HAS BEEN APPROVED BY THE ENGINEER.

AFTER CURING FOR A MINIMUM OF 28 DAYS, THE MOCKUP PANEL WILL ALSO BE USED TO DEMONSTRATE THE COLORED SEALER COATING APPLICATION. SEE DETAILS AND NOTES ON DESIGN SHEET 62 FOR FURTHER INFORMATION REGARDING COLORED SEALER.

ALL COSTS ASSOCIATED WITH CONCRETE TEXTURING AND FORM LINERS INCLUDING CONSTRUCTING AND REMOVING THE MOCKUP PANEL ARE TO BE INCLUDED IN THE BID ITEM. "STRUCTURAL CONCRETE (HIGH PERFORMANCE)".

THESE BRIDGE PLANS LABEL ALL REINFORCING STEEL WITH ENGLISH NOTATION (5a) is \$ inch diameter bar). ENGLISH REINFORCING STEEL RECEIVED IN THE FIELD MAY DISPLAY THE FOLLOWING "BAR DESIGNATION". THE "BAR DESIGNATION" IS THE STAMPED IMPRESSION OF THE REINFORCING BARS, AND IS EQUIVALENT TO THE BAR DIAMETER IN MILLIMETERS.

ENGLISH SIZE	BAR DESIGNATION
3	10
4	13
5	16
6	19
7	22
8	25
9	29
10	32
H	36

THE CONCRETE USED TO FILL THE SHEAR STUD POCKETS AND GIRDER HAUNCHES SHALL BE A CLASS O-4WR, WITH THE FOLLOWING REQUIREMENTS:

SIZE OF AGGREGATE SHALL BE SUCH THAT A MAXIMUM OF 5% IS RETAINED ON THE 3" SIEVE AND 100% PASSING THE 1" SIEVE.
MAXIMUM WATER CEMENT RATIO OF 0.38.

THE SLUMP SHALL BE A MAXIMUM OF 3 INCHES AT THE PLANT AND A MID RANGE OR HIGH RANGE WATER REDUCER SHALL BE ADDED AT THE SITE. THE MAXIMUM SLUMP FOR A MID RANGE WATER REDUCER SHALL BE 6 INCHES, AND THE MAXIMUM SLUMP FOR A HIGH RANGE WATER REDUCER SHALL BE 8 INCHES. CONCRETE TEMPERATURE AT PLACEMENT SHALL BE A MINIMUM OF 70° F.

IN ORDER TO OBTAIN THE PROPER AMOUNT OF THE WATER REDUCER ALONG WITH ITS USEFUL TIME, THE CONTRACTOR SHALL PRODUCE A TRIAL BATCH THAT SHALL BE REPRESENTATIVE OF THE PRODUCTION CONCRETE.

THE TRIAL BATCH CONCRETE SHALL BE MADE AT LEAST 14 CALENDAR DAYS PRIOR TO PLACEMENT.

THE DISTRICT MATERIALS ENGINEER SHALL BE NOTIFIED AT LEAST 7 CALENDAR DAYS PRIOR TO BATCHING.

THE DISTRICT MATERIALS ENGINEER SHALL BE WITNESS TO THE MIXING OF THE TRIAL BATCH.

TRIAL BATCH MATERIALS, PROPORTIONS, AND TEST RESULTS SHALL BE REPORTED TO THE DISTRICT MATERIALS ENGINEER FOR APPROVAL.

THE MAXIMUM EVAPORATION RATE SHALL BE O.I POUNDS PER SQUARE FOOT PER HOUR. WET BURLAP CURING SHALL BE PLACED IMMEDIATELY AFTER FINISHING AND COVERED WITH PLASTIC. CURING SHALL REMAIN IN PLACE AND KEPT WET FOR A MINIMUM OF 7 DAYS. THE CURING MAY BE REMOVED PRIOR TO 7 DAYS IF THE SPECIFIED STRENGTH HAS BEEN REACHED AND THE CURING IS REMOVED JUST PRIOR TO PLACEMENT OF THE BRIDGE FLOOR SURFACING, TEMPERATURES WILL BE MONITORED BY THE DISTRICT MATERIALS ENGINEER AND INSULATING BLANKETS MAY BE REQUIRED TO MAINTAIN TEMPERATURE.

OTHER MIXES MAY BE CONSIDERED PROVIDED THEY HAVE BEEN REVIEWED AND APPROVED BY THE DISTRICT MATERIALS ENGINEER.

HR

HDR Engineering, Inc. DESIGN TEAM RRP/HHK/ACB NOTE: ROADWAY QUANTIES SHOWN IN IM-080-1(334)2--13-78. NOTE: POLLUTION PREVENTION PLAN SHOWN IN IM-080-I(334)2--I3-78.

TRAFFIC CONTROL PLAN: REFER TO THE TRAFFIC CONTROL PLAN SHOWN IN IM-080-1(334)2--13-78

GENERAL NOTES (CON'T.):

IT SHALL BE THE BRIDGE CONTRACTOR'S RESPONSIBILITY TO PROVIDE SITES FOR EXCESS EXCAVATED MATERIAL. NO PAYMENT FOR OVERHAUL WILL BE ALLOWED FOR MATERIAL HAULED TO THESE SITES.

THE APPROACH FILLS AS SHOWN ARE NOT A PART OF THIS CONTRACT, BUT ARE TO BE IN PLACE BEFORE ABUTMENT PILES ARE DRIVEN. THE BRIDGE CONTRACTOR IS TO LEVEL OFF AND SHAPE THE BERMS TO THE ELEVATIONS AND DIMENSIONS SHOWN, DRESSING OF SLOPES OUTSIDE THE BRIDGE AREA NOT DISTURBED BY THE BRIDGE CONTRACTOR SHALL BE PAID FOR AS EXTRA WORK.

	DESIGN HISTORY AT THIS SITE								
DES. NO.	DES. NO. TYPE OF WORK								
6665	ORIGINAL DESIGN								
492	BEAM REPLACEMENT								
696	REPAIR + OVERLAY								
508	BRIDGE REPLACEMENT								
708	TERRACE WALLS								

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

GENERAL NOTES

JUNE, 2007

STA. 40176+95.25 (24TH STREE STA. 7476+95.25 (FUTURE 1-80) POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 2 OF 62 FILE NO. 30169 DESIGN NO. 508

POTTAWATTAMIE COUNTY PROJECT NUMBER IM-080-I(308)2--I3-78

	ESTIMATED BRIDGE QUANTITIES									
ITEM NO.	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUANTITY					
1	2401-6745625	RMVL OF EXIST BRIDGE	LS	1.0						
2	2402-2720000	EXCAVATION, CL 20	CY	1031.0						
3	2403-0100010	STRUCT CONC (BRIDGE)	CY	96.2						
4	2403-7000210	HIGH PERFORMANCE STRUC CONC	CY	1086.4						
5	2403-7000220	TRIAL BATCH HIGH PERFORMANCE STRUC CONC	LS	1.0						
6	2403-7302000	COLORED SEALER COAT - STRUCT CONC	SY	1711.0						
7	2404-7775005	REINFORC STEEL, EPOXY COATED	LB	164639						
8	2408-7800000	STRUCTURAL STEEL	LB	1485189						
9	2413-0698130	BRIDGE FLOOR SURFACE (CLASS HPC-0 PCC)	SY	3072						
10	2413-1200000	STEEL EXTRUSION JOINT W/NEOPRENE	LF	211.5						
11	2414-6425410	CONC BARRIER, REINFORCED, SEPARATION	LF	745.0						
12	2414-6425420	CONC BARRIER, PARAPET	LF	708.5						
13	2414-6445100	STRUCTURAL STEEL PEDESTRIAN HAND RAIL	LF	372.3						
14	2414-6772020	STEEL FENCE, WELDED WIRE MESH	LF	707.3						
15	2501-0201057	PILE, STEEL, HP 10×57	LF	13250.0						
16	2501-0201489	PILE, STEEL, HP 14×89	LF	8100.0						
17	2501-8400172	TEMP SHORING	LS	1.0						
18	2526-8285000	CONSTRUCTION SURVEY	LS	1.0						
19	2533-4980005	MOBILIZATION	LS	1.0						
20	2599-9999005	PRECAST POST-TENSIONED SLAB PANELS	EA	70.0						
21	2599-9999005	DISC BEARINGS	EA	12.0						
22	2599-9999014	STONE VENEER	SF	2428						
23	2601-2638610	CONC SLOPE PROTECTION	SY	572.0						

ITEM NO.	ESTIMATE REFERENCE INFORMATION ALL PIER FOOTING CONCRETE SHALL BE CLASS "C".
4	ALL CAST-IN-PLACE SLAB CONCRETE, INCLUDING CLOSURE POUR AND END SECTIONS, SIDEWALK, PIER CONCRETE ABOVE FOOTING AND CONCRETE FOR ABUTMENTS SHALL BE STRUCTURAL CONCRETE (HIGH PERFORMANCE). INCLUDES 722 FT. OF 3" DIA. RIGID STEEL CONDUIT IN SIDEWALK, 227 FT. OF 2" DIA. RIGID STEEL CONDUIT AND 629 FT. OF 1" DIA. RIGID STEEL CONDUIT. INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING AND PLACING CONDUITS IN THE ABUTMENTS, SIDEWALKS AND UNDER SLAB FOR BRIDGE LIGHTING. INCLUDES FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), GRANULAR BACKFILL, POROUS BACKFILL, AND SUBDRAIN OUTLET AT ABUTMENTS. INCLUDES FURNISHING AND PLACING CONCRETE SEALER. INCLUDES ALL PREFORMED EXPANSION JOINT FILLER REQUIRED. INCLUDES 36.8 CY OF INTEGRALLY COLORED CONCRETE FOR RAISED MEDIAN. INCLUDES ANCHOR BOLTS AND PLATES AT LIGHT POLE BASES. INCLUDES ALL COSTS ASSOCIATED WITH THE PIER FORM LINER.
7	INCLUDES COST OF 149 LBS. OF STAINLESS STEEL REINFORCING IN THE ABUTMENT PAVING NOTCHES. FOR DETAILS, SEE DESIGN SHEET 15.
8	INCLUDES 662,991 LBS. OF ASTM A709 GRADE HPS 70W (AASHTO M270 GRADE HPS 70W) WEATHERING STEEL. INCLUDES COST OF ABUTMENT BEARING MATERIALS.
9	PORTLAND CEMENT CONCRETE OVERLAY. INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING, PLACING AND REMOVING PLUGS OVER MECHANICAL CONNECTORS FOR MEDIAN CONSTRUCTION.
10	INCLUDES ALL NECESSARY HARDWARE AND ACCESSORIES INCLUDING THE ANCHORAGE SYSTEM, TEMPORARY ERECTION MATERIAL, AND THE COVER PLATES WITH THEIR ANCHORAGE SYSTEMS.
11, 12	IF PLACEMENT OF CONCRETE IS DONE BY THE SLIPFORMING METHOD, CLASS BR CONCRETE IS REQUIRED. CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. PRICE BID FOR THIS ITEM SHALL INCLUDE THE COST OF CAST-IN-PLACE FORMS IF REQUIRED FOR PLACEMENT OF THE CONCRETE. INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING AND PLACING CONDUITS IN BARRIERS. INCLUDES 1068 FT. OF 2" DIA. RIGID STEEL CONDUIT, 15 FT. OF 1" DIA. RIGID STEEL CONDUIT AND 133 FT. OF 3" DIA. RIGID STEEL CONDUIT IN BARRIERS. INCLUDES MATERIAL AND LABOR ASSOCIATED WITH PROVIDING AND INSTALLING RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS.
14	THE QUANTITY FOR "STEEL FENCE, WELDED WIRE MESH" WILL BE MEASURED IN LINEAR FEET END TO END OF FENCE AS SHOWN IN THE CONTRACT DOCUMENTS. FOR THE NUMBER OF LINEAR FEET OF FENCE CONSTRUCTED THE CONTRACTOR WILL BE PAID THE CONTRACT UNIT PRICE PER LINEAR FOOT. PAYMENT FOR THE FENCE QUANTITY SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIALS, ANCHORAGES, PAINTING, EQUIPMENT AND LABOR NECESSARY TO CONSTRUCT THE FENCE AS SHOWN IN THE CONTRACT DOCUMENTS.
15, 16	PILING SHALL BE GRADE 50.
17	INCLUDES THE COST TO DESIGN, FURNISH AND INSTALL SHEETING & SHORING TYPE "A" & TYPE "C" AS REQUIRED TO FACILITATE CONSTRUCTION OF THE ABUTMENTS AND THE PIER WITHOUT LOSS OF SUPPORT OF THE ADJACENT AT-GRADE PAVEMENT. TEMPORARY SHEET PILING & SHORING SHALL BE MEASURED AND PAID FOR AT THE LUMP SUM CONTRACT PRICE. TYPE "B" SHEETING & SHORING IS PAID FOR IN ANOTHER PROJECT ASSOCIATED WITH THIS CONTRACT.
20	THIS ITEM INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING AND PLACING THE PRECAST SLAB PANELS. INCLUDING GROUT FOR TRANSVERSE JOINTS (19.8 CY), ¾" DIA. TRANSVERSE BACKING ROD (3559 LF), LEVELING DEVICES, CONCRETE FOR GIRDER HAUNCHES & SHEAR STUD POCKETS (125.8 CY), HIGH PERFORMANCE STRUCTURAL CONCRETE FOR PANELS (812.0 CY) MILD REINFORCING (124,021 LBS), 0.6" DIA. 270-LL POST TENSIONING STRANDS (78,400 LF), P.T. PRESSURE GROUT INSIDE OF DUCTS AND REQUIRED POST TENSION END ANCHORAGES, ALL EMBEDED ITEMS SUCH AS DUCTS & DUCT SPLICES, LIFTING AND LEVELING DEVICES. TRIAL BATCH FOR CLASS 0-4 WR CONCRETE. INCLUDES ALL COSTS ASSOCIATED WITH DECK FORMING AS SHOWN ON DETAIL "C" ON DESIGN SHEET 7.

INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING AND APPLYING ANTI-GRAFFITI COATING (270.0 SY).

METHOD OF MEASUREMENT AND BASIS OF PAYMENT SHALL BE PER SQUARE YARD AS MEASURED IN THE FIELD.

22

23

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH 24TH STREET OVER 1-80
178'-6 & 175'-0 SPANS

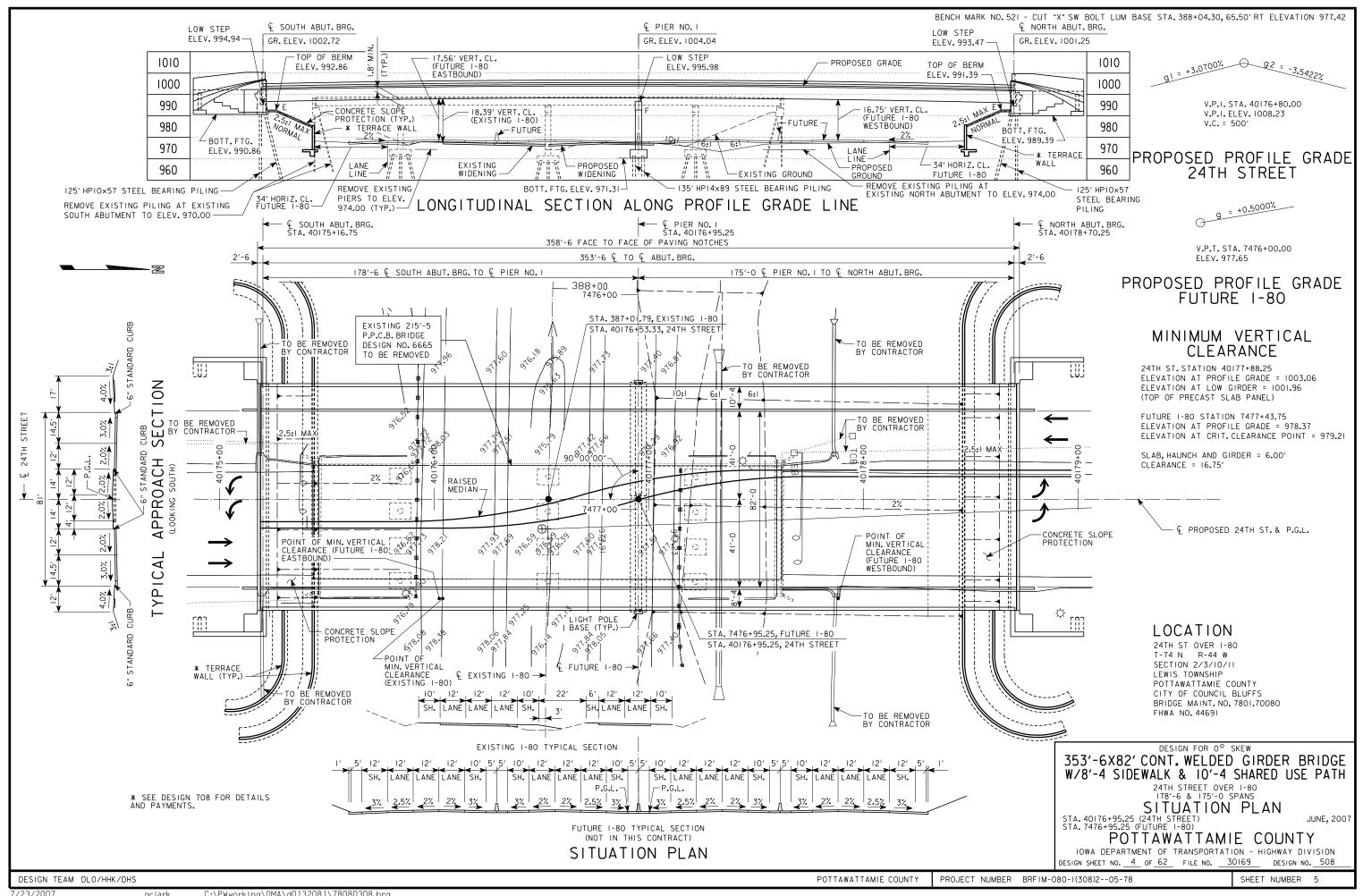
ESTIMATED QUANTITIES
STA. 40176+95.25 (24TH STREET)
STA. 7476+95.25 (FUTURE 1-80)

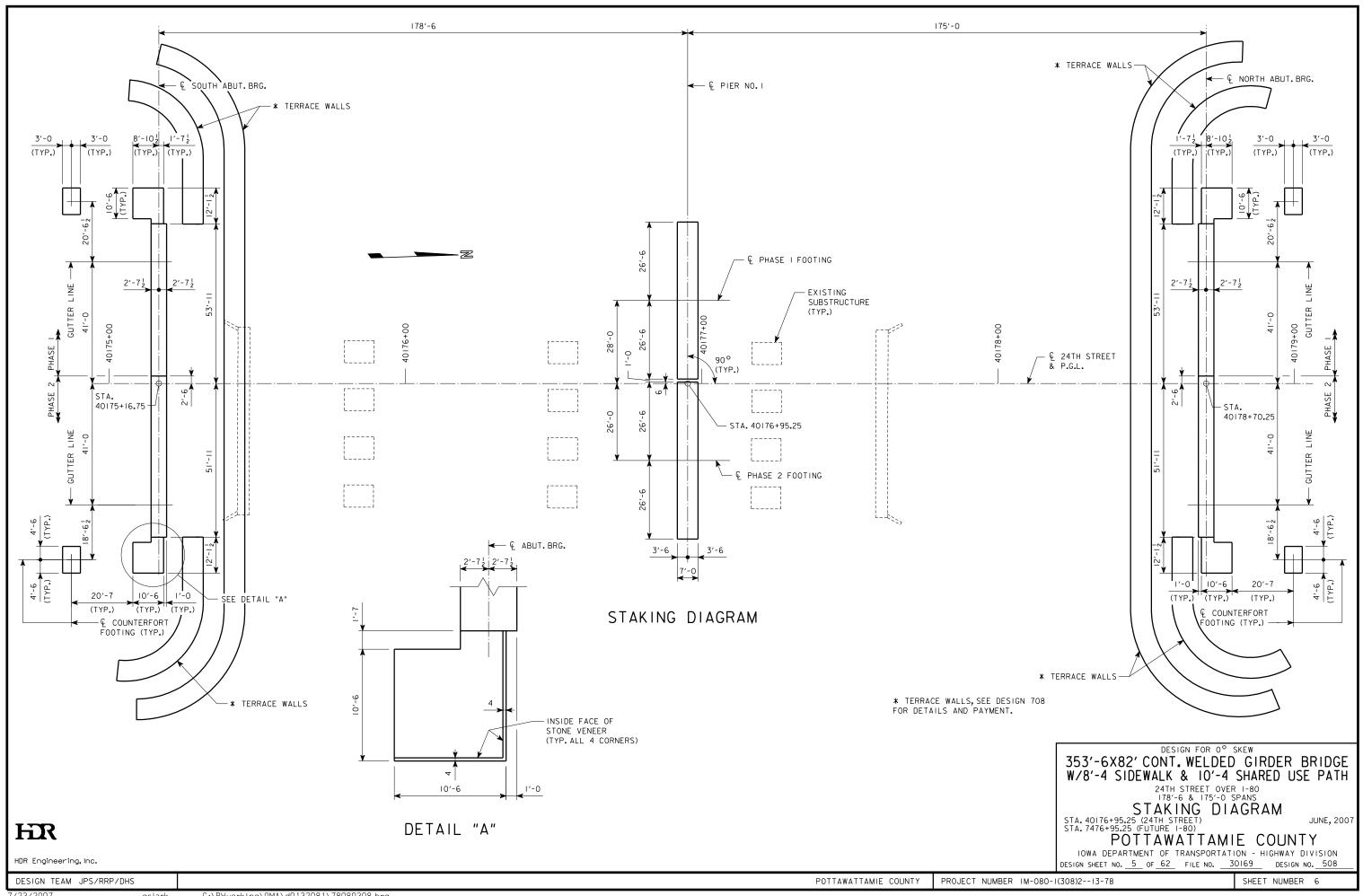
POTTAWATTAMIE COUNTY

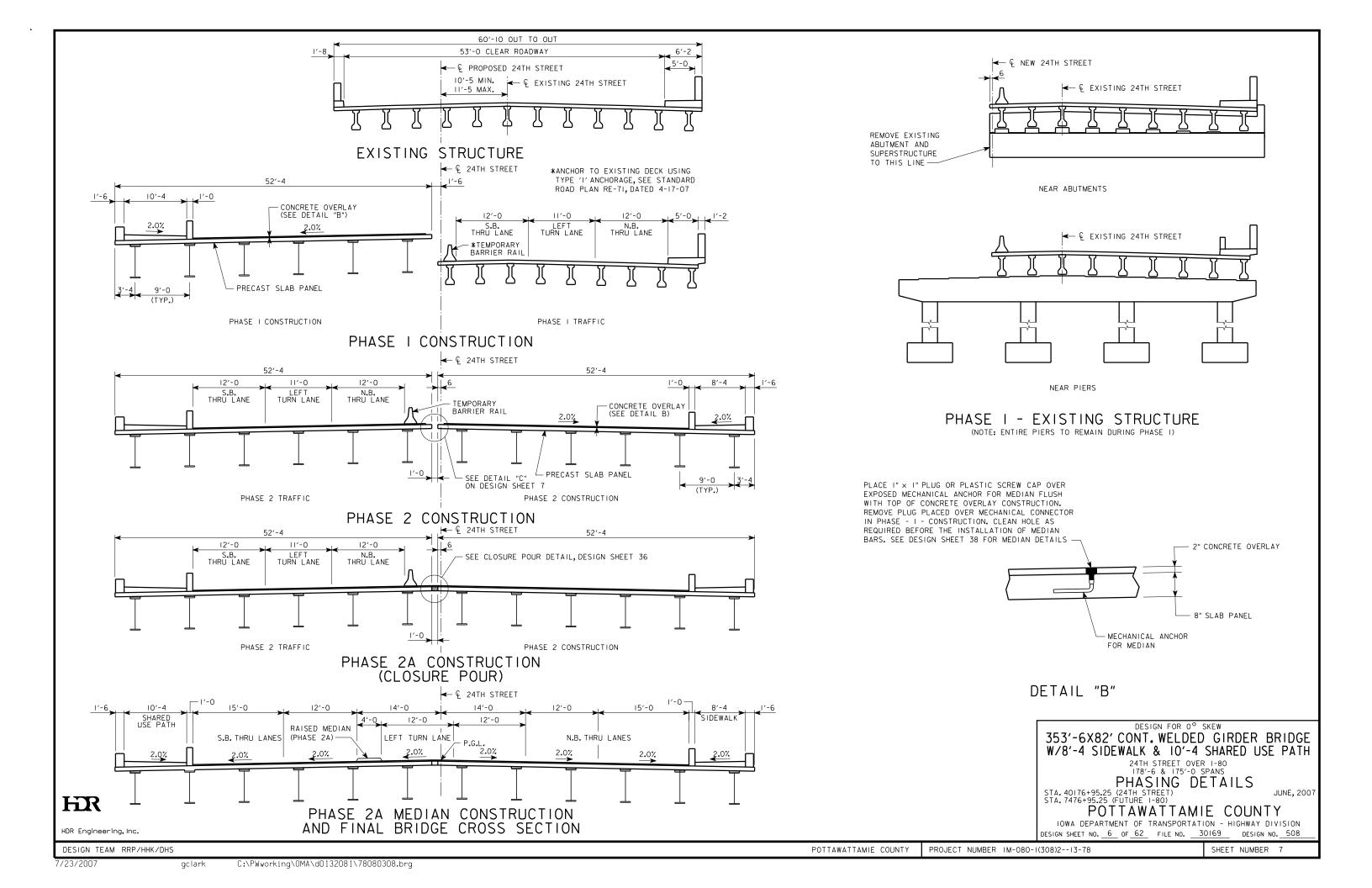
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 3 OF 62 FILE NO. 30169 DESIGN NO. 508

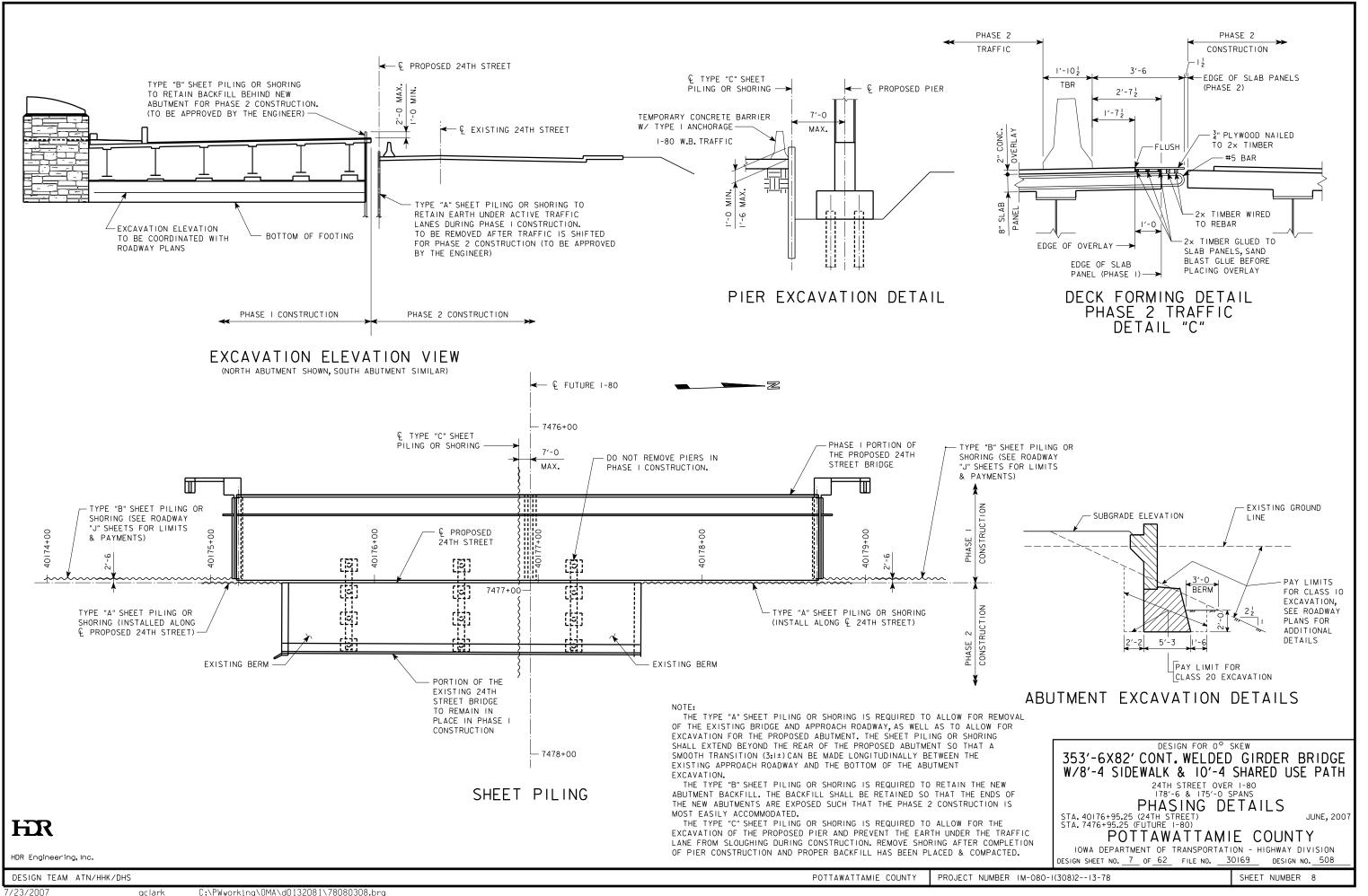
HDR Engineering, Inc.

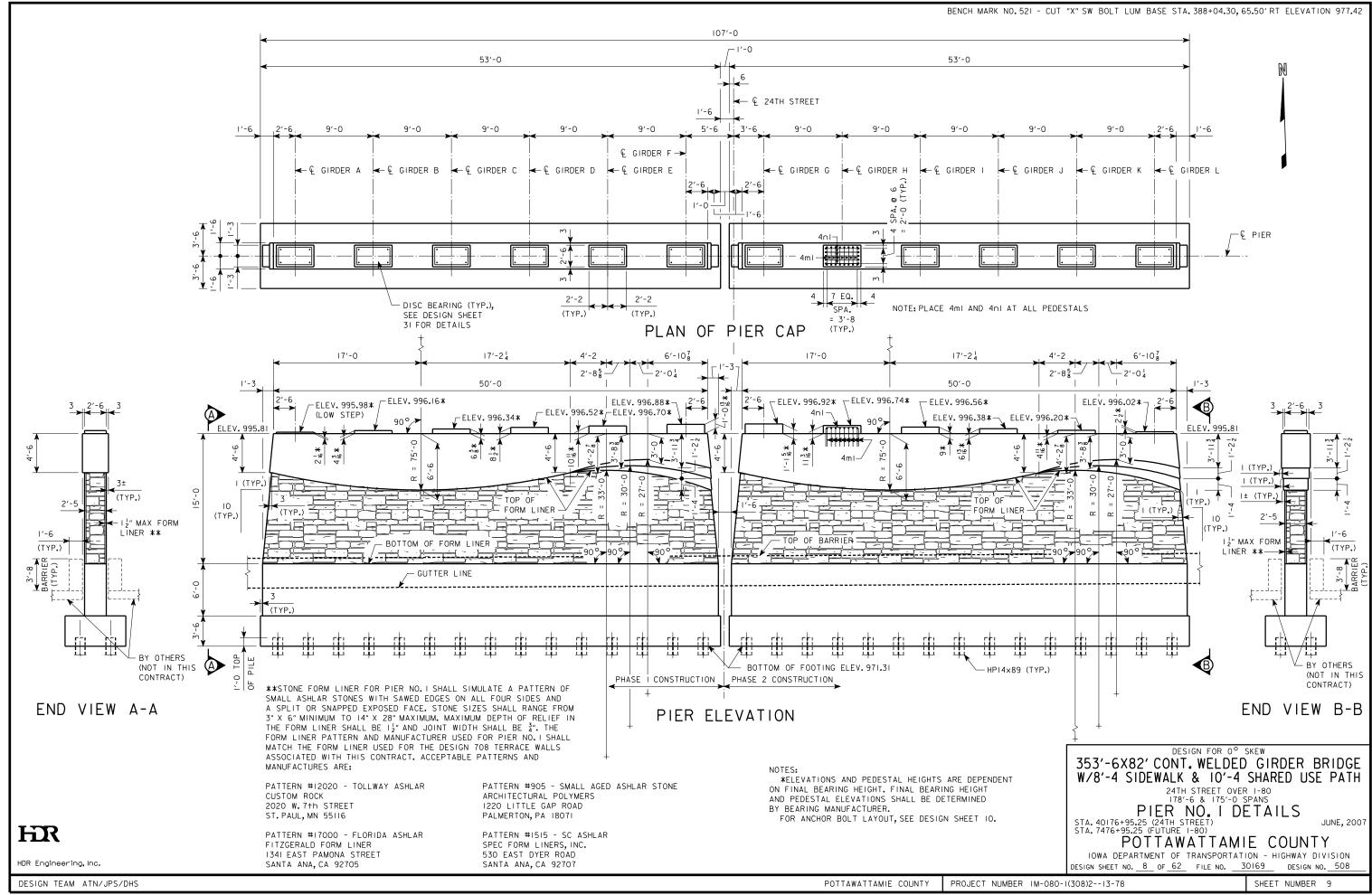
DESIGN TEAM RRP/JPS/ACB

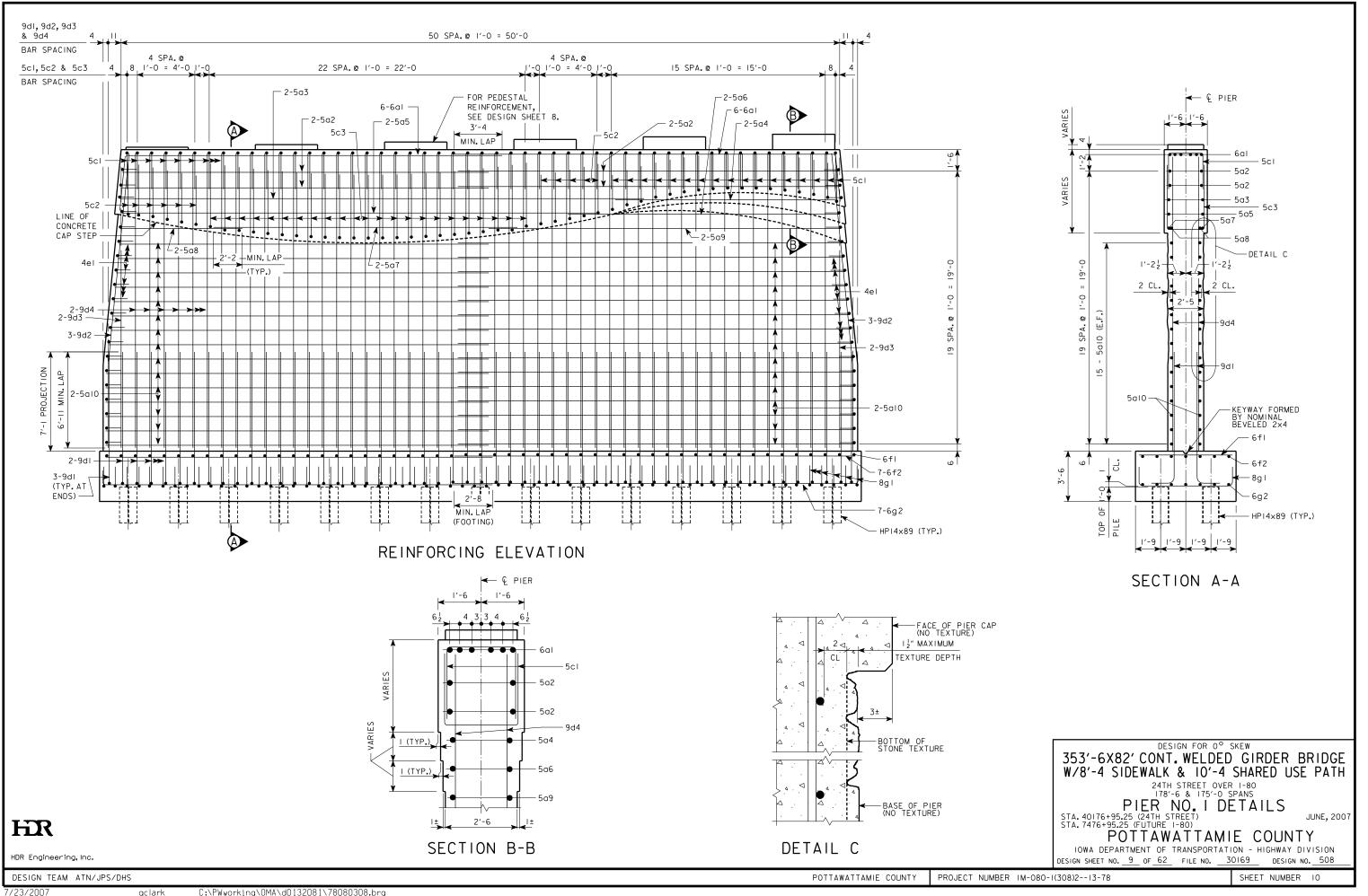


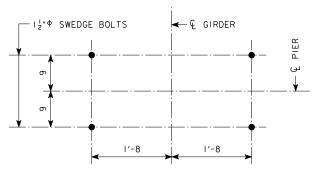












ANCHOR BOLT LAYOUT

ANCHOR BOLT LAYOUT IS DEPENDENT ON FINAL BEARING DESIGN. FINAL ANCHOR BOLT LOCATIONS SHALL BE DETERMINED BY BEARING MANUFACTURER.

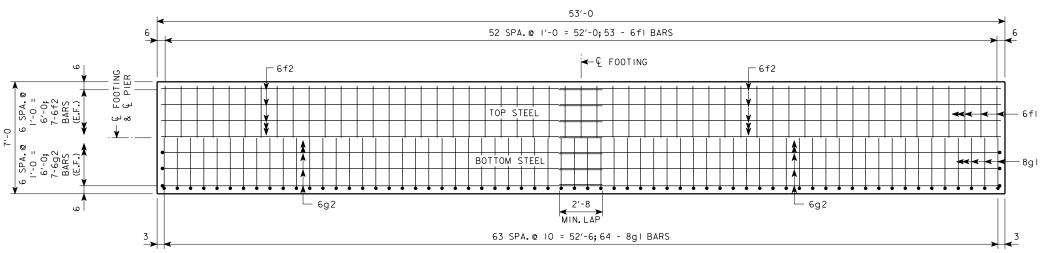
CONCRETE PLACEMENT QUANTITIES (EACH PHASE)

LOCATION		QUANTITY
WALL & STEPS (HIGH PERFORMANCE CONCRETE) *		106.8
FOOTING		48.1
	TOTAL C.Y.	154.9

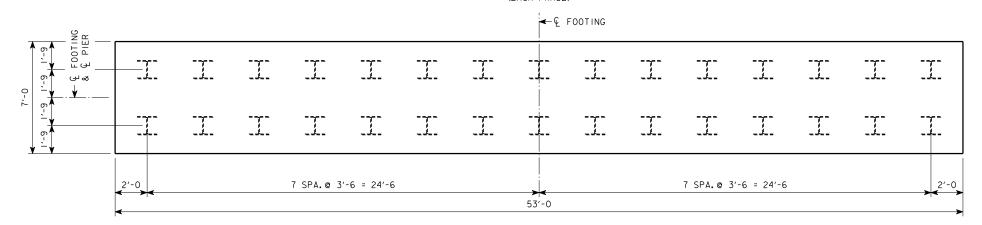
ESTIMATED QUANTITIES											
ITEM	UNIT	PHASE I PIER	PHASE 2 PIER	QUANTITY							
STRUCTURAL CONCRETE (HIGH PERFORMANCE)	C.Y.	106.8	106.8	213.6							
STRUCTURAL CONCRETE (BRIDGE)	C.Y.	48.1	48.1	96.2							
REINFORCING STEEL - EPOXY COATED	LB.	19,011	19,011	38,022							
CLASS 20 EXCAVATION	C.Y.	139	138	277							
HPI4×89 STEEL BEARING PILING	L.F.	30 @ 135	30 @ 135	8100							

* QUANTITY IS CALCULATED ASSUMING THE WALL THICKNESS IS 2'-6 AND IGNORES THE DEDUCTION OF CONCRETE VOLUME DUE TO THE FORM LINER.

POTTAWATTAMIE COUNTY



FOOTING REINFORCING LAYOUT (EACH PHASE)



FOOTING PILE LAYOUT

PIER NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE

2" UNLESS OTHERWISE NOTED OR SHOWN.

ALL EXPOSED CORNERS 90° OR SHARPER TO BE FILLETED WITH A 3" DRESSED AND

ALL REINFORCING IS TO BE SECURLY WIRED IN PLACE BEFORE CONCRETE IS POURED.

30 - HPI4×89 STEEL BEARING PILING ARE REQUIRED FOR EACH FOOTING.

CONCRETE FORM LINERS AND REQUIRED TEXTURED CONCRETE MOCK-UP PANEL.

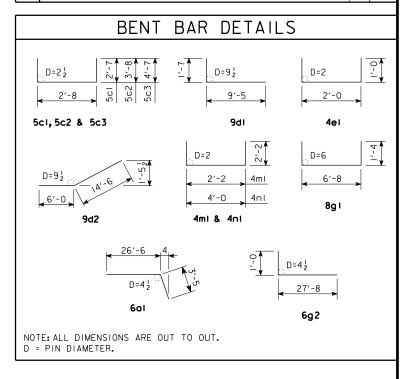
HR

HDR Fnaineering, Inc.

DESIGN TEAM ATN/JPS/DHS

THE DESIGN BEARING FOR THE PIER PILES IS 78 TONS. SEE GENERAL NOTES ON DESIGN SHEET 2 FOR ADDITIONAL NOTES REGARDING TEXTURED

REINFORCING STEEL (EACH PHASE) SHAPE NO. LENGTH WEIGHT RΔR LOCATION WALL, TOP, LONGITUDINAL 6al 12 29'-11 539 WALL, SIDE, LONGITUDINAL 8 218 5a2 26'-2 WALL, SIDE, LONGITUDINAL 2 36′-6 76 5a3 5a4 WALL, SIDE, LONGITUDINAL 16'-0 33 2 5a5 WALL, SIDE, LONGITUDINAL 32′-8 68 5a6 WALL, SIDE, LONGITUDINAL 2 19'-9 41 WALL, SIDE, LONGITUDINAL 2 20′-9 5a7 43 5a8 WALL, SIDE, LONGITUDINAL 8'-11 5a9 WALL, SIDE, LONGITUDINAL 2 25′-0 52 5aIO WALL, SIDE, LONGITUDINAL 60 27'-2 1700 늘 5c1 CAP, STIRRUP L____ 68 5c2 CAP, STIRRUP 115 CAP, STIRRUP 5c3 L___ 23 | II'-IO 9dI FOOTING TO COLUMN DOWELS L___ 108 11'-0 4039 WALL, VERTICAL, ENDS ED 9d2 6 20'-6 418 WALL, VERTICAL 13'-8 186 9d3 4 WALL, VERTICAL 98 20′-6 6831 9d4 0 4eI WALL, ENDS 4'-0 107 LJ 40 FOOTING, TOP 53 6'-8 531 6f2 FOOTING, TOP 14 27'-8 582 0 FOOTING, BOTTOM LJ 64 1595 8g I 9'-4 ۵ FOOTING, BOTTOM 14 28′-8 603 6g2 ш PEDESTAL, TRANSVERSE L____ 48 4ml PEDESTAL, LONGITUDINAL **└** 30 8′-4 167 REINFORCING STEEL EPOXY COATED - TOTAL (LBS.) 19.011



DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80

PIER NO. I DETAILS

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

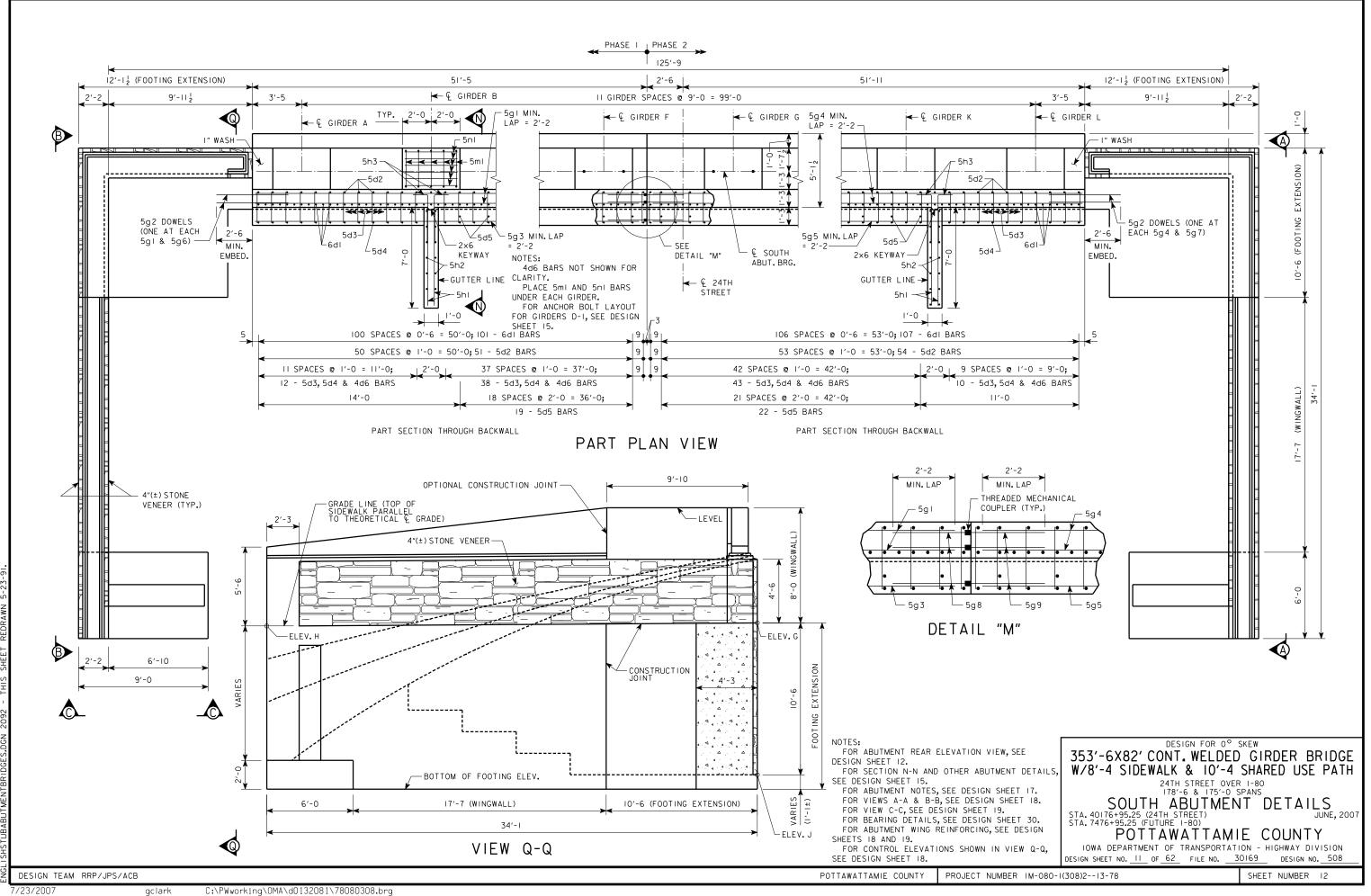
JUNE, 2007

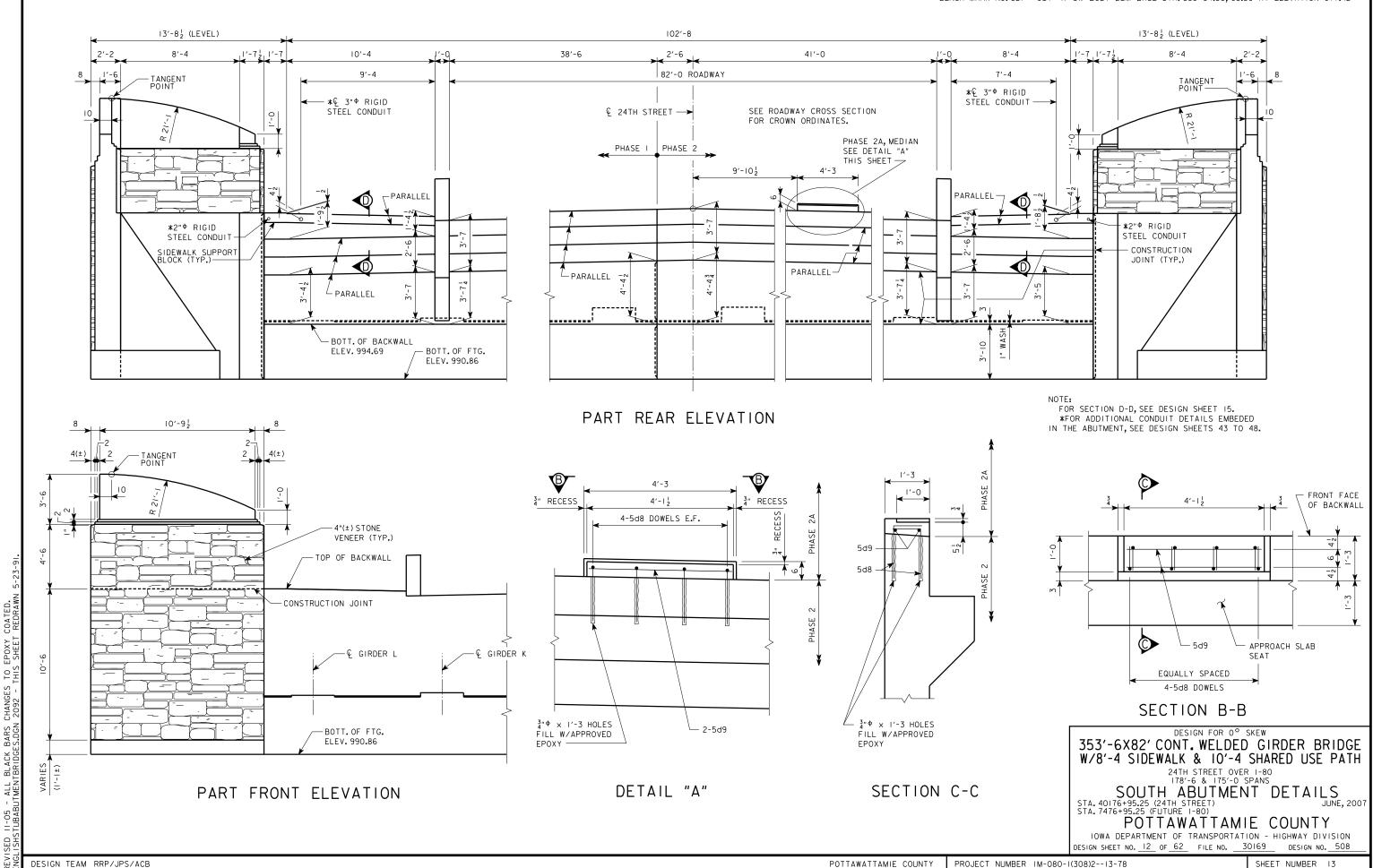
POTTAWATTAMIE COUNTY

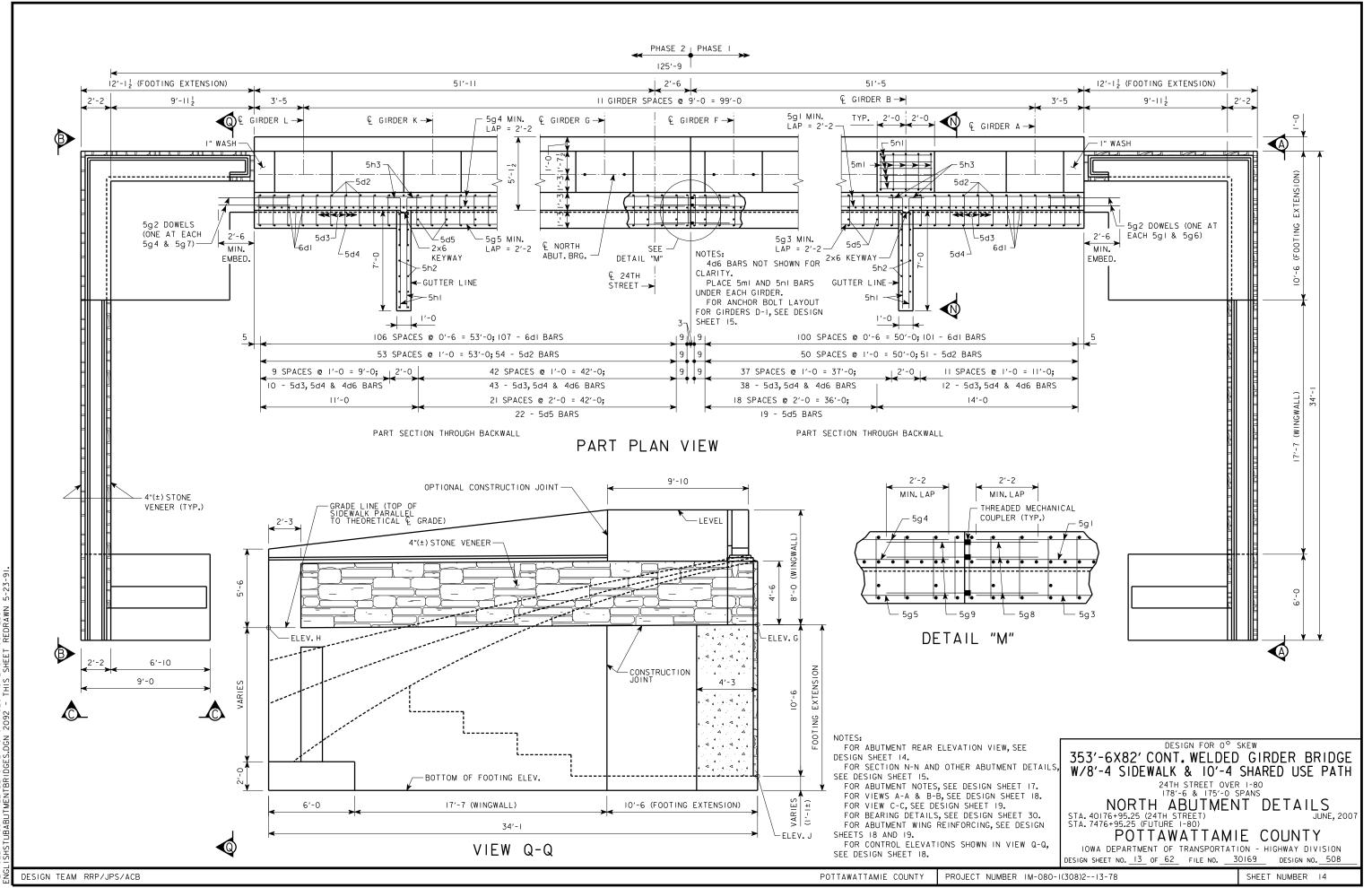
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 10 OF 62 FILE NO. 30169 DESIGN NO. 508

PROJECT NUMBER IM-080-1(308)2--13-78

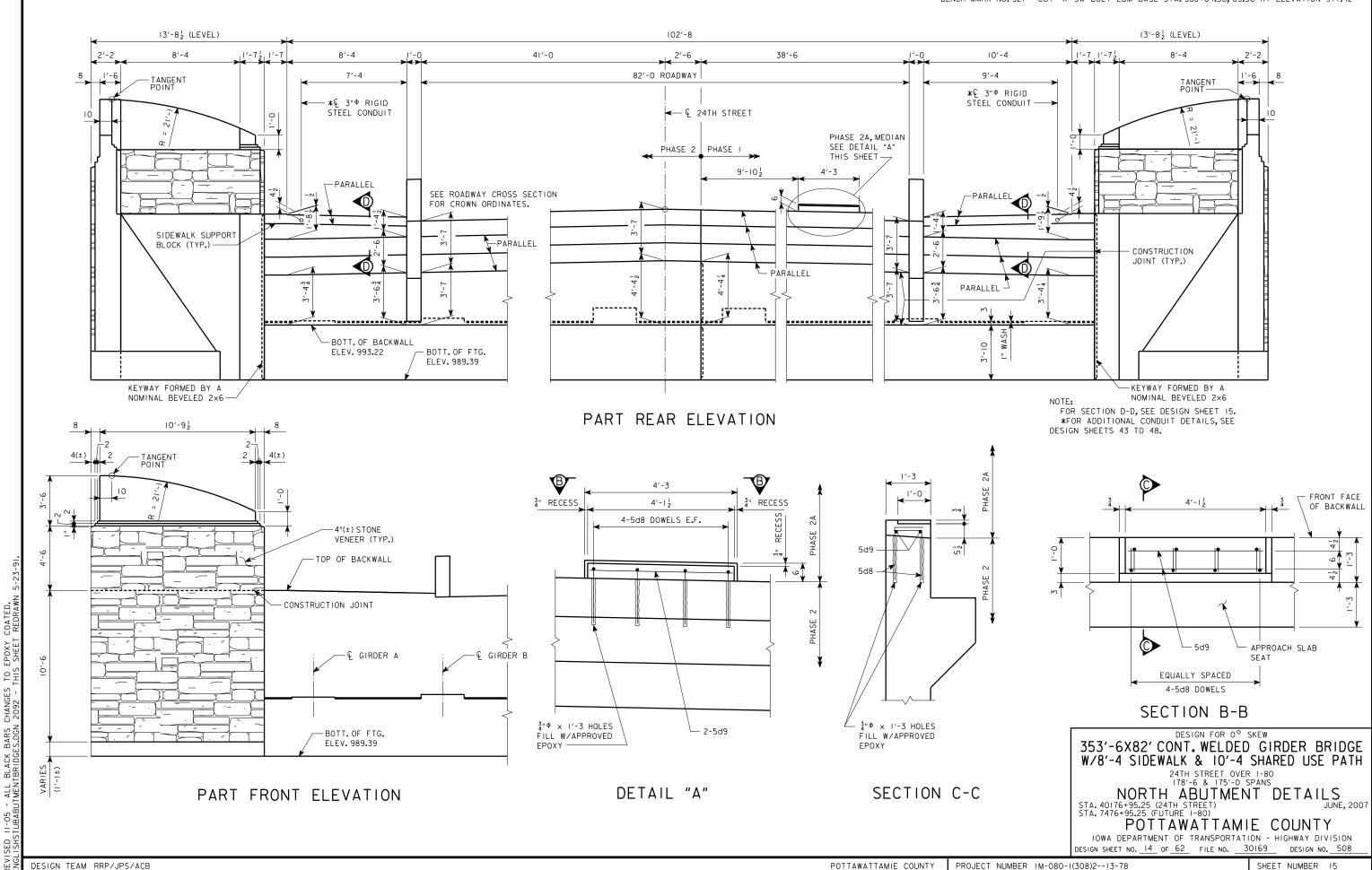
SHEET NUMBER II

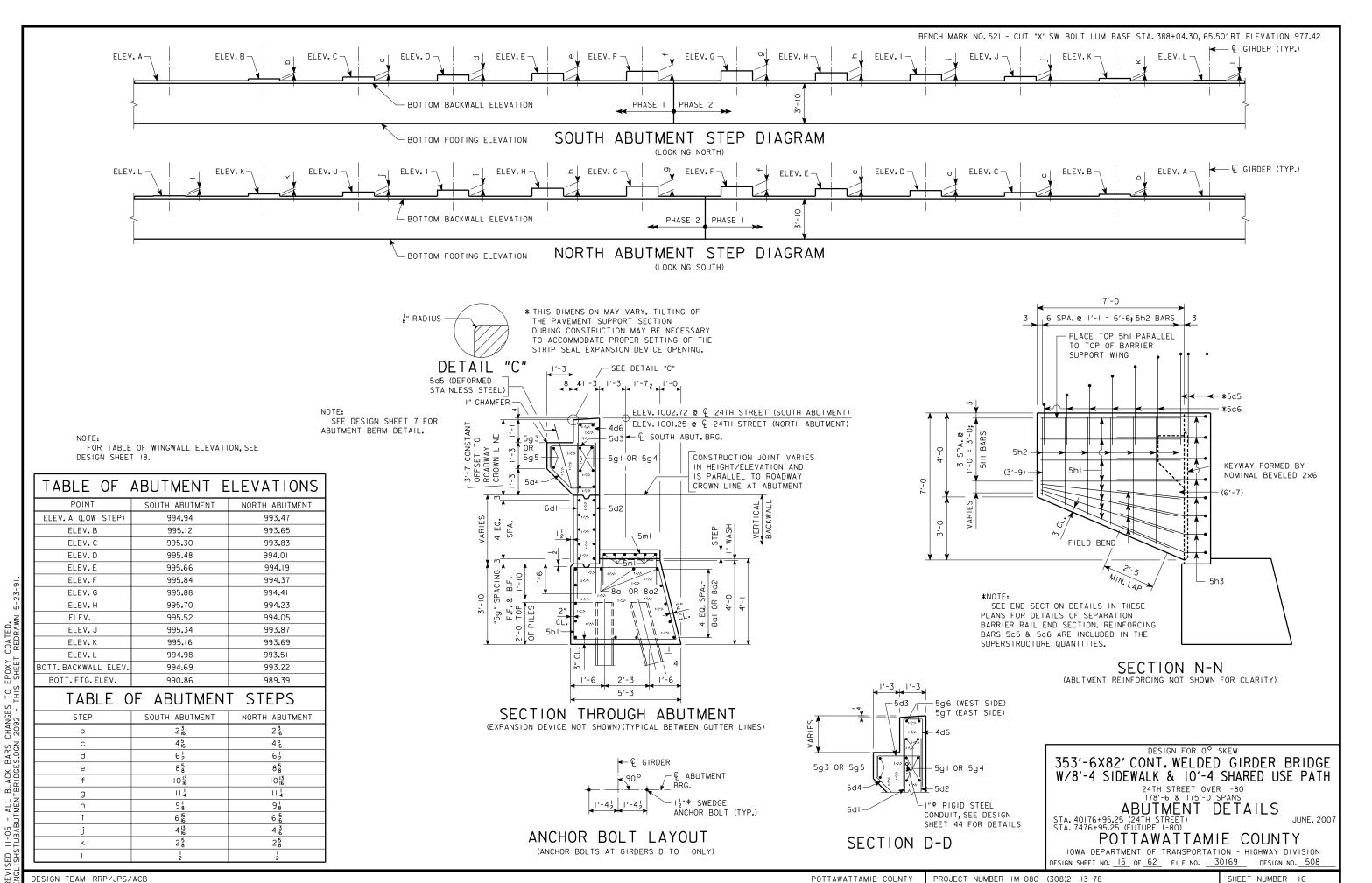




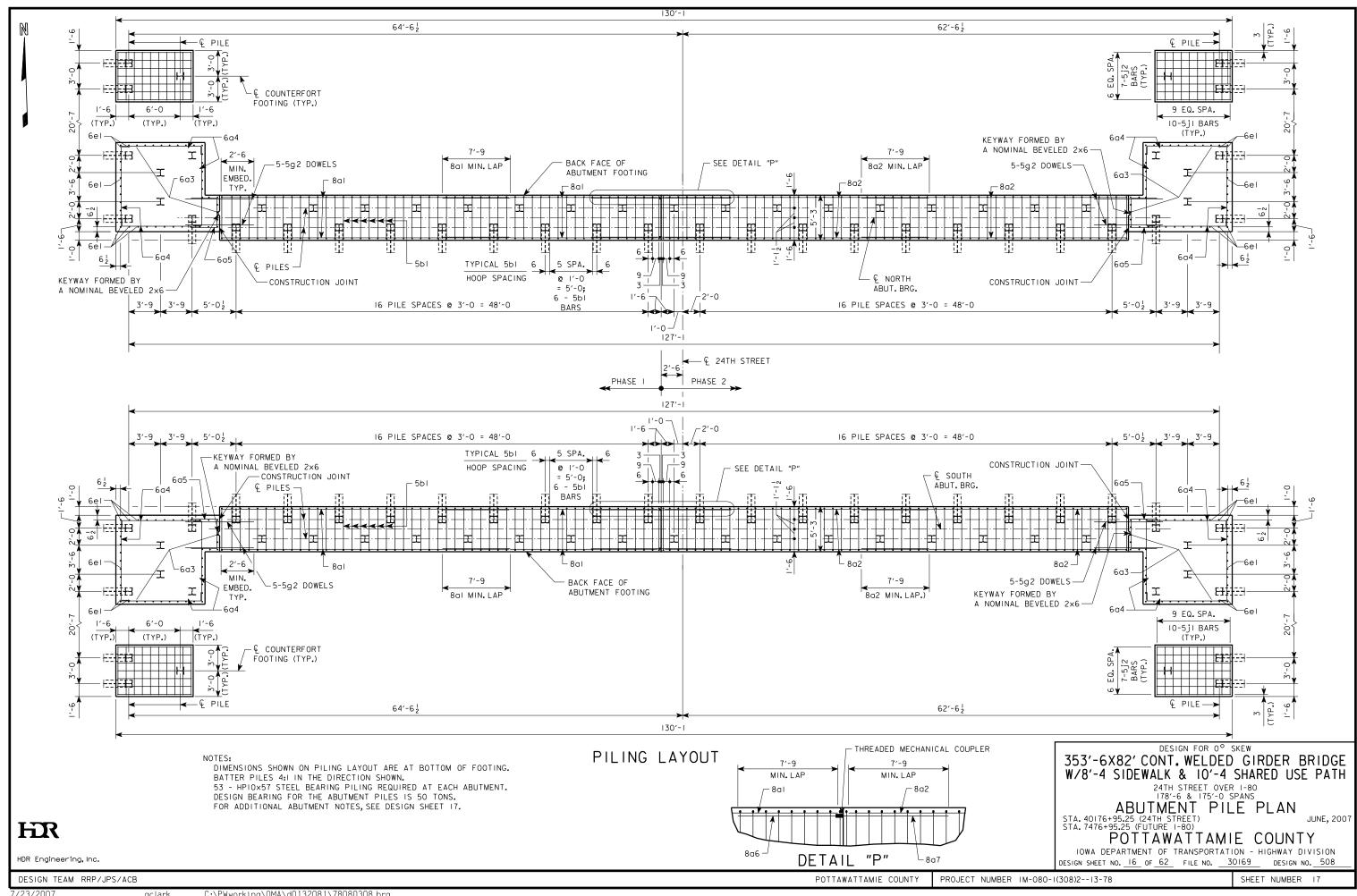


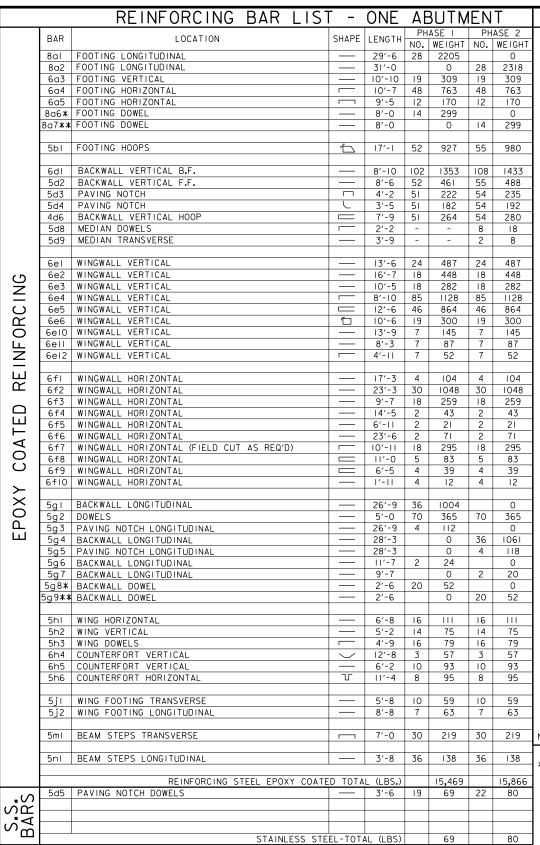
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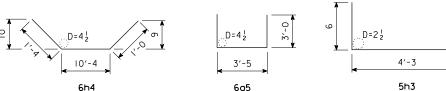
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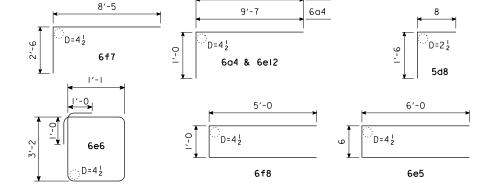


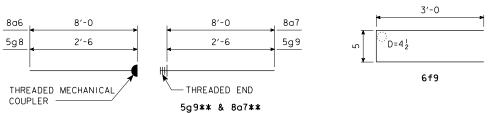
BENT BAR DETAILS 4'-01 D=2 2'-2 5b1 D=2 2 ...D=2 1 D=2 2 5d3 4d6 4'-11

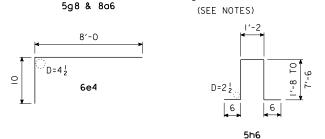
6e12



3'-11







NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIA

*INCLUDES | THREADED MECHANICAL COUPLER.

**SFF ABUTMENT NOTES.

ABUTMENT NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. CONSTRUCTION JOINT KEYWAYS ARE TO BE FORMED WITH BEVELED

THE PORTION OF THE BACKWALL CONTAINING THE ABUTMENT ANCHORAGE OF THE EXPANSION DEVICE IS TO BE PLACED AFTER THE BRIDGE FLOOR IS PLACED.

CONCRETE SEALER IS TO BE APPLIED TO THE ABUTMENT BRIDGE SEAT IN ACCORDANCE WITH THE CURRENT IOWA D.O.T. STANDARD SPECIFICATIONS.

THE COST OF PREFORMED EXPANSION JOINT FILLER, AND COST OF FURNISHING AND PLACING CONCRETE SEALER IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)".

PAVING NOTCH DOWEL (5d5) SHALL BE DEFORMED BAR GRADE 60, TYPE 316 LN IN ACCORDANCE WITH ASTM A955/A955M-OI. THE COST AND WEIGHT OF THE STAINLESS STEEL PAVING NOTCH DOWEL IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL-EPOXY COATED"

IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE

ALL THREADED MECHANICAL COUPLER ASSEMBLIES TO BE USED IN SPLICING THE REINFORCING IN THE ABUTMENT SHALL BE EPOXY COATED. THREE ADDITIONAL NON EPOXY COATED SPLICE ASSEMBLIES OF EACH SIZE SHALL BE FURNISHED TO THE ENGINEER FOR TESTING AND APPROVAL. THE COST OF ALL COUPLERS, INCLUDING THE 3 TO BE FURNISHED FOR TESTING, IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL EPOXY COATED" AND NO ADDITIONAL PAYMENT WILL BE MADE. THE WEIGHT OF THE MECHANICAL COUPLERS IS NOT INCLUDED. IN THE QUANTITY SHOWN FOR "REINFORCING STEEL EPOXY COATED".

EXPOSED CONCRETE SURFACES OF THE ABUTMENT WING WALLS ABOVE THE STONE VENEER SHALL BE SMOOTH AND SHOW NO WOOD GRAIN OR OTHER TEXTURE FROM THE FACE OF THE FORMS USED. ALL COSTS FOR REPAIRS OR COVERING THE WOOD GRAIN OR OTHER TEXTURE ON THESE SURFACES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

** THE LENGTHS SHOWN DO NOT INCLUDE AN ALLOWANCE FOR THE THREADED ENDS. BAR LENGTHS MAY NEED TO INCREASE DEPENDING ON THE MECHANICAL COUPLER ASSEMBLY USED. THE COST OF ALL THREADED PORTIONS OF THESE BARS IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL EPOXY COATED" AND NO ADDITIONAL PAYMENT WILL BE MADE. THE WEIGHT OF THE THREADED ENDS IS NOT INCLUDED IN THE QUANTITY SHOWN FOR "REINFORCING STEEL EPOXY COATED".

ALL BATTERED PILING SHALL BE TRIMMED TO THE HORIZONTAL LINE TO AID IN THE PLACEMENT OF THE REINFORCING.

CONCRETE PLACEM	JENT	QUAI	TITM	IES				
LOCATION	SOUTH	ABUT.	NORTH	NORTH ABUT.				
	PHASE I	PHASE 2	PHASE I	PHASE 2				
FOOTING AND STEPS	37.2	39.4	37.2	39.4				
BACKWALL BELOW CONSTR. JOINT	9.2	9.9	9.1	9.8				
BACKWALL ABOVE CONSTR. JOINT	13.2	13.9	13.2	13.9				
BARRIER SUPPORT WING	1.4	1.4	1.4	1.4				
WING FOOTING	4.0	4.0	4.0	4.0				
COUNTERFORT	1.5	1.5	I . 5	1.5				
WINGWALL	31.9	31.8	31.8	31.7				
FOOTING EXTENSION	47.6	47.5	47.5	47.4				
TOTAL (C.Y.)	146.0	149.4	145.7	149.1				

ESTIMATED QUANTITIES - BOTH ABUTMENTS										
ITEM	UNIT	SOUTH	ABUT.	NORTH	ABUT.	TOTAL				
I I E IVI	UNIT	PHASE I	PHASE 2	PHASE I	PHASE 2	TOTAL				
STRUCTURAL CONCRETE (HIGH PERFORMANCE)	C.Y.	146.0	149.4	145.7	149.1	590.2				
REINFORCING STEEL - EPOXY COATED	LB.	15,538	15,946	15,538	15,946	62,968				
CLASS 20 EXCAVATION	C.Y.	187	191	186	190	754				
HPIO×57 STEEL BEARING PILING	L.F.	26 @ 125	27 @ 125	26 @ 125	27 @ 125	13,250				
STONE VENEER	S.F.	617	617	617	617	2,468				

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

ABUTMENT DETAILS

JUNE, 2007

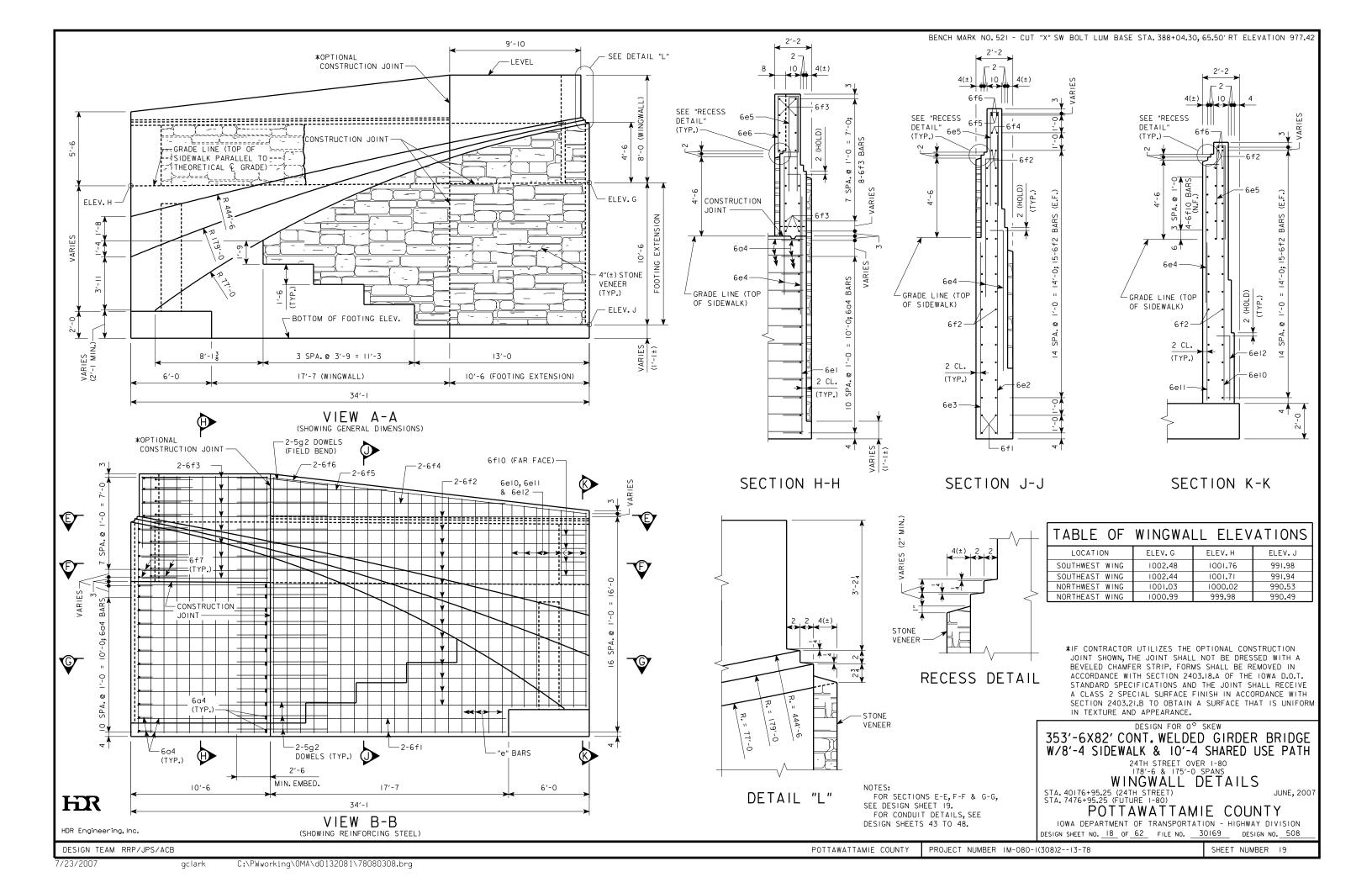
STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80) POTTAWATTAMIE COUNTY

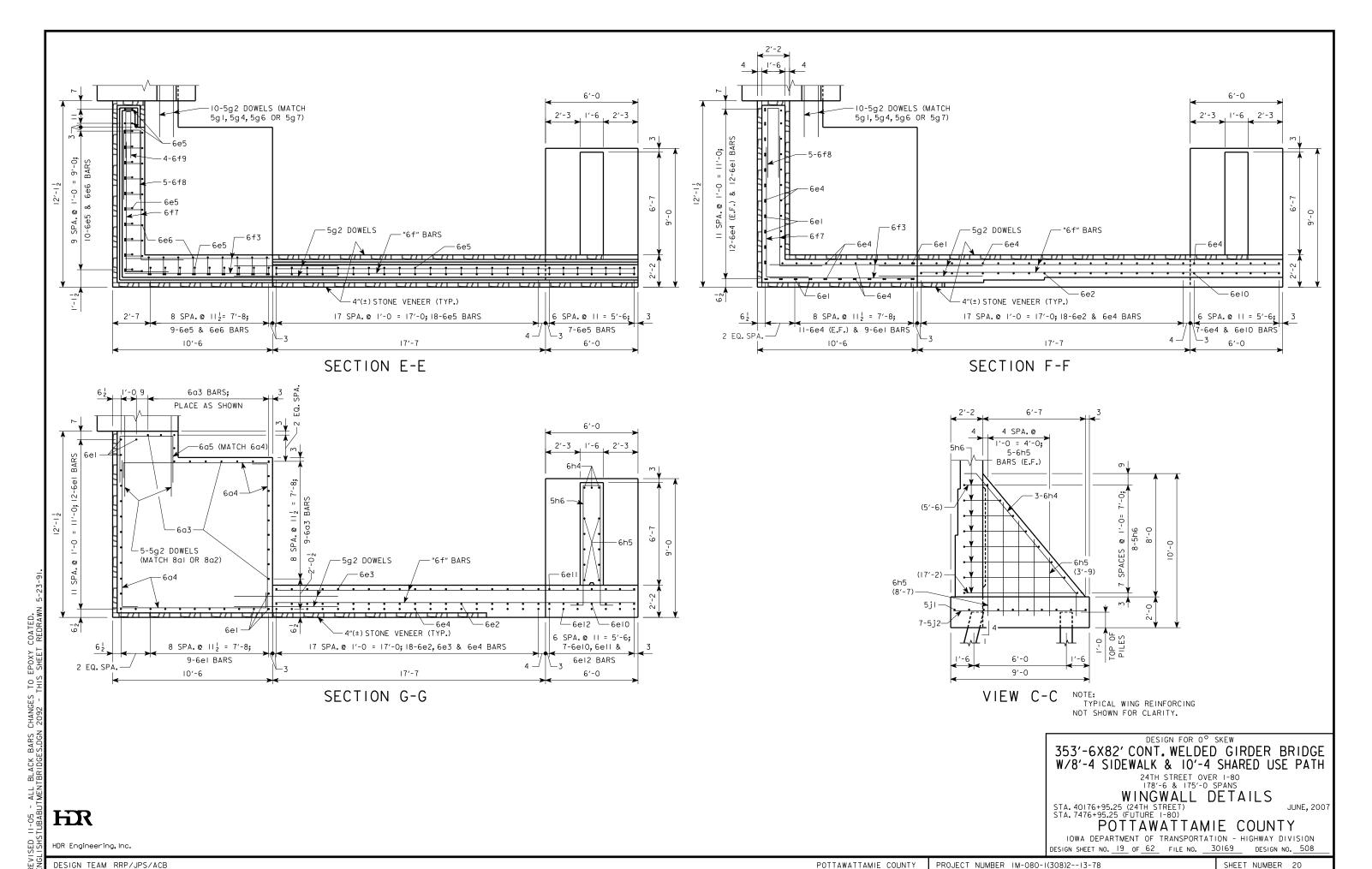
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 17 OF 62 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM RRP/JPS/ACB

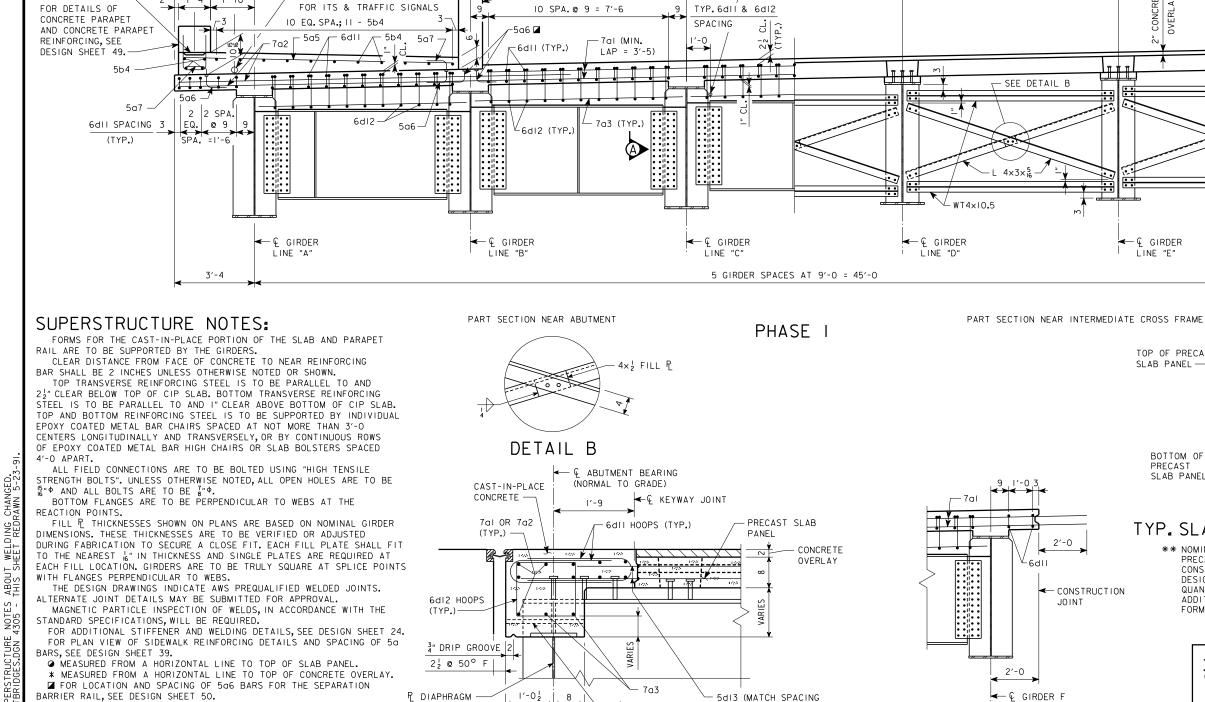
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POTTAWATTAMIE COUNTY PROJECT NUMBER IM-080-I(308)2--I3-78





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I"♥ RIGID

SECTION A-A

(NORMAL TO ABUTMENT)

STANDARD SHEET 4305

STEEL CONDUIT

GUTTER LINE

FOR DETAILS OF SEPARATION

BARRIER RAIL REINFORCING,

SEE DESIGN SHEET 50.

BARRIER RAIL AND SEPARATION

← Ç GIRDER LINE "F" - CONCRETE TOP OF PRECAST OVERLAY 2% BOTTOM OF SLAB PANEL 4" NOMINAL CIP HAUNCH TYP. SLAB & NOMINAL HAUNCH DETAIL ** NOMINAL HAUNCH DIMENSION SHOWN IS MEASURED FROM BOTTOM OF PRECAST SLAB PANEL TO TOP OF WEB. THEORETICALLY THIS IS A CONSTANT DIMENSION ALONG THE GIRDER AND IS USED BY THE DESIGNER TO SET BRIDGE SEAT ELEVATIONS AND ESTIMATE CONCRETE QUANTITIES. REFER TO THE FIELD HAUNCH DATA DETAIL SHEET FOR ADDITIONAL INFORMATION TO AID THE CONTRACTOR, METHOD FOR FORMING THE HAUNCH IS TO BE DETERMINED BY THE CONTRACTOR. DESIGN FOR O° SKEW 353'-6X82' CONT. WELDED GIRDER BRIDGE

HR

HDR Engineering, Inc.

DESIGN TEAM RRP/JPS/DHS

HOLD THIS LINE FOR PRECAST SLAB

PANEL ERECTION -

LEVEL

1×8×0′-8 INDENTATION SPACED AT 2'-0

> W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH 24TH STREET OVER I-80

178'-6 & 175'-0 SPANS

SUPERSTRÜCTÜRE DETAILS STA. 40176+95.25 (24TH STREET STA. 7476+95.25 (FUTURE 1-80) JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 20 OF 62 FILE NO. 30169 DESIGN NO. 508

POTTAWATTAMIE COUNTY

PHASE 2

3'-0

PARABOLIC

CROWN OVERLAY

1'-0

FOR DETAILS OF

PANEL, SEE DESIGN

SHEETS 32 TO 37.

PRECAST SLAB

5'-6

1'-6

← \$ 24TH

— LONGITUDINAL

JOINT

CONSTRUCTION

STREET

& P.G.L

10'-4 SHARED USE PATH

3"♥ RIGID STEEL CONDUIT

FACE OF PARAPET

'-0

1'-10

FOR LIGHTING DETAILS, SEE DESIGN SHEETS 43 TO 48.

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30' RDWY. WELDED GIRDER CROSS SECTION

OF PRECAST SLAB PANEL

MECHANICAL ANCHORS)

52'-4 OUT TO OUT OF SLAB (PHASE I)

-I"♥ RIGID STEEL CONDUIT

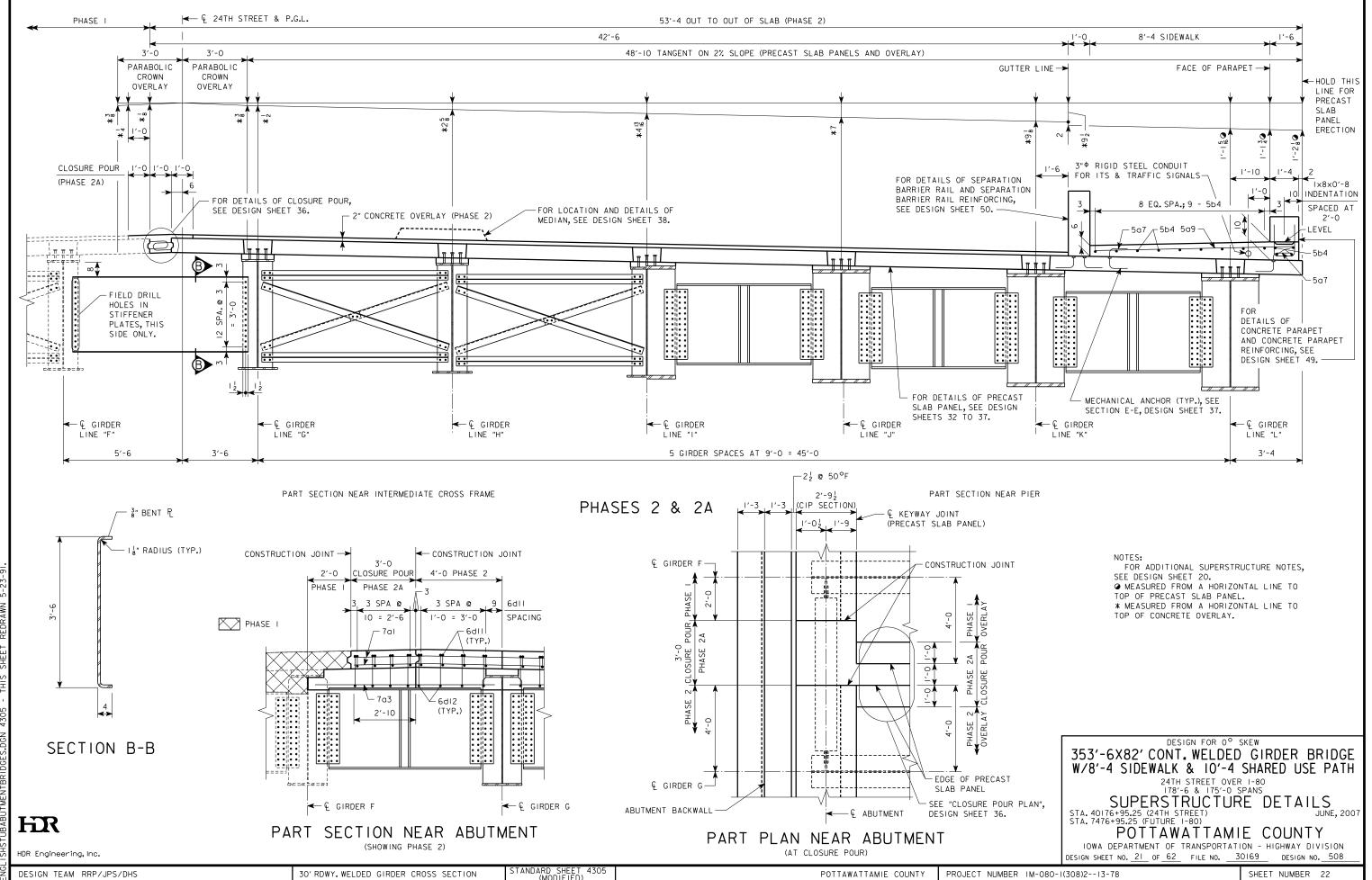
FOR UNDERDECK LIGHTS

50'-10 TANGENT ON 2% SLOPE (PRECAST SLAB PANELS AND OVERLAY)

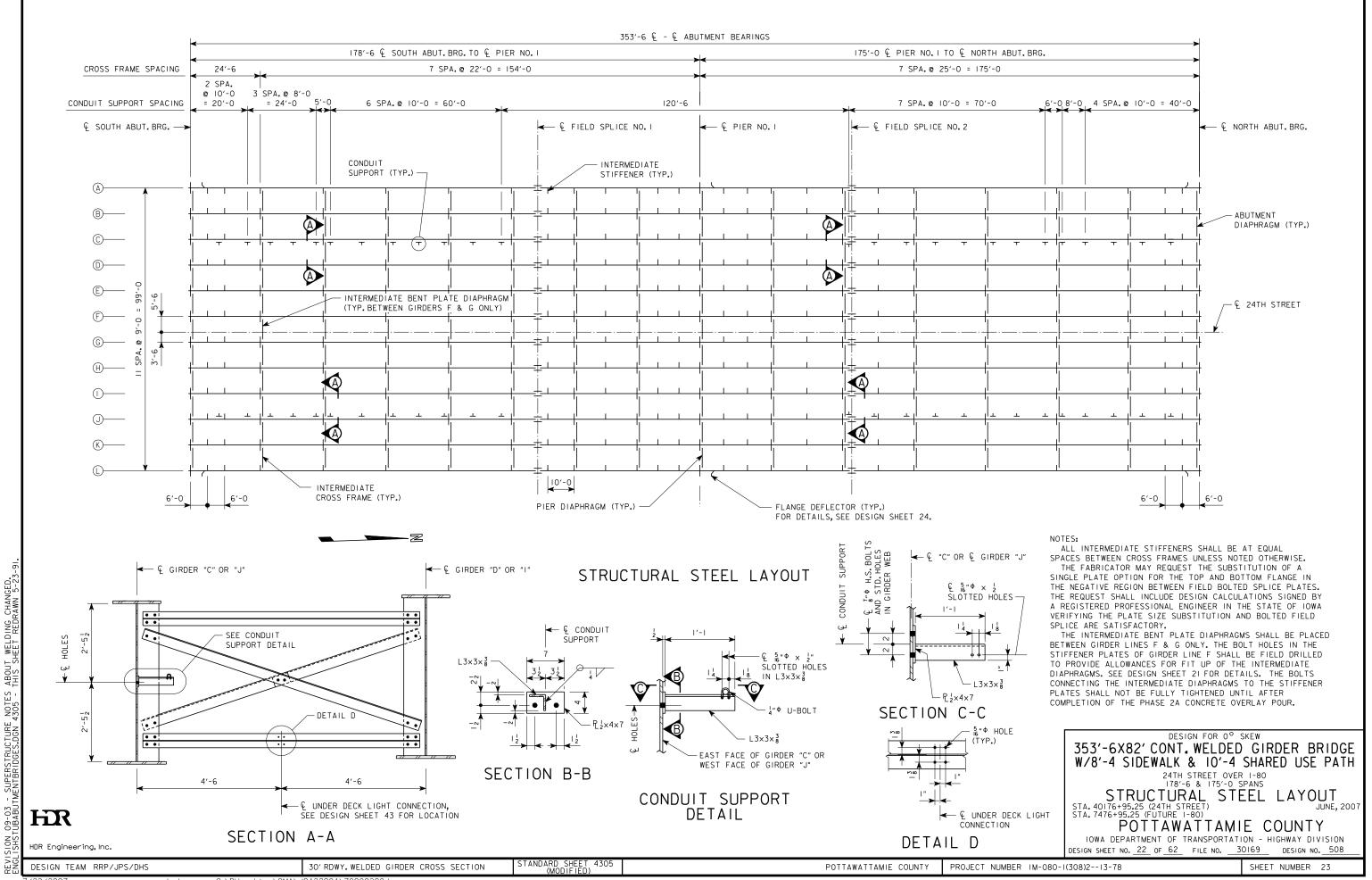
39'-6

PART SECTION NEAR ABUTMENT

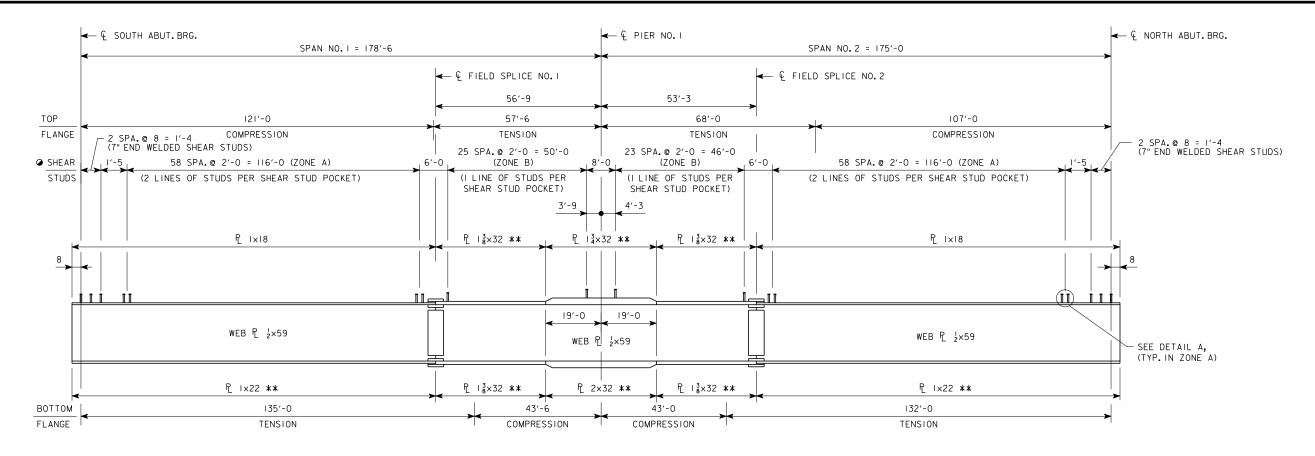
PROJECT NUMBER IM-080-I(308)2--I3-78



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GIRDER ELEVATION

- ** DENOTES ASTM A709 GRADE HPS 70W.
 MINIMUM YIFLD POINT = 70 KSI.
- ◆ FOR ADDITIONAL SHEAR STUD PLACEMENT DETAILS, SEE DESIGN SHEET 33.

WEATHERING STEEL NOTES: ALL STRUCTURAL STEEL, EXCEPT AS NOTED, SHALL CONFORM TO ASTM A709 GRADE HPS 50W. THE MINIMUM YIELD POINT FOR GRADE HPS 50W STRUCTURAL STEEL IS 50 KSI FOR PLATES 4 INCHES AND UNDER IN THICKNESS, AND ALL STRUCTURAL SHAPES. THE GRADE HPS 50W STEEL IS A WEATHERING STEEL AND IS TO REMAIN UNPAINTED, EXCEPT AS NOTED.

ALL PIECES COMPRISING THE ABUTMENT AND PIER BEARINGS SHALL COMPLY WITH THE REQUIREMENTS AS STATED IN THE NOTES ON DESIGN SHEETS 30 AND 31.

SHEAR STUDS ARE TO BE OF AN APPROVED TYPE LISTED IN MATERIALS I.M. 453.10, APPENDIX A.

THE FINISH ON BEARINGS AND WEATHERING STEEL SHALL BE IN ACCORDANCE WITH THE PLAN NOTES AND STANDARD SPECIFICATIONS 2408. EXTERIOR SURFACES OF ALL GALVANIZED COMPONENTS WHICH ARE DESIGNATED IN THE CONTRACT DOCUMENTS TO BE PAINTED SHALL BE PREPARED ACCORDING TO SUPPLEMENTAL SPECIFICATIONS SS-01025.

THE GRADE HPS 50W STEEL FOR THE WEBS OF THE EXTERIOR GIRDERS OF THE BRIDGE SHALL BE OF THE SAME TYPE AND FROM THE SAME STEEL MILL.

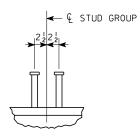
BOLTS FOR USE WITH WEATHERING STEEL SHALL BE A325 TYPE III WITH A563 GRADE DH3 NUTS AND F436 TYPE III WASHERS.

BOLTS USED TO SPLICE GIRDER SECTIONS ARE TO BE INSTALLED SUCH THAT NUTS ARE ON THE INSIDE FACE OF THE GIRDER WEBS FOR THE EXTERIOR GIRDERS, AND ON THE TOP OF BOTH TOP AND BOTTOM FLANGES OF ALL THE GIRDERS.

THE STEEL SHALL BE KEPT FREE OF OIL, GREASE, DIRT, CRAYON OR CHALK MARKS, CONCRETE SPATTER AND ANY OTHER FOREIGN MATTER THAT MAY AFFECT THE NATURAL OXIDATION OF THE STEEL. ANY FOREIGN MATTER REMAINING ON THE STEEL AFTER COMPLETION OF BRIDGE CONSTRUCTION SHALL BE REMOVED BY THE BRIDGE CONTRACTOR AS DIRECTED BY THE ENGINEER. THE RESULTANT SURFACE SHALL BE FREE OF ALL VISIBLE RESIDUES. ALL COSTS ASSOCIATED WITH CLEANING STEEL SURFACES SHALL BE BORNE BY THE BRIDGE CONTRACTOR.

SEAL MATERIAL FOR CAULKING SHALL BE NEUTRAL CURE AND NON SAG SILICONE. THREE PRODUCTS MEETING THESE CRITERIA ARE DOW 888, CSL342 JOINT SEALANT AND CRAFCO ROADSAVER SILICONE.

TO ENSURE UNIFORM WEATHERING, ALL UNPAINTED AREAS OF OUTSIDE SURFACES OF THE FASCIA GIRDERS SHALL RECEIVE, AFTER BLASTING, AT LEAST THREE UNIFORM APPLICATIONS OF WATER MIST WITH A 24 HOUR INTERVAL BETWEEN APPLICATIONS. EACH APPLICATION SHALL BE APPLIED ON DRY SURFACES. THE WATER MIST APPLICATION SHALL BE PERFORMED WITHIN 48 HOURS AFTER THE PAINTED SURFACES HAVE BEEN PROPERLY CURED. ALL WATER MIST APPLICATIONS SHALL BE WITNESSED BY A REPRESENTATIVE OF THE CONTRACTING AUTHORITY.



DETAIL A

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

GIRDER ELEVATION

JUNE, 2007

STA. 40176+95.25 (24TH STREET)
STA. 7476+95.25 (FUTURE 1-80)
POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 23 OF 62 FILE NO. 30169 DESIGN NO. 508

HDR Engineering, Inc.

HR

DESIGN TEAM RRP/JPS/DHS 30' RDWY. WELDED GIRDER CROSS SECTION

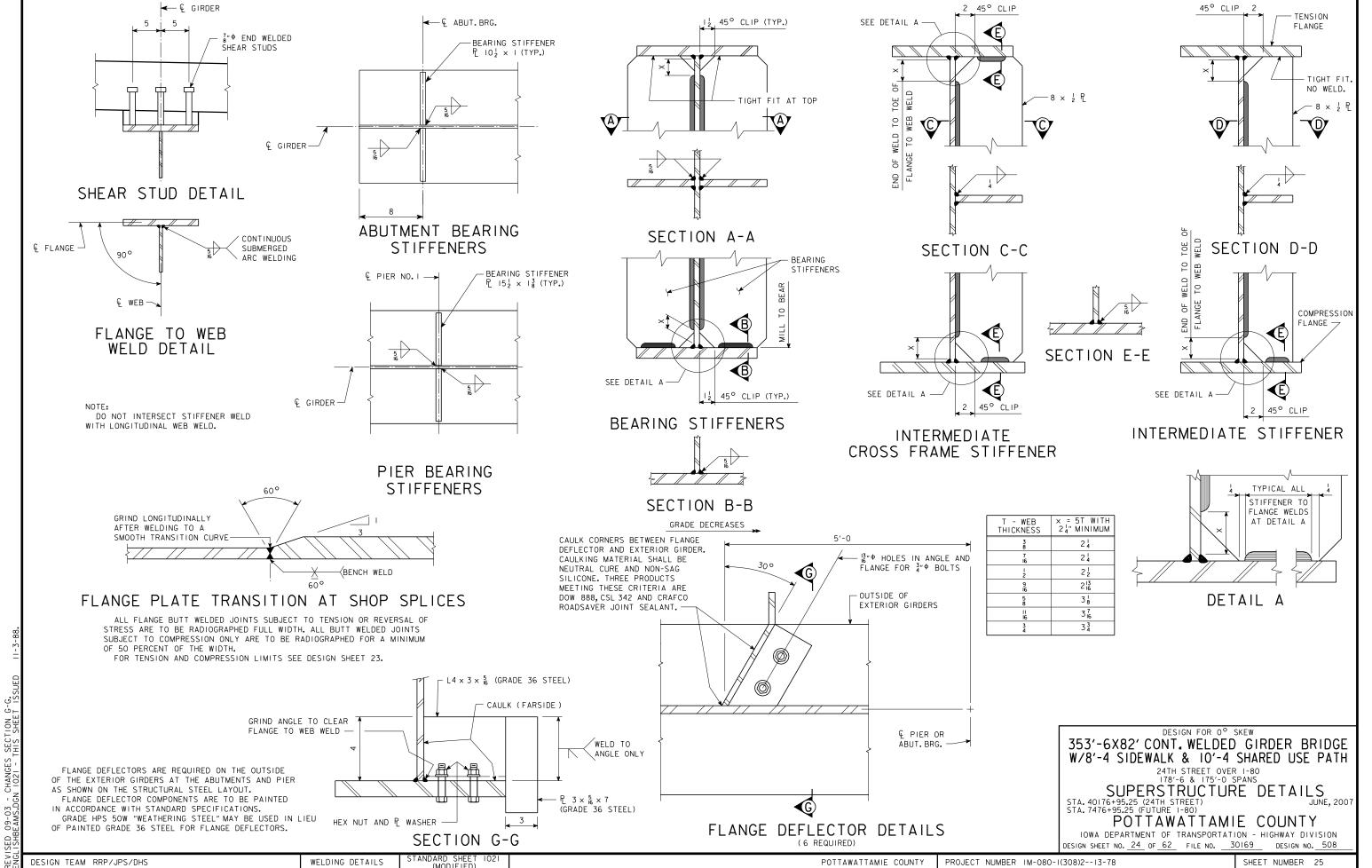
STANDARD SHEET 4305

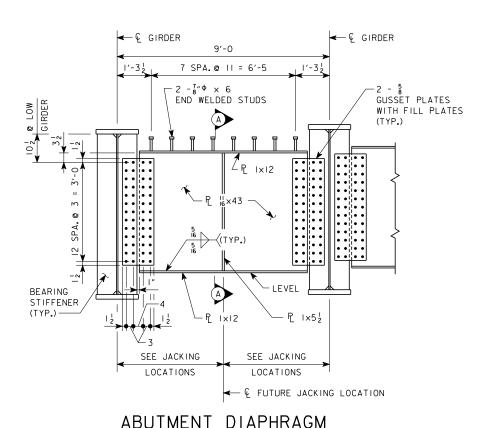
POTTAWATTAMIE COUNTY

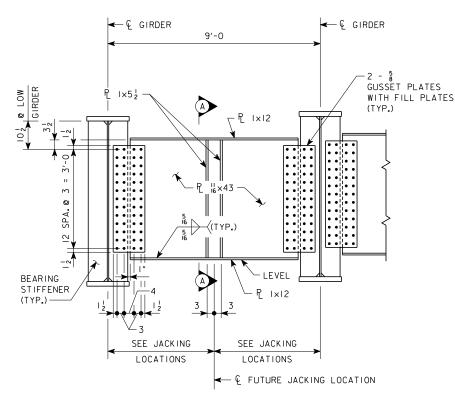
PROJECT NUMBER IM-080-I(308)2--I3-78

30169 DESIGN NO. 508

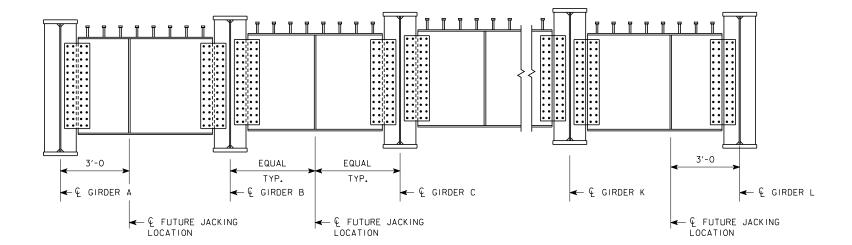
SHEET NUMBER 24





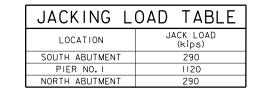


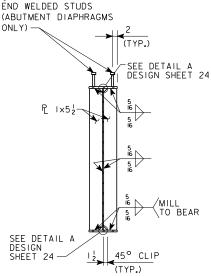
PIER NO. I DIAPHRAGM



JACKING LOCATIONS

(ABUTMENT DIAPHRAGMS SHOWN. PIER NO. I DIAPHRAGMS SIMILAR)





SECTION A-A

JACKING NOTES:

 $_{8}^{7}$ " $\Phi \times 6$

PROVISIONS FOR JACKING HAVE BEEN INCLUDED IN THIS DESIGN TO ALLOW FOR FUTURE BEARING MAINTENANCE.

JACK LOAD IS BASED ON MAXIMUM DEAD LOAD REACTION. (LIVE LOAD IS NOT INCLUDED). FUTURE JACKING AND BEARING REPLACEMENT SHALL BE CONDUCTED IN A MANNER SUCH THAT THE SUPERSTRUCTURE WILL NOT BE DAMAGED.

THE MAXIMUM ALLOWABLE VERTICAL JACKING DISPLACEMENT SHALL BE I" WITH RESPECT TO BEARINGS OF ADJACENT PIERS OR ABUTMENTS. IF ADDITIONAL JACKING HEIGHT IS REQUIRED TO FACILITATE BEARING REMOVAL, THE SUPERSTRUCTURE SHALL BE JACKED AT ADJACENT PIERS OR ABUTMENTS SUCH THAT THE MAXIMUM VERTICAL DIFFERENTIAL DISPLACEMENT BETWEEN ADJACENT PIERS OR ABUTMENTS DOES NOT EXCEED I".

ALL JACKING POINTS INDICATED ON THE DRAWINGS AT A PIER OR ABUTMENT LINE SHALL BE JACKED SIMULTANEOUSLY AND SHALL BE RAISED THE SAME AMOUNT AND AT THE SAME RATE. THE HYDRAULIC PRESSURE OF THE JACKS SHALL BE ADJUSTED AS REQUIRED TO ALLOW FOR EQUAL MOVEMENTS.

TRAFFIC SHALL NOT BE PERMITTED ON THE SUPERSTRUCTURE WHILE JACKING IS OCCURRING. EFFECTS OF VIBRATIONS FROM TRAFFIC NEAR THE SUBSTRUCTURE SHOULD BE CONSIDERED DURING JACKING AND BEARING REPLACEMENT.

WHEN JACKING AT ABUTMENTS, SEPARATION BARRIER AND MEDIAN COVER PLATES SHALL BE REMOVED TO PREVENT DAMAGE TO THE JOINTS. EXPANSION JOINTS SHOULD ALSO BE INSPECTED TO VERIFY THAT ITEMS PLACED ACROSS THE EXPANSION JOINTS WILL NOT BE DAMAGED BY JACKING.

THE BRIDGE SHALL BE INSPECTED PRIOR TO JACKING TO VERIFY THAT ITEMS CONNECTED TO THE SUPERSTRUCTURE OR SUBSTRUCTURE WILL NOT BE DAMAGED DURING THE JACKING AND BEARING REPLACEMENT PROCEDURE.

THE SUPERSTRUCTURE SHALL ONLY BE JACKED AT THE LOCATIONS NOTED ON THE DRAWINGS.

IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR PERFORMING THE JACKING TO SIZE AND DESIGN THE REQUIRED JACKING EQUIPMENT AND BLOCKING AND TO ESTABLISH THE PROCEDURE FOR JACKING AND BEARING REPLACEMENT. JACKS SHALL HAVE A MINIMUM SAFE LOAD CAPACITY OF 125% OF THE LOAD SPECIFIED IN THE JACKING LOAD TABLE.

PROVISIONS SHALL BE MADE TO ACCOUNT FOR THERMAL MOVEMENTS DURING THE PERIOD THAT THE STRUCTURE IS RESTING ON TEMPORARY SUPPORTS.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

JACKING DETAILS

STA. 40176+95.25 (24TH STREET STA. 7476+95.25 (FUTURE 1-80)

JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 25 OF 62 FILE NO. 30169 DESIGN NO. 508

HDR Engineering, Inc.

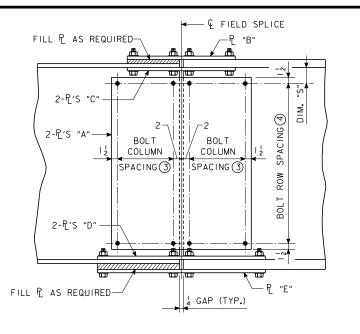
HR

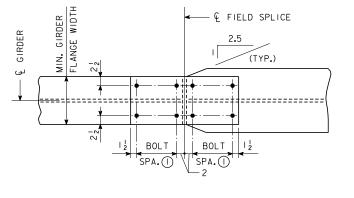
DESIGN TEAM JPS/RRP/DHS

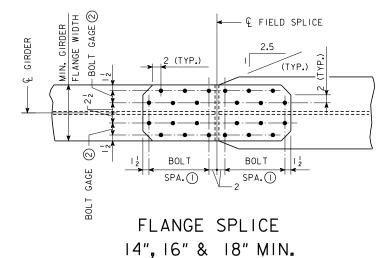
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POTTAWATTAMIE COUNTY

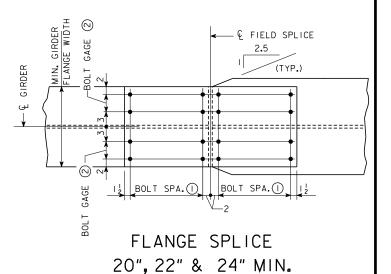
PROJECT NUMBER IM-080-I(308)2--I3-78







GIRDER FLANGE WIDTH



GIRDER FLANGE WIDTH

FIELD SPLICE ELEVATION

FLANGE SPLICE 12" MIN. GIRDER FLANGE WIDTH

FIELD SPLICE SCHEDULE TOP FLANGE SPLICE WEB PLATE SPLICE BOTTOM FLANGE SPLICE MIN. BOLT FIELD BOLT BOLT WEB BOLT ROW BOLT BOLT GIRDER PLATE "C" COLUMN DIM. PLATE "D" GIRDER SPLICE GAGE 2 PLATE "B" SPACING PLATE "A" SPACING SPACING GAGE PLATE "E" FLANGE (2 REQUIRED) SPACING "S" FLANGE (2 REQUIRED) NO. 2 (4) (1)(2 REQUIRED) (1)(3) WIDTH WIDTH 1 & 2 18 7 16×18× 3'-7 ½×8× 3′-7 9 @ 2 = 1'-6 $\frac{3}{8} \times 19 \times 4' - 7$ 2 @ 3 = 0'-6 | 13 @ 4 = 4'-4 22 11 16×10×4′-7 5×22×4′-7 8 @ 3 = 2'-0

MOMI	ENT TABLE (L	JNITS: FOOT-K	IPS)
	POSITIVE MOMENT	NEGATIVE MOMENT	POSITIVE MOMENT
	SPAN NO. I	PIER	SPAN NO.2
DCI	2033	-5794	1852
DC2	2576	-5051	2398
L.L. + IMPACT	3727	-4115	3526
TOTAL	8336	-14960	7776

REACTIO	N TABLE	(UNITS: K	IPS)
	REACTION	REACTION	REACTION
	SOUTH ABUTMENT	PIER	NORTH ABUTMENT
DCI + DC2	123	553	118
L.L. + IMPACT	124	230	122
TOTAL	247	783	240

MOMENTS AND REACTIONS ARE UNFACTORED. MOMENTS AND PIER REACTIONS CONTROLLED BY EXTERIOR GIRDER AND ABUTMENT REACTIONS CONTROLLED BY INTERIOR GIRDER.

DCI COMPRISES ALL NON-COMPOSITE DEAD LOADS DUE TO GIRDER AND SLAB DEAD WEIGHT.

DC2 COMPRISES COMPOSITE DEAD LOAD DUE TO BARRIER RAILS, SIDEWALKS AND CONCRETE OVERLAY.

LL MOMENTS AND REACTIONS ARE BASED ON LRFD DISTRIBUTION FACTORS.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS FIELD SPLICE DETAILS

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 26 OF 62 FILE NO. 30169 DESIGN NO. 508

HR

HDR Engineering, Inc.

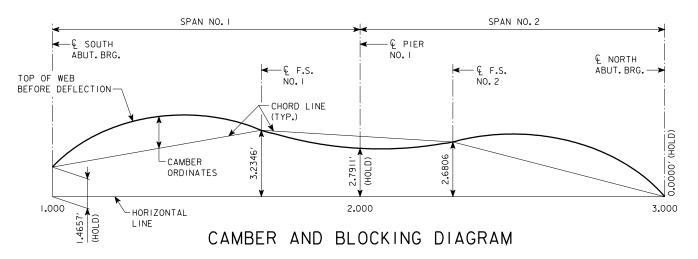
DESIGN TEAM RRP/JPS/DHS

30' RDWY. WELDED GIRDER CROSS SECTION

STANDARD SHEET 4305

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-1(308)2--13-78



CAMBER ORDINATES (INCHES)																									
	© SOUTH ABUT. BRG.				MIDPOINT OF FIELD SECTION				و F.S. NO. I				PIER NO. I				€ F.S. NO. 2				MIDPOINT OF FIELD SECTION				€ NORTH ABUT. BRG.
GIRDER	1.000	1.100	1.200	1.300	1.341	1.400	1.500	1.600	1.682	1.700	1.800	1.900	2.000	2.100	2.200	2.300	2.304	2.400	2.500	2.600	2.652	2.700	2.800	2.900	3.000
ALL	0.0000	4.16	7.24	8.82	8.97	8.6996	6.87	3.56	0.0000	-0.07	-0.80	-1 . 55	-1.89	-1.55	-0.80	-0.03	0.0000	3.68	6.60	8.19	8.41	8.25	6.75	3.88	0.0000

		D	EFLE	CTIC	N ORD)INAT		DUE) WNW		WEIGH DEFL		F SL IONS	AB, E ARE	BARR POS	IERS SITIN	SID E)(I	EWAL NCHE		/ERL	AY A	MD ME	EDIAI	1		
	© SOUTH ABUT. BRG.				MIDPOINT OF FIELD SECTION				و F.S. NO. I				PIER NO. I				Ç F.S. NO. 2				MIDPOINT OF FIELD SECTION				Ç NORTH ABUT. BRG.
GIRDER	1.000	1.100	1.200	1.300	1.341	1.400	1.500	1.600	1.682	1.700	1.800	1.900	2.000	2.100	2.200	2.300	2.304	2.400	2.500	2.600	2.652	2.700	2.800	2.900	3.000
ALL	0.0000	2.81	5.13	6.64	6.98	7.17	6.71	5.44	4.01	3.70	1.94	0.59	0.00	0.37	1.49	3.02	3.10	4 . 57	5.73	6.19	6.08	5.78	4.49	2.46	0.00

D	EFLEC	TION	ORE) INA	TES DL	JE T	O WE	EIGHT	OF	STR	UCTL	JRAL	STE	EL (DOWN	WAR[) DE	FLEC	TION	IS AF	RE PO	SITIN	/E)(I	NCHE	S)
	© SOUTH ABUT. BRG.				MIDPOINT OF FIELD SECTION				و F.S. NO. ا				PIER NO. I				£ F.S. NO. 2				MIDPOINT OF FIELD SECTION				€ NORTH ABUT. BRG.
GIRDER	1.000	1.100	1.200	1.300	1.341	1.400	1.500	1.600	1.682	1.700	1.800	1.900	2.000	2.100	2.200	2.300	2.304	2.400	2.500	2.600	2.652	2.700	2.800	2.900	3.000
ALL	0.00	0.60	1.10	1.42	1.50	1.54	1.45	1.18	0.87	0.81	0.43	0.13	0.00	0.06	0.29	0.60	0.62	0.92	1.17	1.27	1.25	1.19	0.92	0.50	0.00

								TOP	OF	SLA	3 EL	EVAT	IONS	AL(ONG	PGL								
© SOUTH ABUT. BRG.				MIDPOINT OF FIELD SECTION				F.S. NO. 1				PIER NO. I				F.S. NO. 2				MIDPOINT OF FIELD SECTION				Ç NORTH ABUT. BRG.
1.000	1.100	1.200	1.300	1.341	1.400	1.500	1.600	1.682	1.700	1.800	1.900	2.000	2.100	2.200	2.300	2.304	2.400	2.500	2.600	2.652	2.700	2.800	2.900	3.000
1002.72	1003.04	1003.32	1003.56	1003.65	1003.76	1003.91	1004.02	1004.08	1004.09	1004.12	1004.10	1004.05	1003.95	1003.81	1003.63	1003.63	1003.42	1003.16	1002.86	1002.69	1002.52	1002.14	1001.72	1001.25

NOTES:

CAMBER ORDINATES ARE MEASURED FROM A CHORD LINE BETWEEN FIELD SPLICES. UPWARD CAMBERS ARE POSITIVE. DEFLECTION ORDINATES FOR CAMBER INCLUDE DEFLECTIONS DUE

TO ALL DEAD LOADS. DOWNWARD DEFLECTIONS ARE POSITIVE.

TOP OF GIRDER ELEVATIONS FOR HAUNCH CALCULATIONS SHALL BE
SURVEYED PRIOR TO THE PLACEMENT OF PRECAST DECK PANELS.

HAUNCH THICKENING DIAGRAM NOT PROVIDED BECAUSE THE NOMINAL HAUNCH DIMENSION FROM BOTTOM OF PRECAST SLAB PANEL TO TOP OF GIRDER WEB SHOULD THEORETICALLY BE A CONSTANT DIMENSION. (SEE TYPICAL SLAB AND NOMINAL HAUNCH DETAIL, DESIGN SHEET 20.) GIRDER WEB SHALL BE CUT TO COMPENSATE FOR DEAD LOAD DEFLECTION AND VERTICAL CURVE CORRECTION.

CAMBER VALUES MUST BE MAINTAINED AT THE CENTER LINE OF ABUTMENT AND PIER BEARINGS.

CAMBERS ARE GIVEN FOR THE GIRDERS IN THE NO LOAD POSITION.
FOR INDIVIDUAL GIRDER SPAN LENGTHS AND DISTANCE TO FIELD
SPLICES, SEE DESIGN SHEET 23.

DESIGN FOR 0° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

CAMBER & BLOCKING DIAGRAMS
STA. 40176+95.25 (24TH STREET)
STA. 7476+95.25 (FUTURE 1-80)
JUNE, 200

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 27 OF 62 FILE NO. 30169 DESIGN NO. 508

HR

HDR Engineering, Inc.

DESIGN TEAM RRP/JPS/ACB

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-I(308)2--I3-78

								TABL	E OF	GIRE	ER L	INE	HAUN	ICH E	LEVA	TION	S								
	& SOUTH ABUT. BRG.													F.S. NO. I											F.S. NO. 2
LINE	0	2	3	4	5	6	7	8	9	(1)	(1)	(12)	(13)	(14)	(15)	(6)	(7)	(18)	(19)	20	21	22	23	24	25)
GIRDER LINE A	1000.91	1000.97	1001.28	1001.58	1001.84	1002.08	1002.27	1002.43	1002.54	1002.62	1002.66	1002.67	1002.65	1002.60	1002.54	1002.48	1002.40	1002.33	1002.27	1002.22	1002.18	1002.15	1002.13	1002.11	1002.07
GIRDER LINE B	1001.09	1001.14	1001.46	1001.75	1002.02	1002.25	1002.45	1002.60	1002.72	1002.80	1002.84	1002.85	1002.82	1002.78	1002.72	1002.65	1002.58	1002.51	1002.45	1002.40	1002.36	1002.33	1002.31	1002.28	1002.25
GIRDER LINE C	1001.27	1001.33	1001.64	1001.94	1002.20	1002.43	1002.63	1002.79	1002.90	1002.98	1003.02	1003.03	1003.01	1002.96	1002.90	1002.83	1002.76	1002.69	1002.63	1002.58	1002.54	1002.51	1002.49	1002.46	1002.43
GIRDER LINE D	1001.44	1001.50	1001.82	1002.11	1002.38	1002.61	1002.81	1002.96	1003.08	1003.16	1003.20	1003.21	1003.18	1003.14	1003.08	1003.01	1002.94	1002.87	1002.81	1002.76	1002.72	1002.69	1002.66	1002.64	1002.61
GIRDER LINE E	1001.63	1001.69	1002.00	1002.30	1002.56	1002.79	1002.99	1003.15	1003.26	1003.34	1003.38	1003.39	1003.37	1003.32	1003.26	1003.19	1003.12	1003.05	1002.99	1002.94	1002.90	1002.87	1002.85	1002.82	1002.79
GIRDER LINE F	1001.80	1001.86	1002.18	1002.47	1002.74	1002.97	1003.17	1003.32	1003.44	1003.52	1003.56	1003.57	1003.54	1003.50	1003.44	1003.37	1003.30	1003.23	1003.16	1003.12	1003.08	1003.05	1003.02	1003.00	1002.97
GIRDER LINE G	1001.85	1001.90	1002.22	1002.51	1002.78	1003.01	1003.21	1003.36	1003.48	1003.56	1003.60	1003.61	1003.58	1003.54	1003.48	1003.41	1003.34	1003.27	1003.21	1003.16	1003.12	1003.09	1003.07	1003.04	1003.01
GIRDER LINE H	1001.67	1001.73	1002.04	1002.34	1002.60	1002.84	1003.03	1003.19	1003.31	1003.38	1003.42	1003.43	1003.41	1003.36	1003.30	1003.24	1003.16	1003.09	1003.03	1002.98	1002.94	1002.91	1002.89	1002.87	1002.83
GIRDER LINE I	1001.49	1001.54	1001.86	1002.16	1002.42	1002.65	1002.85	1003.01	1003.12	1003.20	1003.24	1003.25	1003.23	1003.18	1003.12	1003.05	1002.98	1002.91	1002.85	1002.80	1002.76	1002.73	1002.71	1002.68	1002.65
GIRDER LINE J	1001.30	1001.36	1001.68	1001.97	1002.24	1002.47	1002.67	1002.82	1002.94	1003.02	1003.06	1003.07	1003.04	1003.00	1002.94	1002.87	1002.80	1002.73	1002.66	1002.62	1002.58	1002.55	1002.52	1002.50	1002.47
GIRDER LINE K	1001.13	1001.19	1001.50	1001.80	1002.06	1002.29	1002.49	1002.65	1002.76	1002.84	1002.88	1002.89	1002.87	1002.82	1002.76	1002.69	1002.62	1002.55	1002.49	1002.44	1002.40	1002.37	1002.35	1002.32	1002.29
GIRDER LINE L	1000.94	1001.00	1001.32	1001.61	1001.88	1002.11	1002.31	1002.46	1002.58	1002.66	1002.70	1002.71	1002.68	1002.64	1002.58	1002.51	1002.44	1002.37	1002.31	1002.26	1002.22	1002.19	1002.16	1002.14	1002.11
	L 1000.94 1001.00 1001.32 1001.61 1001.88 1002.11 1002.31 1002.46 1002.58 1002.66 1002.70 1002.71 10 TARIF OF CIRDER LINE HALINCH FLEVATIONS									·															

		IABL	E OF	GIKL	JEK L	_ INE	HAUN	ICH E	LEVA	TION	5		
													€ NORTH ABUT. BRG.
LINE	26	27	28	29	30	(31)	32	33	34	35)	36	37	38
GIRDER LINE A	1002.03	1001.96	1001.87	1001.75	1001.59	1001.40	1001.17	1000.90	1000.60	1000.26	999.90	999.51	999.44
GIRDER LINE B	1002.20	1002.14	1002.05	1001.93	1001.77	1001.58	1001.35	1001.08	1000.77	1000.44	1000.08	999.69	999.62
GIRDER LINE C	1002.39	1002.32	1002.23	1002.11	1001.95	1001.76	1001.53	1001.26	1000.95	1000.62	1000.26	999.87	999.80
GIRDER LINE D	1002.56	1002.50	1002.41	1002.29	1002.13	1001.94	1001.70	1001.44	1001.13	1000.80	1000.44	1000.05	999.98
GIRDER LINE E	1002.75	1002.68	1002.59	1002.47	1002.31	1002.12	1001.89	1001.62	1001.31	1000.98	1000.62	1000.23	1000.16
GIRDER LINE F	1002.92	1002.86	1002.77	1002.64	1002.49	1002.29	1002.06	1001.79	1001.49	1001.16	1000.80	1000.41	1000.34
GIRDER LINE G	1002.96	1002.90	1002.81	1002.69	1002.53	1002.34	1002.11	1001.84	1001.53	1001.20	1000.84	1000.45	1000.38
GIRDER LINE H	1002.79	1002.72	1002.63	1002.51	1002.35	1002.16	1001.93	1001.66	1001.36	1001.02	1000.66	1000.27	1000.20
GIRDER LINE I	1002.61	1002.54	1002.45	1002.33	1002.17	1001.98	1001.75	1001.48	1001.17	1000.84	1000.48	1000.09	1000.02
GIRDER LINE J	1002.42	1002.36	1002.27	1002.14	1001.99	1001.79	1001.56	1001.29	1000.99	1000.66	1000.30	999.91	999.84
GIRDER LINE K	1002.25	1002.18	1002.09	1001.97	18,1001	1001.62	1001.39	1001.12	18,0001	1000.48	1000.12	999.73	999.66
GIRDER LINE L	1002.06	1002.00	1001.91	1001.79	1001.63	1001.44	1001.20	1000.94	1000.63	1000.30	999.94	999.55	999.48

									ľ	MISC	ELLAN	NEOU:	S DA	TA T	ABLE												
	GIRDER	RLINE	€ SOUTH ABUT. BRG.													F.S. NO. I											F.S. NO. 2
			1	2	3	4	(5)	6	7	8	9	(1)	(1)	(12)	(3)	(14)	(15)	(6)	(7)	(18)	(19)	20	2)	22	23	24	25
ANTICIPATED DEFLECTION DUE TO SLAB, BARRIERS, CONCRETE OVERLAY, SIDEWALK AND MEDIAN (IN.)	AL	.L	0	5 16	1 7 8	3 3	4 116	5 3	6 %	7	7 ³ 16	7	6 %	5 ⁷	5	4	3	2 16	1 3 16	1 2	1 8	0	3 16	11	1 ³	2 16	3 8
CROSS SLOPE ADJUSTMENTS (IN.)	AL	.L	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	3 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16	5 16
ALLOWABLE FIELD	MAX.	ALL	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	316	311	316	316	316	3 16	3 16	316	316	316	316	316
HAUNCH (IN.)	MIN.	ALL	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	1 16	13 16	13 16	13 16	I 13	13 16	1 13	13 16	l 13	13 16	l 13	l 13	1 13

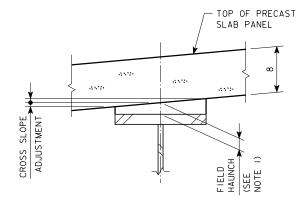
				MISC	ELLAN	NEOU:	S DA	TA T	ABLE						
	GIRDEF	R LINE													€ NORTH ABUT. BRG.
			26	27	28	29	30	31)	32	33	34)	35	36	37	38
ANTICIPATED DEFLECTION DUE TO SLAB, BARRIERS, CONCRETE OVERLAY, SIDEWALK AND MEDIAN (IN.)	Al	LL	4	4 13	5 2	6	6 16	6 8	5 ³	5 16	4 8	3	1 3	4	0
CROSS SLOPE ADJUSTMENTS (IN.)	Al	LL	3 16	3 16	3 16	3 16	3 16	3 16	3 16						
ALLOWABLE FIELD	MAX.	ALL	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13	3 13
HAUNCH (IN.)	MIN.	ALL	1 16	116	1 16	1 16	111	1 16	111	1 16	1 16	1 16	1116	1 16	111

TO CALCULATE FIELD HAUNCH NEEDED AT EACH LOCATION, SURVEY THE TOP OF GIRDER TOP FLANGES AT THE POINTS AND FIELD SPLICE LOCATIONS AS INDICATED IN THE TABLE OF GIRDER LINE HAUNCH ELEVATIONS. SUBTRACT THE SURVEYED GIRDER SHOT FROM THE "GIRDER LINE HAUNCH ELEVATION". THIS VALUE WILL BE THE FIELD HAUNCH NEEDED (SEE "FIELD HAUNCH" IN FIELD HAUNCH DETAIL). THE "GIRDER LINE HAUNCH ELEVATION" INCLUDES ADJUSTMENTS FOR PRECAST SLAB PANEL THICKNESSES AND ANTICIPATED DEFLECTIONS. NO ADDITIONAL CALCULATIONS ARE REQUIRED. IF THE FIELD HAUNCH EXCEEDS THE MAXIMUMS AND MINIMUMS INDICATED IN THE MISC. DATA TABLE, ADJUSTMENTS TO THE GRADE OR ADDITIONAL HAUNCH REINFORCEMENT WILL BE REQUIRED.

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FIELD HAUNCHES ARE DETERMINED USING SURVEYED TOP OF GIRDER TOP FLANGE ELEVATIONS AND "GIRDER LINE HAUNCH ELEVATION" DATA. ALLOWABLE MAXIMUM AND MINIMUM "FIELD HAUNCH" VALUES ARE GIVEN IN THE "MISCELLANEOUS DATA" TABLE. "CROSS SLOPE ADJUSTMENT" VALUES WILL AID THE CONTRACTOR IN DETERMINING ACTUAL FORMED HAUNCH DIMENSIONS AT THE EDGES OF THE TOP FLANGE.

DOWNWARD DEFLECTIONS ARE POSITIVE. FOR "HAUNCH LOCATIONS" DIAGRAM, SEE DESIGN SHEET 29.



FIELD HAUNCH DETAIL

DESIGN FOR 0° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

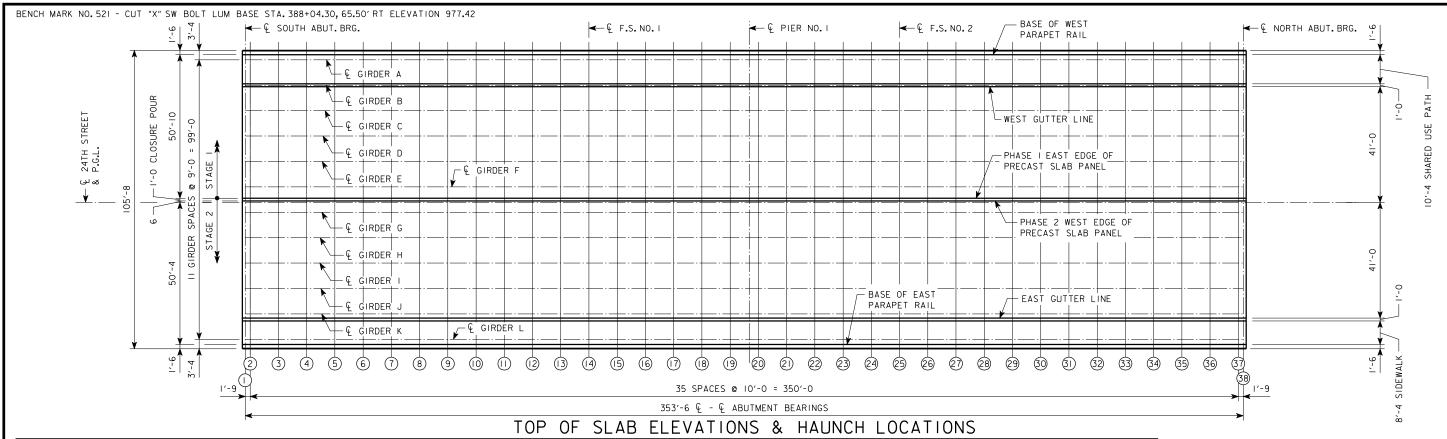
FIELD HAUNCH DATA DETAIL STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80) JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 28 OF 62 FILE NO. 30169 DESIGN NO. 508

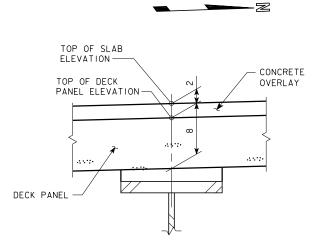
DESIGN TEAM RRP/JPS/DHS

POTTAWATTAMIE COUNTY | PROJECT NUMBER IM-080-1(308)2--13-78



								<u> </u>				<u> </u>			, , , , , ,				
					TABI	LE OF	TOP	OF S	SLAB	ELEVA	TION	S							
LINE		2	3	4	5	6	7	8	9	(10)	(1)	(12)	(3)	(14)	(15)	(6)	(7)	(18)	19
BASE OF WEST PARAPET RAIL*	1001.54	1001.57	1001.76	1001.93	1002.08	1002.23	1002.36	1002.47	1002.58	1002.67	1002.75	1002.81	1002.86	1002.90	1002.92	1002.94	1002.93	1002.92	1002.89
GIRDER LINE A*	1001.57	1001.61	1001.79	1001.96	1002.12	1002.26	1002.39	1002.51	1002.61	1002.70	1002.78	1002.85	1002.90	1002.94	1002.96	1002.97	1002.97	1002.95	1002.93
GIRDER LINE B*	1001.75	1001.79	1001.97	1002.14	1002.30	1002.44	1002.57	1002.69	1002.79	1002.88	1002.96	1003.02	1003.07	1003.11	1003.14	1003.15	1003.15	1003.13	1003.10
WEST GUTTER LINE	1001.93	1001.96	1002.15	1002.32	1002.47	1002.62	1002.75	1002.86	1002.97	1003.06	1003.14	1003.20	1003.25	1003.29	1003.31	1003.33	1003.32	1003.31	1003.28
GIRDER LINE C	1002.10	1002.13	1002.32	1002.49	1002.64	1002.79	1002.92	1003.04	1003.14	1003.23	1003.31	1003.37	1003.42	1003.46	1003.49	1003.50	1003.50	1003.48	1003.45
GIRDER LINE D	1002.28	1002.31	1002.49	1002.66	1002.82	1002.97	1003.10	1003.21	1003.32	1003.41	1003.49	1003.55	1003.60	1003.64	1003.66	1003.67	1003.67	1003.66	1003.63
GIRDER LINE E	1002.46	1002.49	1002.68	1002.85	1003.00	1003.15	1003.28	1003.40	1003.50	1003.59	1003.67	1003.73	1003.78	1003.82	1003.85	1003.86	1003.86	1003.84	1003.81
GIRDER LINE F	1002.64	1002.67	1002.85	1003.02	1003.18	1003.32	1003.46	1003.57	1003.68	1003.77	1003.84	1003.91	1003.96	1004.00	1004.02	1004.03	1004.03	1004.02	1003.99
PHASE I EAST EDGE OF SLAB PANEL	1002.71	1002.74	1002.93	1003.10	1003.25	1003.40	1003.53	1003.65	1003.75	1003.84	1003.92	1003.98	1004.03	1004.07	1004.10	1004.11	1004.11	1004.09	1004.06
PHASE 2 WEST EDGE OF SLAB PANEL	1002.72	1002.75	1002.94	1003.11	1003.26	1003.41	1003.54	1003.66	1003.76	1003.85	1003.93	1003.99	1004.04	1004.08	1004.11	1004.12	1004.12	1004.10	1004.07
P.G.L. 24TH STREET	1002.72	1002.75	1002.94	1003.11	1003.26	1003.41	1003.54	1003.66	1003.76	1003.85	1003.93	1003.99	1004.04	1004.08	1004.11	1004.12	1004.12	1004.10	1004.07
GIRDER LINE G	1002.68	1002.71	1002.90	1003.07	1003.22	1003.37	1003.50	1003.61	1003.72	1003.81	1003.89	1003.95	1004.00	1004.04	1004.06	1004.08	1004.07	1004.06	1004.03
GIRDER LINE H	1002.50	1002.54	1002.72	1002.89	1003.05	1003.19	1003.32	1003.44	1003.54	1003.63	1003.71	1003.77	1003.82	1003.86	1003.89	1003.90	1003.90	1003.88	1003.85
GIRDER LINE I	1002.32	1002.35	1002.54	1002.71	1002.86	1003.01	1003.14	1003.25	1003.36	1003.45	1003.53	1003.59	1003.64	1003.68	1003.71	1003.72	1003.71	1003.70	1003.67
GIRDER LINE J	1002.14	1002.17	1002.35	1002.52	1002.68	1002.82	1002.96	1003.07	1003.18	1003.27	1003.34	1003.41	1003.46	1003.50	1003.52	1003.53	1003.53	1003.52	1003.49
GIRDER LINE K	1001.96	1001.99	1002.18	1002.35	1002.50	1002.65	1002.78	1002.90	1003.00	1003.09	1003.17	1003.23	1003.28	1003.32	1003.35	1003.36	1003.36	1003.34	1003.31
EAST GUTTER LINE	1001.93	1001.96	1002.15	1002.32	1002.47	1002.62	1002.75	1002.86	1002.97	1003.06	1003.14	1003.20	1003.25	1003.29	1003.31	1003.33	1003.32	1003.31	1003.28
GIRDER LINE L*	1001.61	1001.64	1001.83	1002.00	1002.16	1002.30	1002.43	1002.55	1002.65	1002.74	1002.82	1002.88	1002.93	1002.97	1003.00	1003.01	1003.01	1002.99	1002.96
BASE OF EAST PARAPET RAIL*	1001.57	1001.61	1001.79	1001.96	1002.12	1002.26	1002.39	1002.51	1002.61	1002.70	1002.78	1002.85	1002.90	1002.94	1002.96	1002.97	1002.97	1002.95	1002.93

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					TABL	_E OF	TOP	OF S	LAB	ELEVA	COLLY	S							
LINE	20	21	22	23	24)	25	26	27	28	29	30	31	32	33	34)	35	36	37	38
BASE OF WEST PARAPET RAIL*	1002.85	1002.79	1002.73	1002.65	1002.55	1002.44	1002.32	1002.19	1002.04	1001.88	1001.71	1001.52	1001.32	1001.11	1000.88	1000.64	1000.39	1000.12	1000.07
GIRDER LINE A*	1002.89	1002.83	1002.76	1002.68	1002.59	1002.48	1002.36	1002.22	1002.08	1001.92	1001.74	1001.56	1001.36	1001.14	1000.92	1000.68	1000.42	1000.16	1000.11
GIRDER LINE B*	1003.06	1003.01	1002.94	1002.86	1002.76	1002.66	1002.54	1002.40	1002.25	1002.09	1001.92	1001.73	1001.53	1001.32	1001.09	1000.85	1000.60	1000.33	1000.29
WEST GUTTER LINE	1003.24	1003.18	1003.12	1003.04	1002.94	1002.83	1002.71	1002.58	1002.43	1002.27	1002.10	1001.91	1001.71	1001.50	1001.27	1001.03	1000.78	1000.51	1000.46
GIRDER LINE C	1003.41	1003.36	1003.29	1003.21	1003.11	1003.01	1002.88	1002.75	1002.60	1002.44	1002.27	1002.08	1001.88	1001.67	1001.44	1001.20	1000.95	1000.68	1000.64
GIRDER LINE D	1003.59	1003.53	1003.47	1003.38	1003.29	1003.18	1003.06	1002.93	1002.78	1002.62	1002.45	1002.26	1002.06	1001.85	1001.62	1001.38	1001.13	1000.86	1000.81
GIRDER LINE E	1003.77	1003.72	1003.65	1003.57	1003.47	1003.36	1003.24	1003.11	1002.96	1002.80	1002.63	1002.44	1002.24	1002.03	1001.80	1001.56	1001.31	1001.04	1000.99
GIRDER LINE F	1003.95	1003.89	1003.83	1003.74	1003.65	1003.54	1003.42	1003.29	1003.14	1002.98	1002.81	1002.62	1002.42	1002.21	1001.98	1001.74	1001.49	1001.22	1001.17
PHASE I EAST EDGE OF SLAB PANEL	1004.02	1003.97	1003.90	1003.82	1003.72	1003.61	1003.49	1003.36	1003.21	1003.05	1002.88	1002.69	1002.49	1002.28	1002.05	1001.81	1001.56	1001.29	1001.24
PHASE 2 WEST EDGE OF SLAB PANEL	1004.03	1003.98	1003.91	1003.83	1003.73	1003.63	1003.50	1003.37	1003.22	1003.06	1002.89	1002.70	1002.50	1002.29	1002.06	1001.82	1001.57	1001.30	1001.25
P.G.L. 24TH STREET	1004.03	1003.98	1003.91	1003.83	1003.73	1003.63	1003.50	1003.37	1003.22	1003.06	1002.89	1002.70	1002.50	1002.29	1002.06	1001.82	1001.57	1001.30	1001.25
GIRDER LINE G	1003.99	1003.93	1003.87	1003.79	1003.69	1003.58	1003.46	1003.33	1003.18	1003.02	1002.85	1002.66	1002.46	1002.25	1002.02	1001.78	1001.53	1001.26	1001.21
GIRDER LINE H	1003.81	1003.76	1003.69	1003.61	1003.51	1003.41	1003.29	1003.15	1003.00	1002.84	1002.67	1002.48	1002.28	1002.07	1001.84	1001.60	1001.35	1001.08	1001.04
GIRDER LINE I	1003.63	1003.58	1003.51	1003.43	1003.33	1003.22	1003.10	1002.97	1002.82	1002.66	1002.49	1002.30	1002.10	1001.89	1001.66	1001.42	1001.17	1000.90	1000.85
GIRDER LINE J	1003.45	1003.39	1003.33	1003.24	1003.15	1003.04	1002.92	1002.79	1002.64	1002.48	1002.31	1002.12	1001.92	1001.71	1001.48	1001.24	1000.99	1000.72	1000.67
GIRDER LINE K	1003.27	1003.22	1003.15	1003.07	1002.97	1002.86	1002.74	1002.61	1002.46	1002.30	1002.13	1001.94	1001.74	1001.53	1001.30	1001.06	1000.81	1000.54	1000.49
EAST GUTTER LINE	1003.24	1003.18	1003.12	1003.04	1002.94	1002.83	1002.71	1002.58	1002.43	1002.27	1002.10	1001.91	1001.71	1001.50	1001.27	1001.03	1000.78	1000.51	1000.46
GIRDER LINE L*	1002.92	1002.87	1002.80	1002.72	1002.62	1002.52	1002.40	1002.26	1002.11	1001.95	1001.78	1001.59	1001.39	1001.18	1000.95	1000.71	1000.46	1000.19	1000.15
BASE OF EAST PARAPET RAIL*	1002.89	1002.83	1002.76	1002.68	1002.59	1002.48	1002.36	1002.22	1002.08	1001.92	1001.74	1001.56	1001.36	1001.14	1000.92	1000.68	1000.42	1000.16	1000.11



TOP OF SLAB ELEVATION DETAIL

THE ELEVATIONS IN THE "TABLE OF TOP OF SLAB ELEVATIONS" ARE TYPICALLY GIVEN AT THE TOP OF THE 2" CONCRETE OVERLAY AS SHOWN IN THE DETAIL. FOR LOCATIONS OUTSIDE OF THE 2" CONCRETE OVERLAY, THE ELEVATIONS ARE GIVEN AT THE TOP OF THE PRECAST SLAB PANEL AND ARE MARKED WITH AN ASTERISK() IN THE TABLE.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80
178'-6 & 175'-0 SPANS
TOP OF SLAB ELEVATIONS
STA. 40176+95.25 (24TH STREET)
STA. 7476+95.25 (FUTURE 1-80) JUNE, 2007

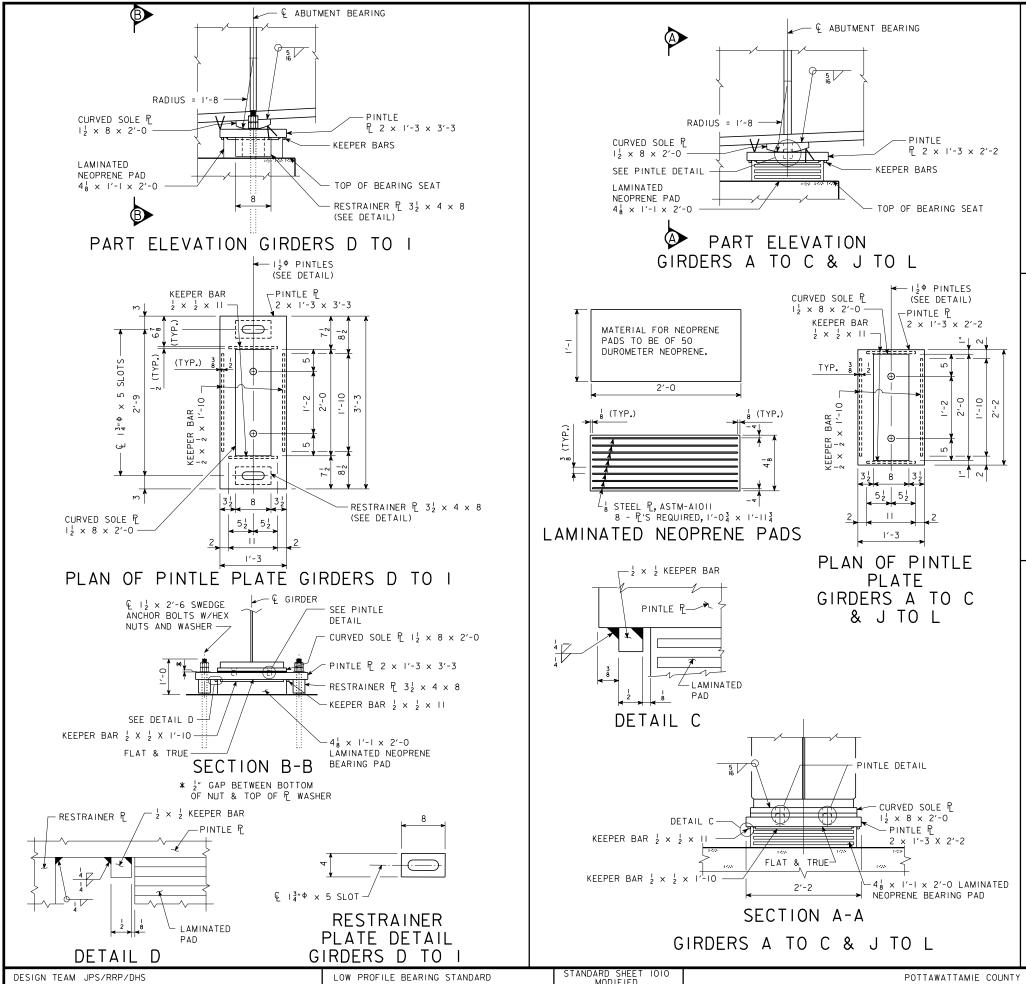
POTTAWATTAMIE COUNTY

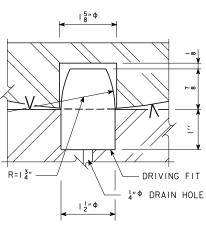
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 29 OF 62 FILE NO. 30169 DESIGN NO. 508

POTTAWATTAMIE COUNTY | PROJECT NUMBER IM-080-1(308)2--13-78

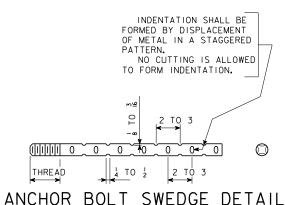
SHEET NUMBER 30

DESIGN TEAM RRP/JPS/ACB





PINTLE DETAIL



BEARING NOTES:

SURFACES MARKED "V" SHALL BE FINISHED ANSI 250.

PINTLE PLATES AND SOLE PLATES ARE A PART OF THE SUPERSTRUCTURE STRUCTURAL STEEL QUANTITY. UNIT PRICE BID FOR "STRUCTURAL STEEL" SHALL INCLUDE ALLOWANCE FOR COST OF THE NEOPRENE BEARING PADS.

THE PINTLE PLATES, KEEPER BARS, AND RESTRAINER PLATES SHALL BE GALVANIZED. WELDING SHALL BE COMPLETED PRIOR TO GALVANIZING. THE SURFACES OF THE PINTLE PLATE IN CONTACT WITH THE CURVED SOLE PLATE AND THE LAMINATED NEOPRENE PAD SHALL BE FREE OF PROJECTIONS DUE TO GALVANIZING.

CURVED SOLE PLATES SHALL COMPLY WITH ASTM A 709 GRADE 50W. KEEPER BARS, PINTLE PLATES AND RESTAINER PLATES SHALL COMPLY WITH ASTM A 709 GRADE 50.

ANCHOR BOLTS, NUTS AND WASHERS SHALL MEET THE REQUIREMENTS OF IM 453.08.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

ABUTMENT BEARING DETAILS

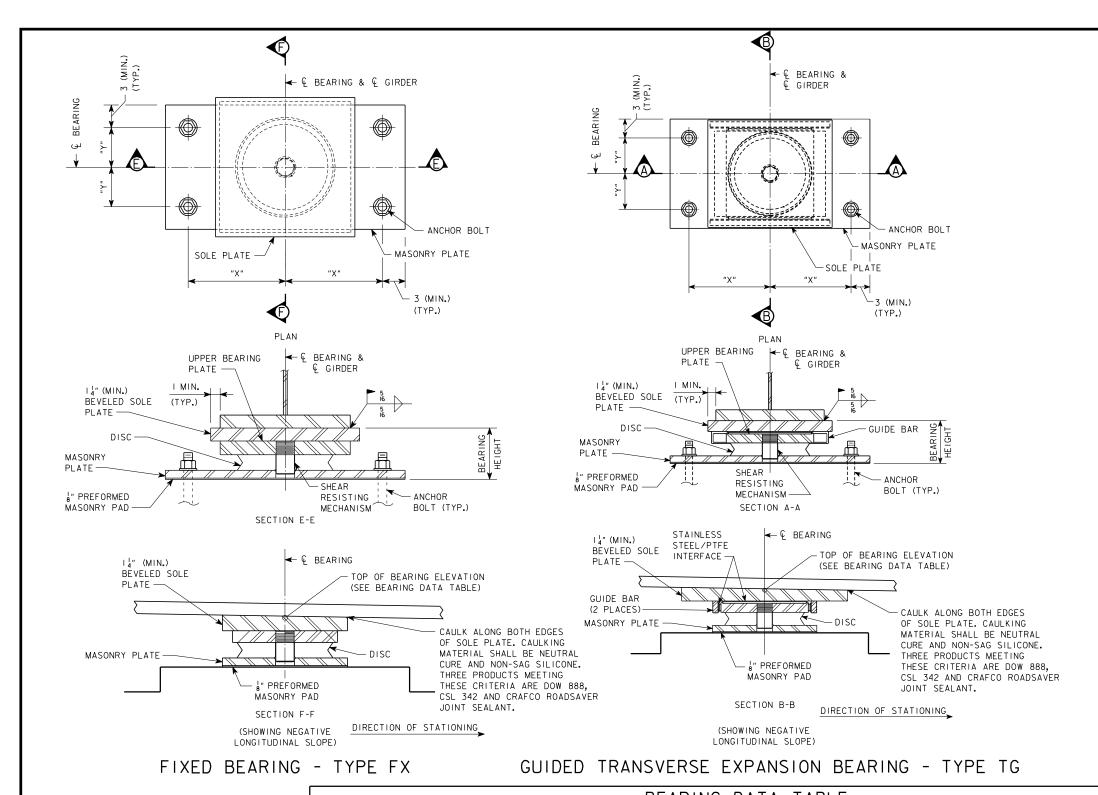
STA. 40176+95.25 (24TH STREET STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 30 OF 62 FILE NO. 30169 DESIGN NO. 508

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POTTAWATTAMIE COUNTY | PROJECT NUMBER IM-080-1(308)2--13-78



997.21

997.03

996.85

BEARING DATA TABLE LONGITUDINAL BEARING VERTICAL LOAD HORIZONTAL TOTAL ANCHOR BOLTS TOP OF BEARING LOCATION TYPE GIRDER SLOPE OF SOLE HEIGHT MAX. LONGITUDINAL DIM. "X" DIM. "Y" NUMBER AND SIZE MIN. ELEVATION PLATE (%) (IN) (KIPS) (KIPS) MOVEMENT (IN) (EACH BEARING) (IN) (IN) 996.81 TG 996.99 997.17 997.35 997.53 997.71 PIFR NO. I -0.4378 10 670 471 33 0 20 4 - 11" DIA. 9 997.75 997.57 997.39

DISC BEARING NOTES:

THE BEARING DEVICES, INCLUDING SOLE PLATES AND MASONRY PLATES, SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE 17TH EDITION OF THE AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", INCLUDING CURRENT INTERIM SPECIFICATIONS.

THE MASONRY PLATES SHALL BE DESIGNED FOR AN ALLOWABLE BEARING STRESS AS SPECIFIED IN AASHTO, SECTION 8.15.2.1.3.

THE SOLE PLATES SHALL BE TAPERED TO THE LONGITUDINAL SLOPE SHOWN, AND SHALL BE SIZED FOR THE MOVEMENTS SHOWN IN THE BEARING DATA TABLE. ADDITIONALLY, THE SOLE PLATES SHALL BE 2" WIDER THAN THE GIRDER BOTTOM FLANGE TO ALLOW THE PLACEMENT OF A HORIZONTAL WELD.

THE BEARINGS SHALL BE DESIGNED FOR THE LOADS AND MOVEMENTS SHOWN IN THE BEARING DATA TABLE OCCURRING SIMULTANEOUSLY, ALL LOADS SHOWN ARE SERVICE LOADS, MINIMUM VERTICAL LOADS SHOWN ARE DUE TO MINIMUM D.L. AND L.L. WITH IMPACT CAUSING UPLIFT, MAXIMUM VERTICAL LOADS SHOWN ARE DUE TO D.L., S.D.L. AND L.L. WITH IMPACT. NO INCREASE IN ALLOWABLE WORKING STRESSES IS ALLOWED.

BEARINGS SHALL BE DESIGNED TO ACCOMMODATE A ROTATION OF 0.02 RADIANS. ALL BEARINGS SHALL BE FULLY REMOVABLE.

FOR GUIDED EXPANSION BEARINGS, STAINLESS STEEL SURFACES SHALL EXTEND A MINIMUM OF I" EACH WAY BEYOND THE SPECIFIED MOVEMENT RANGE. WHERE VALUES OF MOVEMENT ARE NOT SPECIFIED, STAINLESS STEEL SURFACES SHALL EXTEND I" MINIMUM BEYOND THE LOWER ASSEMBLY CONTACT SURFACES.

TOTAL MOVEMENTS SHOWN IN THE BEARING DATA TABLE REPRESENT THE COMBINED MOVEMENT RANGE FOR BRIDGE EXPANSION (50° F. TO 125° F.) AND BRIDGE CONTRACTION (50° F. TO -25° F.).

ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION IN THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER THE BEARING IS INSTALLED. THE MARKS SHALL BE ON THE TOP PLATE OF THE BEARING. ALL BEARINGS SHALL HAVE A MAXIMUM FRICTION COEFFICIENT OF 3%.

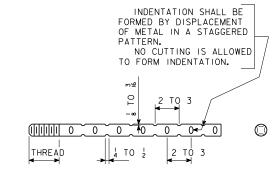
THE GAP BETWEEN THE GUIDE BARS AND THE BEARINGS SHALL BE $^{\rm l}_8$ ". STEEL COMPONENTS OF BEARING ASSEMBLIES SHALL BE OF ASTM A709M GRADE 50W STEEL.

THE BEARING HEIGHT NOTED IN THE BEARING DATA TABLE REPRESENTS THE ASSUMED TOTAL HEIGHT OF THE BEARING ASSEMBLY PLUS THE &"PREFORMED MASONRY PAD. THIS HEIGHT WAS USED BY THE DESIGNER TO ESTABLISH THE PEDESTAL ELEVATIONS AS NOTED ON PIER AND ABUTMENT DETAIL SHEETS. THE MINIMUM PEDESTAL HEIGHT SHALL NOT BE CHANGED WITHOUT WRITTEN APPROVAL OF THE ENGINEER. THE ACTUAL BEARING HEIGHT DETERMINED BY THE BEARING MANUFACTURER SHALL BE USED TO SET THE TOP OF PEDESTAL ELEVATIONS TO ACHIEVE THE PROPER TOP OF BEARING ELEVATIONS GIVEN IN THE BEARING DATA TABLE. THE TOP OF PEDESTAL ELEVATIONS SHALL BE SHOWN ON THE SHOP DRAWINGS. IN ORDER TO COORDINATE TOP OF PEDESTAL ELEVATIONS AND ANCHOR BOLT

IN ORDER TO COORDINATE TOP OF PEDESTAL ELEVATIONS AND ANCHOR BOLT LOCATIONS, PIERS AND ABUTMENTS SHALL NOT BE POURED PRIOR TO RECEIVING APPROVED BEARING SHOP DRAWINGS FOR THIS CONTRACT.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF IM 453.08. ANCHOR BOLT LAYOUT SHOWN IN THE DETAILS IS BASED ON PRELIMINARY BEARING DESIGN. THE ANCHOR BOLT LAYOUT WAS USED IN SETTING THE GEOMETRY OF THE PIER AND ABUTMENT REINFORCING WHICH SHOULD ALLOW THE ANCHOR BOLTS TO BE INSTALLED WITHOUT CONFLICT WITH THE REINFORCING. ANY CHANGES TO THE ANCHOR BOLT PATTERN MAY REQUIRE A PLAN CHANGE TO THE REINFORCING LAYOUT.

ANCHOR BOLTS SHALL BE EMBEDDED IN CONCRETE A MINIMUM OF 1'-6.
FABRICATOR SHALL DETERMINE REQUIRED ANCHOR BOLT LENGTH BASED ON BEARING
DETAILS AND REQUIRED ANCHOR BOLT EMBEDMENT. SHOP DRAWINGS SHALL SHOW
ANCHOR BOLT EMBEDMENT, PROJECTION, THREAD LENGTH, AND TOTAL BOLT LENGTH.



ANCHOR BOLT SWEDGE DETAIL

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

DISC BEARING DETAILS

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 31 OF 62 FILE NO. 30169 DESIGN NO. 508

HR

HDR Engineering, Inc.

DESIGN TEAM JPS/RRP/DHS

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-I(308)2--I3-78

SHEET NUMBER 32

TG

PRECAST SLAB PANEL CONSTRUCTION SEQUENCE

THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL A DETAILED CONSTRUCTION SEQUENCE OF WORK TASKS TO BE PERFORMED BEFORE REMOVAL OF EXISTING STRUCTURE. DETAIL WORK TASK SEQUENCE SHALL INCLUDE THE INTENDED METHOD FOR FORMING THE GIRDER HAUNCHES & QUALITY CONTROL CONSTRUCTION METHOD FOR OBTAINING THE PROPER ALIGNMENT AND GRADE FOR THE PRECAST SLAB PANELS. THE PLANS HAVE BEEN DEVELOPED ASSUMING THE FOLLOWING CONSTRUCTION SEQUENCE, FOR EACH PHASE:

I) ERECT ALL OF THE GIRDERS IN THE ACTIVE CONSTRUCTION PHASE FOR THE ENTIRE LENGTH OF THE BRIDGE INCLUDING ALL CROSS FRAMES.

2) FORM THE GIRDER HAUNCHES. NOTE: ALL PANELS FOR EACH PHASE SHALL BE ERECTED AND THE PANELS SHALL BE LONGITUDINALLY POST-TENSIONED AND ACCEPTED BY THE ENGINEER PRIOR TO PLACING CONCRETE FOR HALINCHES (SEE STEP 9 BELOW).

3) ERECT ALL OF THE PRECAST SLAB PANELS AS SHOWN IN THE PRECAST SLAB PANEL LAYOUT SHEET. CARE SHOULD BE TAKEN TO ENSURE THE PRECAST SLAB PANELS ARE IN TIGHT CONTACT WITH THE BACKER ROD SEPARATING THEM AND PROPER ALIGNMENT IS ACHIEVED. USE LEVELING BOLTS, OR OTHER APPROVED METHODS, TO ACHIEVE THE REQUIRED GRADE. AT NO TIME WILL CONSTRUCTION EQUIPMENT BE ALLOWED ON THE PRECAST SLAB PANELS UNTIL CONSTRUCTION OF THE PRECAST SLAB IS COMPLETE AND THE HAUNCHES AND KEYWAYS HAVE ACHIEVED A MINIMUM COMPRESSIVE STRENGTH OF 6000 PSI. IF THE OPTIONAL LEVELING BOLTS ARE USED TO PROPERLY SET THE PRECAST SLAB PANELS TO GRADE, THEN THE CONTRACTOR SHALL ENSURE ALL BOLTS ARE IN CONTACT WITH THE TOP FLANGE BEFORE THE PRECAST SLAB PANELS ARE RELEASED FROM THE ERECTING CRANE AND THE PRECAST SLAB PANELS ARE SOLEY SUPPORTED BY ALL THE LEVELING BOLTS.

4) INSTALL SHEAR STUDS. THE INSTALLATION OF SHEAR STUDS CAN COMMENCE AFTER AT LEAST ONE-THIRD OF THE PANELS FOR A PHASE HAVE BEEN ERECTED AND THE CONTRACTOR IS ASSURED THAT ERECTION TOLERANCES AND PROPER GRADE HAVE BEEN ATTAINED AND APPROVED BY THE ENGINEER.

5) JOIN DUCTS FOR POST-TENSIONING TENDONS AT ALL TRANSVERSE JOINTS. IT IS SUGGESTED THAT THE DUCT SPLICE BE ATTACHED TO THE DUCTS PROTRUDING OUT OF THE PANELS BEFORE THE NEXT SUCCESSIVE PANELS ARE ERECTED.

6) FILL THE TRANSVERSE JOINTS WITH NON-SHRINK GROUT LEVEL WITH THE TOPS OF THE PRECAST SLAB PANELS. ALLOW THE GROUT TO ATTAIN A COMPRESSIVE STRENGTH OF 6000 PSI BEFORE PROGRESSING.

7) INSTALL THE 0.6" POST-TENSIONING STRANDS THROUGH THE POST-TENSIONING DUCTS AND ANCHORAGE SYSTEMS.

8) BEGINNING AT EITHER END OF THE PRECAST SLAB, TENSION THE STRANDS IN EACH POST-TENSIONING DUCT, TO THE SPECIFIED FORCE AND AS PER THE SEQUENCE AS SPECIFIED IN THE APPROVED SHOP

9) FILL ALL SHEAR STUD POCKETS IN THE PRECAST SLAB PANELS AND HAUNCHES WITH THE SPECIFIED CONCRETE MIX.

IO) CONSTRUCT THE REMAINING CAST-IN-PLACE FND SECTIONS AND FND DAMS.

II) PRIOR TO CONSTRUCTING OVERLAY, PLACE PLASTIC SCREW CAPS FOR MECHANICAL ANCHORS FOR MEDIAN AS DETAILED ON DESIGN SHEET 38. PLACE BARRIER RAIL, SIDEWALK, SEPARATION RAIL AND 2" CONCRETE OVERLAY.

12) REPEAT STEPS | THROUGH | 12 FOR PHASE 2.

13) CONSTRUCT FULL DEPTH CLOSURE POUR. CONCRETE PLACEMENT FOR CLOSURE POUR SHALL BE PERFORMED WITH 24th STREET CLOSED TO TRAFFIC.

14) CONSTRUCT CONCRETE MEDIAN.

PRECAST SLAB PANEL NOTES:

FABRICATOR SHALL BE RESPONSIBLE FOR EXERCISING CARE IN LIFTING. HANDLING. STORING, AND TRANSPORTATION OF THE PRECAST SLAB PANELS TO PREVENT CRACKING OR DAMAGE. PANELS SHALL BE LIFTED BY DEVICES AS SHOWN ON THE PLANS OR AS DESIGNED BY THE FABRICATOR AND APPROVED BY THE ENGINEER.

PRETENSIONING STRANDS FOR THE PRECAST SLAB PANELS SHALL BE UNCOATED, SEVEN-WIRE, LOW-RELAXATION STEEL STRAND OF 1" NOMINAL DIAMETER AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A416, GRADE 270. STRANDS SHALL BE TENSIONED TO THE LOADS SHOWN ON DESIGN SHEET 35 BEFORE RELEASE ALL METHODS EMPLOYED AND PROCEDURES TO BE FOLLOWED IN TENSIONING THE STRANDS SHALL BE SUBJECT TO THE ENGINEER'S APPROVAL. THE METHOD CHOSEN SHALL BE EXECUTED IN A MANNER TO ASSURE THAT BOTH ENDS OF ALL STRANDS IN THE PANEL ARE UNIFORMLY TENSIONED. THE PRESTRESSED STRAND SHALL BE RELEASE IN A MANNER THAT WILL MINIMIZE ECCENTRICITY.

POST-TENSIONING STRANDS SHALL BE UNCOATED, SEVEN-WIRE, LOW-RELAXATION STEEL STRAND OF 0.6" NOMINAL DIAMETER AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A416, GRADE 270. ALL METHODS EMPLOYED AND PROCEDURES TO BE FOLLOWED IN POST-TENSIONING THE STRANDS SHALL BE SUBJECT TO THE ENGINEER'S APPROVAL.

POST-TENSIONING PARAMETERS: MAXIMUM JACKING STRESS = 0.8 FU = 216 KSI MAXIMUM STRESS AT ANCHOR (SET) = 0.70 FU = 189 KSI ASSUMED ANCHOR SET = 0.25 IN FOUR STRANDS PER DUCT, JACKING FORCE PER STRAND = 41 KIPS ASSUMED FRICTION COEFFICIENT = 0.23

END ANCHORAGES FOR POST-TENSIONING SHALL BE SPRING LOADED CHUCKS TO BE APPROVED BY THE ENGINEER.

CONCRETE IN THE PRECAST SLAB PANELS SHALL HAVE A MIMIMUM CONCRETE STRENGTH OF 4000 PSI BEFORE RELEASE OF PRE-TENSIONING STRANDS. IN ORDER TO MINIMIZE THE EFFECTS OF LONG TERM LOSSES DUE TO CREEP AND SHRINKAGE, THE SLAB PANELS REQUIRE HIGHER CONCRETE STRENGTH FOR YOUNGER AGE PANELS BEFORE ANY BOND STRESSES FROM POST-TENSIONING ARE TRANSFERED TO THE CONCRETE AND ANCHORAGES. THEREFORE, TO PROVIDE FLEXIBILITY FOR FABRICATION AND CONSTRUCTION, THE CONCRETE STRENGTH OF THE PRECAST SLAB PANELS SHALL CONFORM TO THE TABLE AS SHOWN BELOW.

AGE OF PRECAST SLAB PANELS AT TIME OF POST-TENSIONING	MINIMUM f'c
28 DAYS	11,000 PSI
40 DAYS	10,000 PSI
70 DAYS	9,000 PSI
IOO DAYS	8,000 PSI

SPIRALS SHALL BE 4" DIAMETER HIGH CARBON SPRING WIRE AND CONFORM TO THE REQUIREMENTS OF ASTM A227.

TRANSVERSE STRANDS AT PANEL ENDS SHALL BE REMOVED TO A DEPTH OF I INCH INSIDE THE PANEL EDGE. THE RESULTING POCKET SHALL BE GROUTED WITH HIGH STRENGTH, NON-SHRINK

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM DESIGNATION A615 GRADE 60.

ALL SIDES OF THE PANEL, WITH CONCRETE PLACED AGAINST IT, SHALL BE ROUGHENED BY EITHER:

I) SAND BLASTING - PROTECTING EXPOSED SURFACES

2) "NEDDLE" GUN - TYPICALLY USED BY PRECAST MANUFACTURERS ON PPC BEAM ENDS.

THE TOP SURFACE OF THE PRECAST SLAB PANELS SHALL BE INTENTIONALLY ROUGHENED OR RAKED TO A MINIMUM DEPTH OF 4". THE ROUGHENING OF THE PRECAST SLAB PANELS SHALL BE DONE WITH A MECHANICAL DEVICE SUCH AS A WIRE BROOM OR A TINING RAKE, TINING CAN BE TRANSVERSE OR LONGITUDINAL. TEXTURE RAKE TINE SPACING SHALL BE EQUAL SPACES OF 12 INCHES OR UNEQUAL SPACES IN ACCORDANCE WITH ARTICLE 2412.06 OF THE STANDARD SPECIFICATIONS. THIS OPERATION SHALL BE DONE AT SUCH TIME AND IN SUCH A MANNER THAT THE DESIRED SURFACE TEXTURE WILL BE ACHIEVED WHILE MINIMIZING DISPLACEMENT OF THE LARGER AGGREGATE PARTICLES AND BEFORE THE SURFACES PERMANENTLY SETS. THIS OPERATION SHALL NOT DELAY THE PLACEMENT OF WET BURLAP WITHIN THE ALLOTTED TIME AS SPECIFIED BY THE APPROPRIATE SPECIFICATIONS.

ADDITIONAL REQUIREMENTS AS SPECIFIED IN SECTION 2413.04 OF THE STANDARD SPECIFICATIONS SHALL APPLY.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

PRECAST SLAB PANEL NOTES JUNE, 2007

STA. 40176+95.25 (24TH STREE STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 32 OF 62 FILE NO. 30169 DESIGN NO. 508

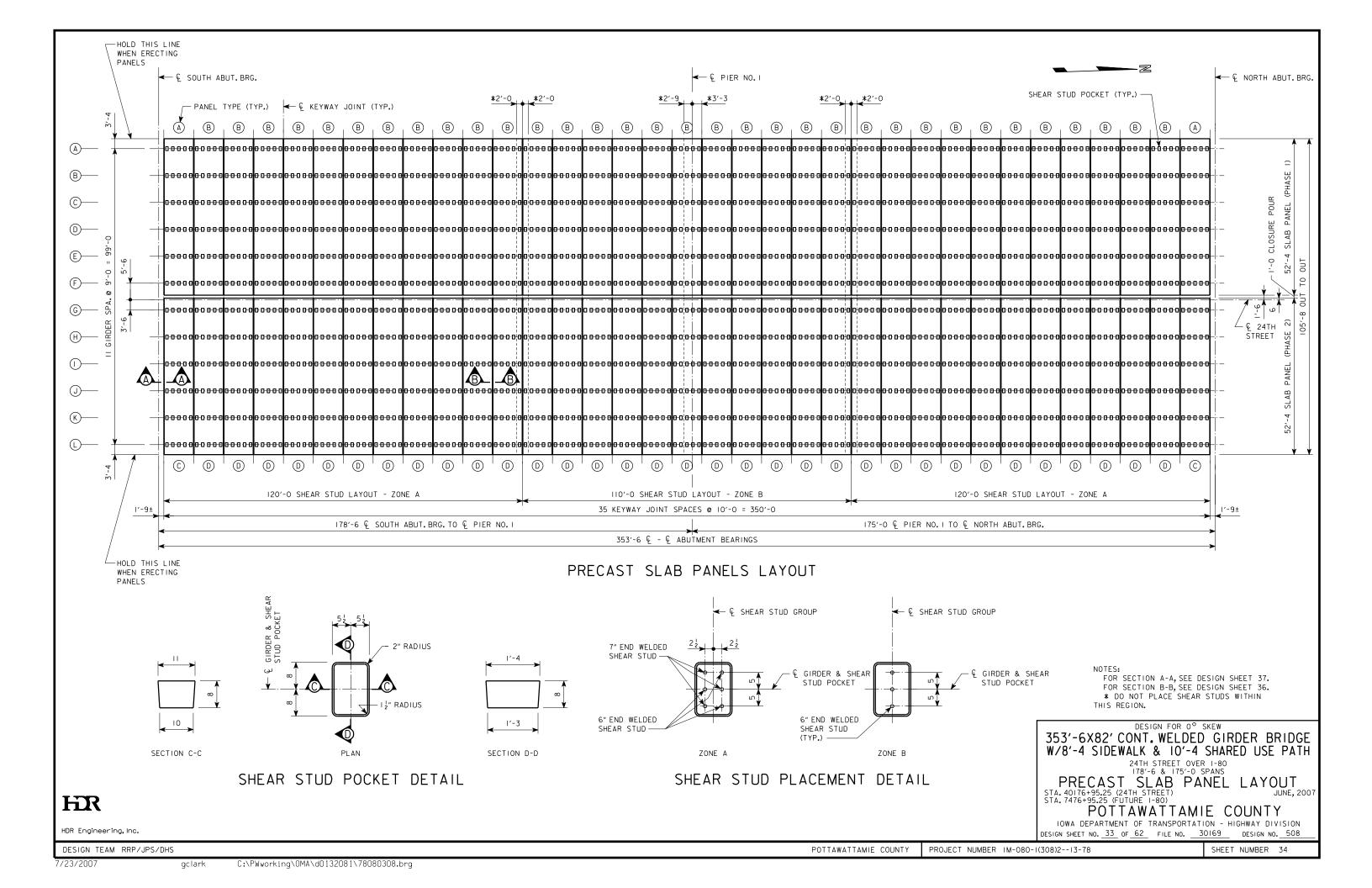
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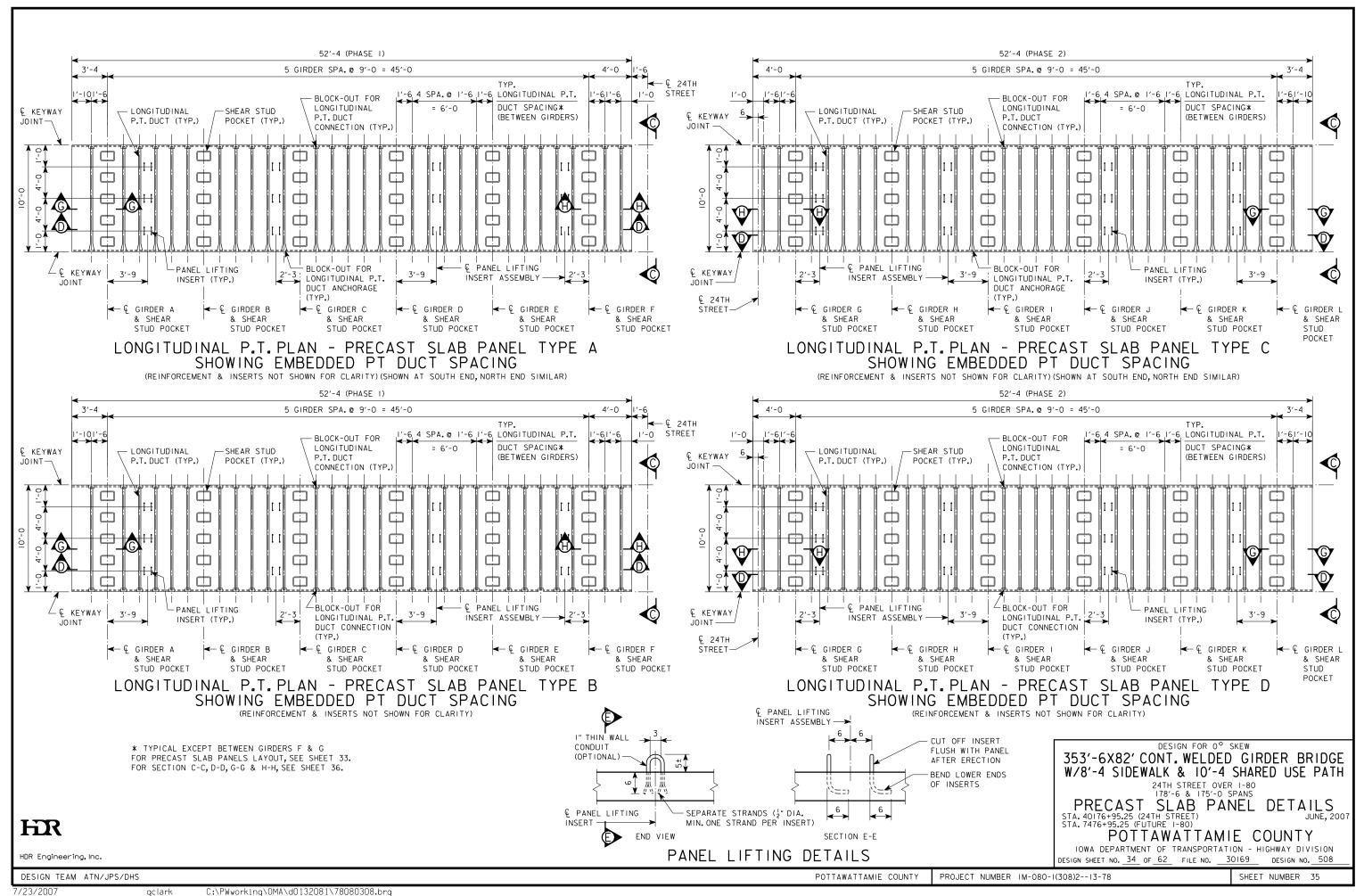
HDR Engineering, Inc.

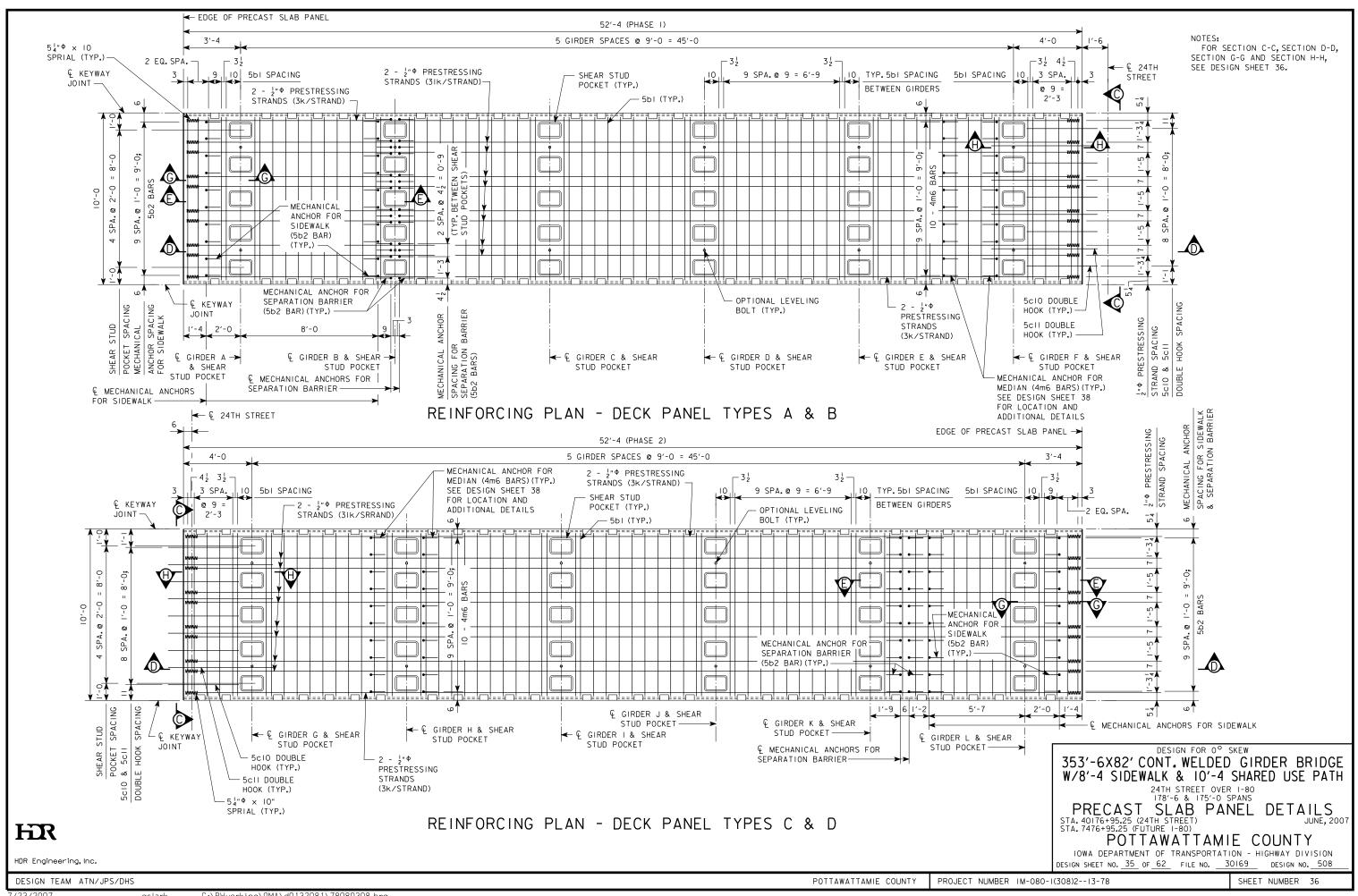
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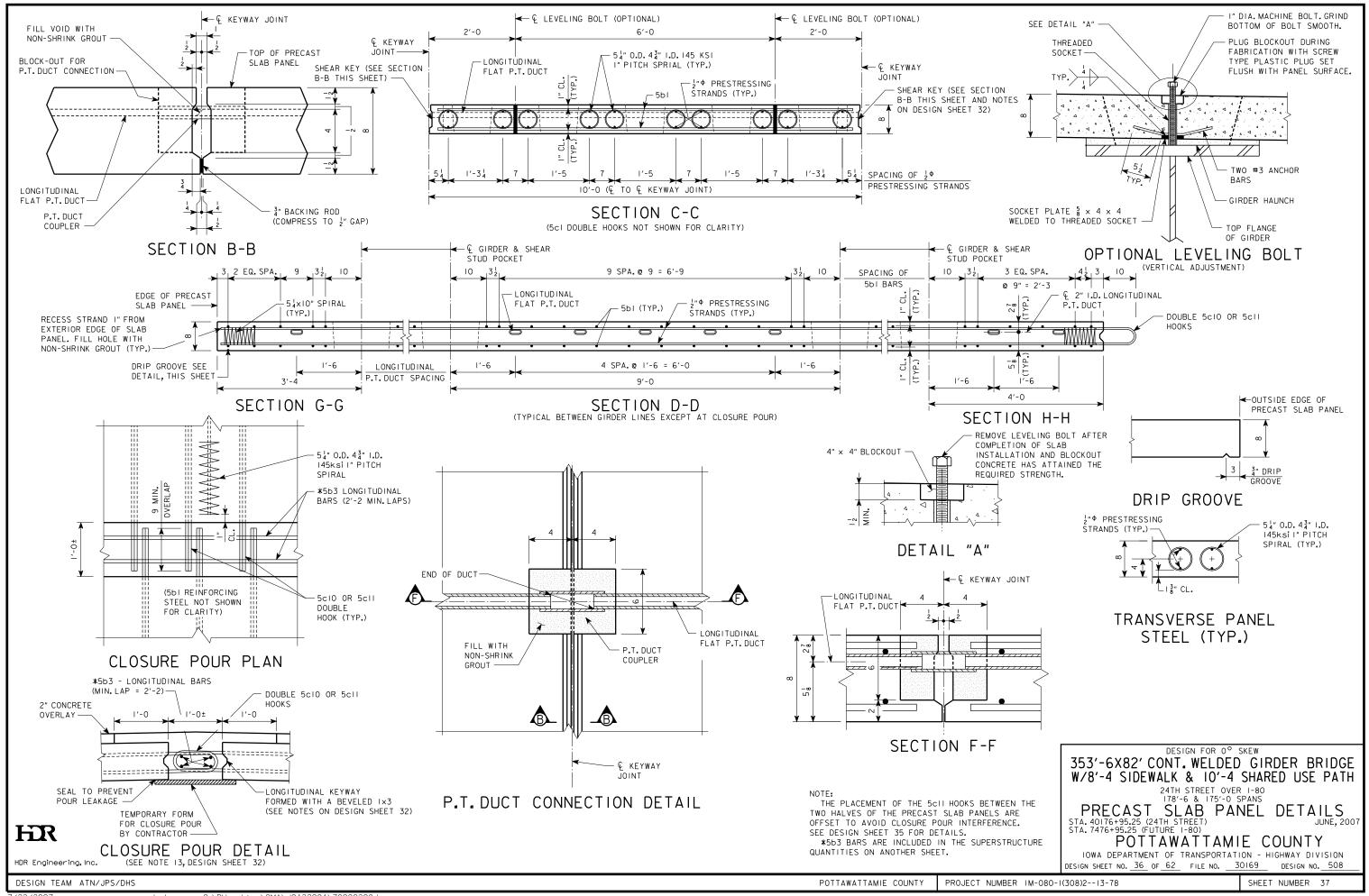
POTTAWATTAMIE COUNTY

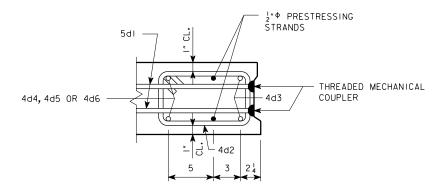
PROJECT NUMBER IM-080-1(308)2--13-78





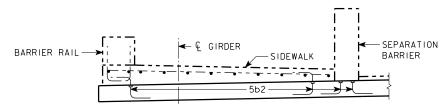






SECTION A-A

(5b) BARS AND POST-TENSION DUCT NOT SHOWN FOR CLARITY)



SECTION E-E

(SEPARATION BARRIER & SIDEWALK INSERT DETAIL)

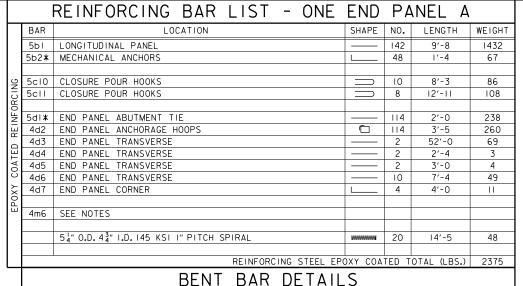
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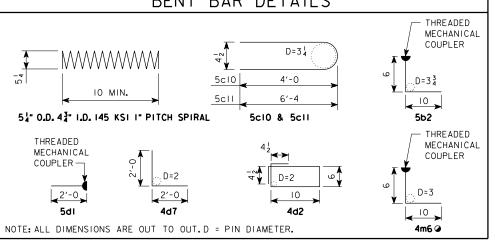
EACH PANEL CONTAINS 20 UNCOATED $\frac{1}{2}$ " DIA. PRESTRESSING STRANDS 270 KSI-LL.

* INCLUDES I THREADED MECHANICAL COUPLER.

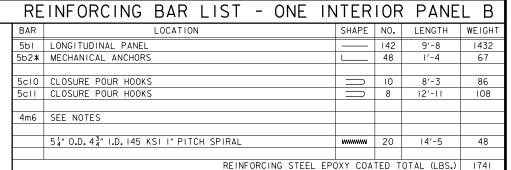
☑ THE TOTAL NUMBER OF 4m6 BARS (712 WEIGHING 634 LB.)
WAS COMPUTED BASED ON SPACING AS SHOWN ON DESIGN
SHEET 38. THE TOTAL NUMBER OF 4m6 BARS MAY VARY FROM
THE TOTAL NUMBER AS SHOWN ON THIS SHEET. HOWEVER
ANY ADDITIONAL BARS REQUIRED WILL BE CONSIDERED
SUBSIDIARY TO THE DECK PANEL.

MECHANICAL ANCHORS FOR SIDEWALK, SEPARATION BARRIER, MEDIAN AND CIP END SECTION TIE REINFORCING BARS ARE TO BE SPLICED AT THE LOCATION SHOWN USING A THREADED MECHANICAL COUPLER. THREE COUPLER ASSEMBLIES FOR THE #5 REINFORCING BARS ARE TO BE SUBMITTED FOR TESTING. THE COST OF ALL COUPLERS, INCLUDING THOSE TO BE SUBMITTED FOR TESTING, IS TO BE INCLUDED IN THE PRICE BID FOR "PRECAST POST-TENSIONED SLAB PANELS" AND NO ADDITIONAL PAYMENT WILL BE MADE. THE WEIGHT OF THE MECHANICAL COUPLERS IS NOT INCLUDED IN THE QUANTITY SHOWN IN THE REINFORCING BAR LIST.





PRECAST SLAB PANE	L CONC.QUANT.
HIGH PERFORMANCE CONCRETE	UNIT TOTAL
END PANEL A	CY 2 @ II.6 = 23.2
INTERIOR PANEL B	CY 33 @ II.6 = 382.8
END PANEL C	CY 2 @ 11.6 = 23.2
INTERIOR PANEL D	CY 33 @ II.6 = 382.8
T	OTAL (CY) 812.0



REINFORCING BAR LIST - ONE END PANEL C

						_
	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
	5b1	LONGITUDINAL PANEL		142	9′-8	1432
	5b2*	MECHANICAL ANCHORS		40	1′-4	56
9						
CII	5c10	CLOSURE POUR HOOKS		10	8′-3	86
REINFORCING	5cII	CLOSURE POUR HOOKS		8	12'-11	108
Ä						
₹E I	5dl*	END PANEL ABUTMENT TIE		114	2'-0	238
	4d2	END PANEL ANCHORAGE HOOPS		114	3′-5	260
COATED	4d3	END PANEL TRANSVERSE		2	52′-0	69
0 A	4d4	END PANEL TRANSVERSE		2	2'-4	3
	4d5	END PANEL TRANSVERSE		2	3′-0	4
ΧY	4d6	END PANEL TRANSVERSE		10	7′-4	49
ЕРОХҮ	4d7	END PANEL CORNER		4	4'-0	H
Ш						
	4m6	SEE NOTES				
		54" O.D. 44" I.D. 145 KSI I" PITCH SPIRAL	wwwwi	20	14'-5	48

REINFORCING STEEL EPOXY COATED TOTAL (LBS.) 2364

RE	INFORCING BAR LIST - ONE I	NTER	IOR	PANE	L D
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
5b1	LONGITUDINAL PANEL	—	142	9′-8	1432
5b2*	MECHANICAL ANCHORS	L	40	1'-4	56
5c10	CLOSURE POUR HOOKS		10	8′-3	86
5cII	CLOSURE POUR HOOKS		8	12'-11	108
4m6	SEE NOTES				
	5¼" O.D. 4¾" I.D. 145 KSI I" PITCH SPIRAL	wwwww	20	14′-5	48
	REINFORCING STEEL F	POXY COA	TFD T	OTAL (LBS.)	1730

EDGE OF PRECAST SLAB PANEL -> 4'-0 9'-0 3'-4 2 - 5dl (TYP.) 2 - 4d52 - 4d6 BETWEEN 2 - 4d7 AT CORNER ALIGN WITH 2-4d4 ___ 2-4d7 -GIRDERS (TYP.) OF PRECAST SLAB 4d2 BAR TYP. PANEL Shakabe 1'-6 TYP. 4d2 2 - 4d3-(TYP.)-€ LONGITUDINAL BAR SPACING THREADED MECHANICAL C LONGITUDINAL € LONGITUDINAL P.T. DUCT -> AT P.T. DUCT COUPLER (TYP.) P.T. DUCT (TYP.) P.T. DUCT (TYP.) - 2 - ½" ♥ PRESTRESSING ANCHORAGE STRANDS, SEE DESIGN SHEET 35 <- € GIRDER & SHEAR ← Ç GIRDER & SHEAR < ← € GIRDER & SHEAR STUD POCKET FOR ADDITIONAL DETAILS. STUD POCKET STUD POCKET HR

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

PRECAST SLAB PANEL DETAILS
STA. 40176+95.25 (24TH STREET)
STA. 7476+95.25 (FUTURE 1-80)

DETAILS
JUNE, 200

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 37 OF 62 FILE NO. 30169 DESIGN NO. 508

ADDITIONAL END PANEL REINFORCING DETAIL

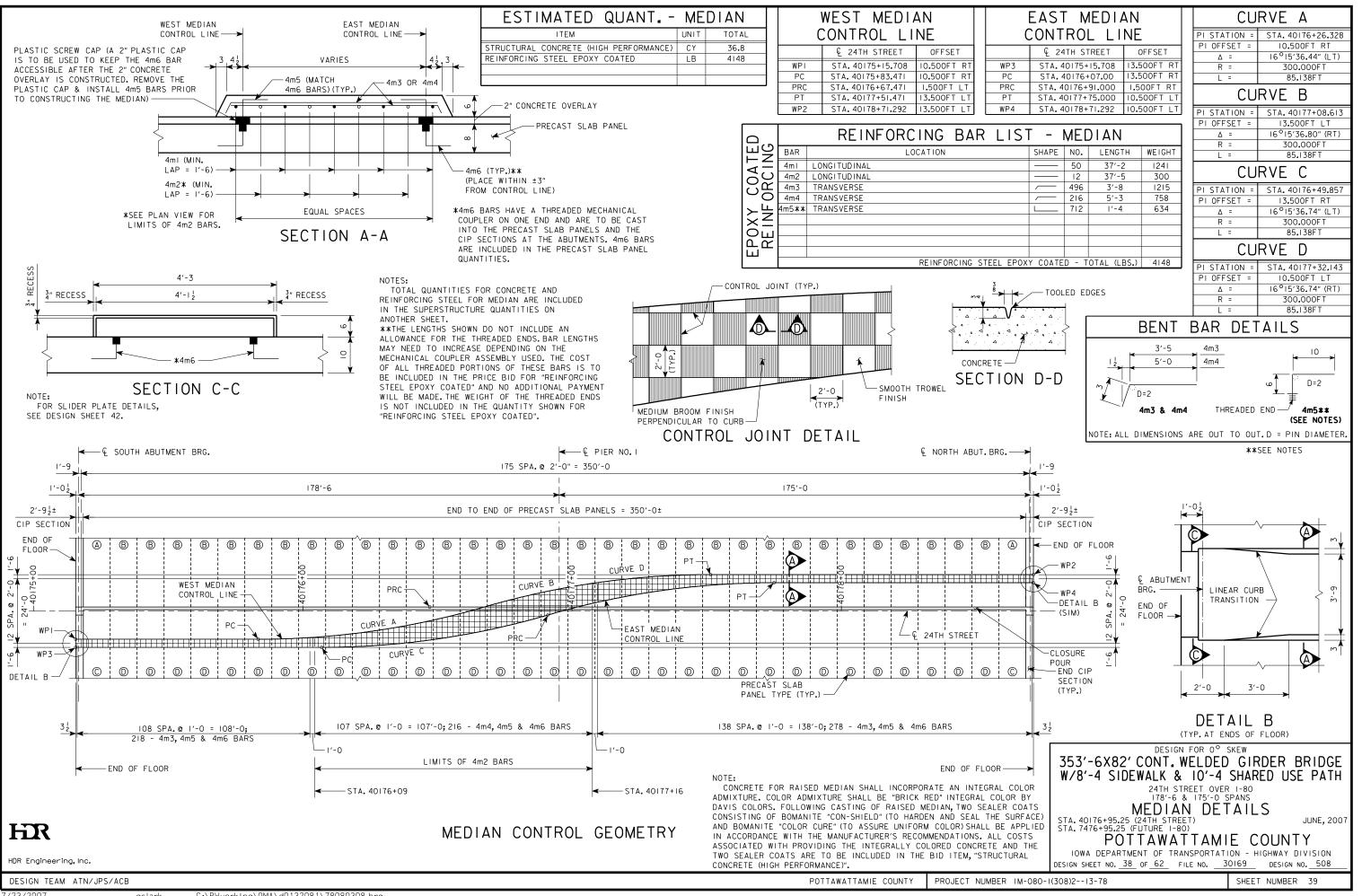
(PANEL TYPE C SHOWN, TYPE A SIMILAR)

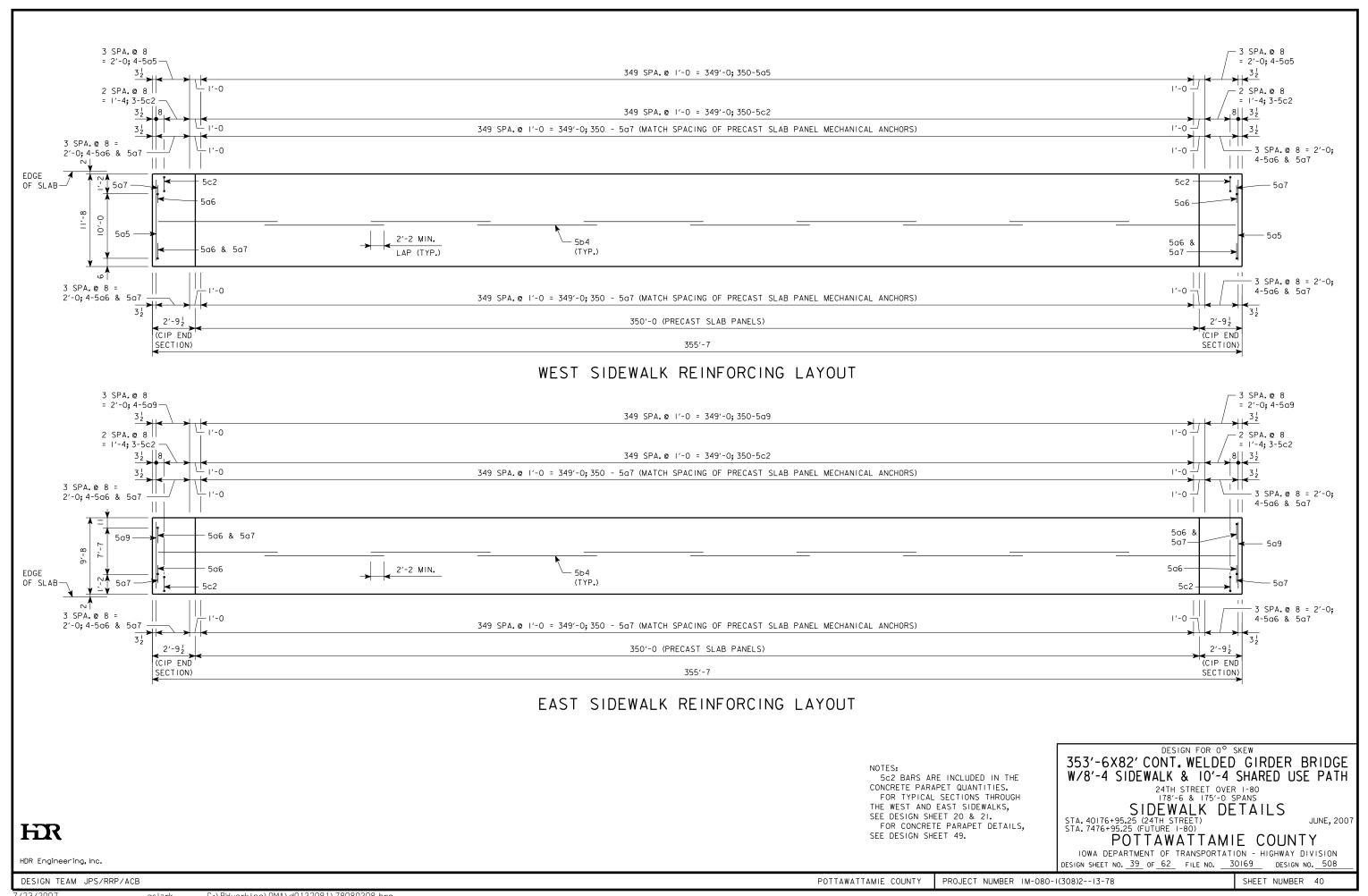
DESIGN TEAM ATN/JPS/ACB

HDR Engineering, Inc.

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-I(308)2--13-78





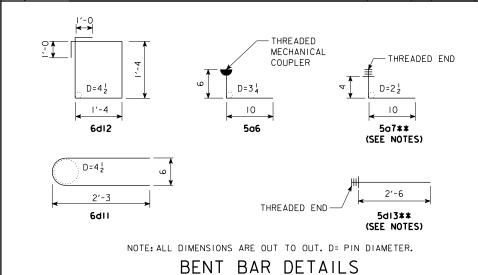
CONC. PLACEMENT	QUANT	ITIES	
LOCATION	PHASE I	PHASE 2	QUANTITY
CIP END SECTION	14.0	14.8	28.8
SIDEWALK	112.0	87.8	199.8
LIGHT POLE BASE	0.5	0.5	1.0
CLOSURE POUR	-	16.2	16.2
MEDIAN	-	36.8	36.8
TOTAL (CU. YDS.)	126.5	156.1	282.6

ESTIMATED QUANTITIES SUPERSTR. UNIT PHASE I PHASE 2 QUANTITY STRUCTURAL CONCRETE (HIGH PERFORMANCE) CU. YD. 126.5 156.1 282.6 REINFORCING STEEL EPOXY COATED LBS. 30,211 33,438 63,649 STRUCTURAL STEEL LBS. 1,485,189 STEEL EXTRUSION JOINT WITH NEOPRENE L.F. 211.5 DISC BEARINGS EACH 12

ALL THREADED MECHANICAL COUPLER ASSEMBLIES TO BE USED IN SPLICING THE REINFORCING IN THE SUPERSTRUCTURE SHALL BE EPOXY COATED. THREE ADDITIONAL NON EPOXY COATED SPLICE ASSEMBLIES OF EACH SIZE SHALL BE FURNISHED TO THE ENGINEER FOR TESTING AND APPROVAL. THE COST OF ALL COUPLERS, INCLUDING THE 3 TO BE FURNISHED FOR TESTING, IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL EPOXY COATED" AND NO ADDITIONAL PAYMENT WILL BE MADE. THE WEIGHT OF THE MECHANICAL COUPLERS IS NOT INCLUDED IN THE QUANTITY SHOWN FOR "REINFORCING STEEL EPOXY COATED".

** THE LENGTHS SHOWN DO NOT INCLUDE AN ALLOWANCE FOR THE THREADED ENDS. BAR LENGTHS MAY NEED TO INCREASE DEPENDING ON THE MECHANICAL COUPLER ASSEMBLY USED. THE COST OF ALL THREADED PORTIONS OF THESE BARS IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL EPOXY COATED" AND NO ADDITIONAL PAYMENT WILL BE MADE. THE WEIGHT OF THE THREADED ENDS IS NOT INCLUDED IN THE QUANTITY SHOWN FOR "REINFORCING STEEL EPOXY COATED".

REINFORCING BAR LIST-SUPERSTRUCTURE PHASE I PHASE 2 LENGTH NO. WEIGHT NO. WEIGHT LOCATION 7al ABUT. DIAPH. TRANSVERSE 23'-10 40 1949 40 1949 16'-3 20 7a2 ABUT. DIAPH. TRANSVERSE 664 20 664 8'-8 20 7g3 ABUT, DIAPH, TRANSVERSE 354 24 425 5a5 SIDEWALK TRANSVERSE 11'-4 358 4232 5a6* MECHANICAL ANCHORS AT C.I.P. END SECTIONS 1'-4 28 39 28 39 5a7** SIDEWALK TRANSVERSE 1'-2 716 871 716 871 5a9 SIDEWALK TRANSVERSE 3485 5b3 CLOSURE POUR LONGITUDINAL 37'-7 40 | 1568 5b4 SIDEWALK LONGITUDINAL 5880 | 130 | 5096 37'-7 | 150 B 10 6dII ABUT. DIAPH. HOOPS 4'-5 248 1645 272 1804 ΝF 6d12 ABUT. DIAPH. HOOPS 7'-4 | 110 | 1212 | 126 | 1388 2'-6 228 5dl3** ABUT. DIAPH. TIE 595 228 595 RE D Ы 0A \mathbb{C} P0) CONCRETE PARAPET - SEE DES. SHT. NO. 49 4295 SEPARATION BARRIER RAIL - SEE DES. SHT. NO. 50 687 8241 LIGHT POLE BASES - SEE DES. SHT. NO. 48 234 MEDIAN-SEE DES. SHT. NO. 38 4148 REINFORCING STEEL EPOXY COATED - TOTAL (LBS.) 30,211 33,438



*INCLUDES I THREADED MECHANICAL COUPLER **SEE NOTES

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80

SUPERSTRUCTURE DETAILS STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80) JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 40 OF 62 FILE NO. 30169 DESIGN NO. 508

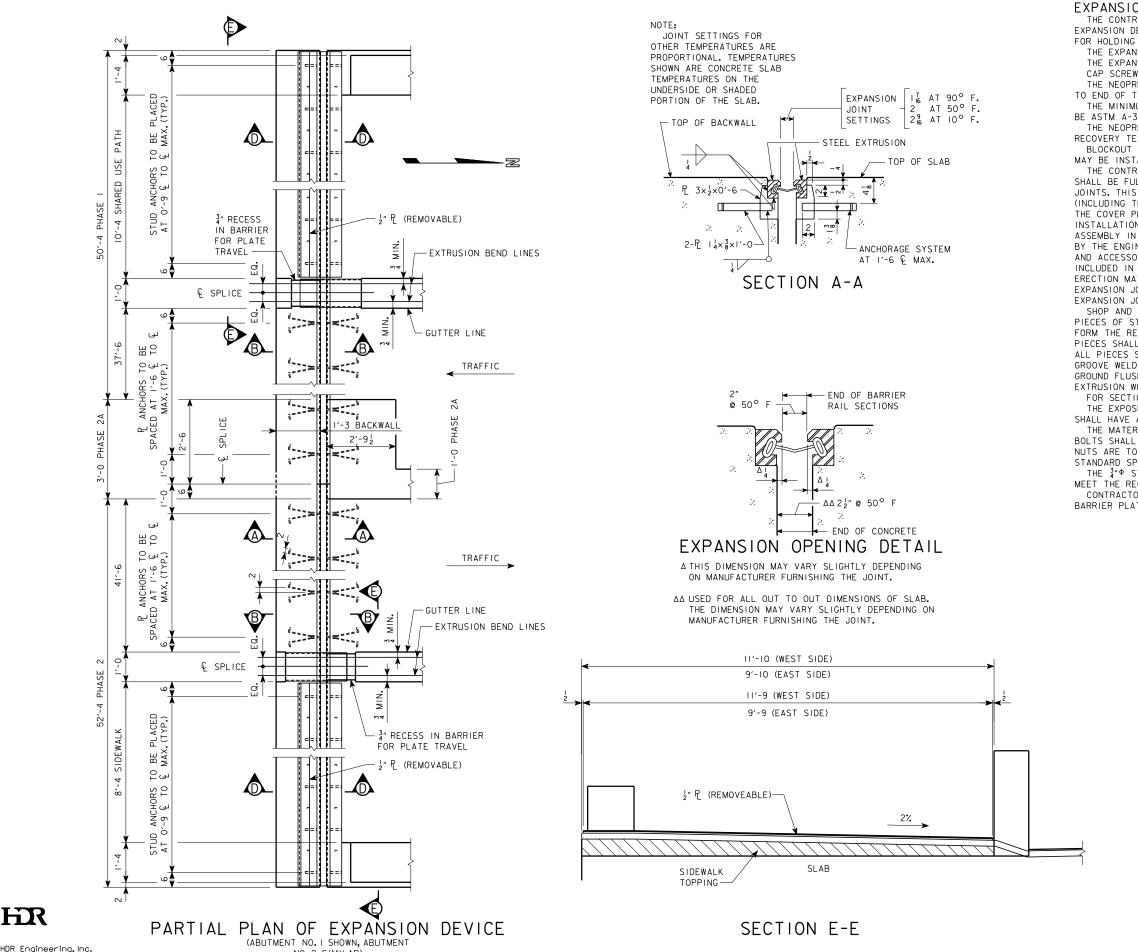
STUB ABUT. B, C, & D BEAMS - BAR LIST & SUPER. DETAILS - 0° SKEW

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-1(308)2--13-78

SHEET NUMBER 41

DESIGN TEAM RRP/JPS/ACB



EXPANSION DEVICE NOTES:

THE CONTRACTOR SHALL SUBMIT FOR APPROVAL SHOP DRAWINGS OF THE EXPANSION DEVICES SHOWING LAYOUT, MATERIAL TO BE USED, AND PROVISIONS FOR HOLDING DEVICE DURING PLACEMENT OF CONCRETE.

THE EXPANSION DEVICE SHALL BE GALVANIZED AFTER WELDING.

THE EXPANSION DEVICE IS TO BE PARALLEL TO GRADE.

CAP SCREWS SHALL BE COUNTERSUNK 16" BELOW TOP OF THE PLATE.

THE NEOPRENE GLAND IS TO BE PLACED AS ONE CONTINUOUS PIECE FROM END TO END OF THE STEEL EXTRUSIONS.

THE MINIMUM GRADE OF STRUCTURAL STEEL FOR THE EXPANSION DEVICE SHALL BE ASTM A-36.

THE NEOPRENE GLAND SHALL CONFORM TO ASTM D-2628 MODIFIED TO EXCLUDE RECOVERY TESTS AND COMPRESSION SET.

BLOCKOUT DETAILS MAY BE ALTERED FROM THOSE SHOWN PROVIDED THE GLAND MAY BE INSTALLED AND REMOVED IF NECESSARY.

THE CONTRACT UNIT PRICE BID FOR "STEEL EXTRUSION JOINT WITH NEOPRENE" SHALL BE FULL COMPENSATION FOR FURNISHING AND INSTALLING THE EXPANSION JOINTS. THIS WORK WILL CONSIST OF FURNISHING ALL REQUIRED MATERIALS, (INCLUDING THE 3" PLATES AT THE BARRIERS AND THEIR ANCHORAGE SYSTEMS AND THE COVER PLATE ASSEMBLIES AT THE SIDEWALKS AND RAISED MEDIAN), AND THE INSTALLATION AND ADJUSTMENT OF THE EXPANSION JOINTS AND COVER PLATE ASSEMBLY IN ACCORDANCE WITH THE DETAILS SHOWN ON THE PLANS AND AS DIRECTED BY THE ENGINEER. THE FURNISHING AND INSTALLATION OF ALL NECESSARY HARDWARE AND ACCESSORIES AS SUPPLIED BY THE EXPANSION JOINT MANUFACTURER ARE TO BE INCLUDED IN THIS WORK, INCLUDING THE ANCHORAGE SYSTEM AND ANY TEMPORARY ERECTION MATERIAL. ALL WORK AND MATERIALS FOR THE INSTALLATION OF THE EXPANSION JOINTS ARE TO COMPLY WITH THE WRITTEN RECOMMENDATIONS OF THE EXPANSION JOINT MANUFACTURER.

SHOP AND OR FIELD SPLICES OF THE STEEL EXTRUSION WILL BE PERMITTED. PIECES OF STEEL EXTRUSION IN THE 15 FT. TO 22 FT. RANGE SHALL BE USED TO FORM THE REQUIRED GUTTER TO GUTTER LENGTH. THE INDIVIDUAL LENGTH OF PIECES SHALL BE CHOSEN SO THAT A MINIMUM NUMBER OF SPLICES IS REQUIRED. ALL PIECES SHALL BE JOINED WITH A PREQUALIFIED PARTIAL PENETRATION SINGLE GROOVE WELD, AND ALL SURFACES NOT IN CONTACT WITH CONCRETE ARE TO BE GROUND FLUSH. NO WELD SHALL BE PERMITTED IN THE INTERNAL SECTION OF THE EXTRUSION WHERE THE NEOPRENE GLAND IS TO BE LOCATED.

FOR SECTIONS B-B & D-D, SEE DESIGN SHEET 42.

THE EXPOSED SURFACE OF THE 1" REMOVABLE COVER PLATE AT THE SIDEWALK SHALL HAVE A NON-SLIP TEXTURE CONFORMING TO ASTM 786.

THE MATERIAL USED FOR THE BARRIER PLATES IS TO BE ASTM A-36 STEEL. THE BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A-307. THE PLATES, BOLTS AND NUTS ARE TO BE GALVANIZED IN ACCORDANCE WITH ARTICLE 4100.07 OF THE STANDARD SPECIFICATIONS.

THE 3" STAINLESS STEEL SOCKET FLAT COUNTERSUNK HEAD CAP SCREWS SHALL MEET THE REQUIREMENTS OF ASTM F879-91.

CONTRACTOR TO NOTE THAT THE CAP SCREW ANCHORAGE SYSTEM FOR THE 3" BARRIER PLATES ARE ALWAYS TO BE PLACED ON THE ONCOMING TRAFFIC SIDE.

TABLE OF APPROVED) EXPANSION	DEVICES
MANUFACTURER	TYPE OF STEEL EXTRUSION	NEOPRENE GLAND
WATSON-BOWMAN & ACME CORP.	Δ	SE-400
D.S. BROWN CO.	SSA2	A2R-400
APPROVED EQUAL		

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

ABUTMENT EXPANSION DEVICE

STA. 40176+95.25 STA. 7476+95.25

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 41 OF 62 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM JPS/RRP/DHS C:\PWworking\OMA\d0132081\78080308.brg

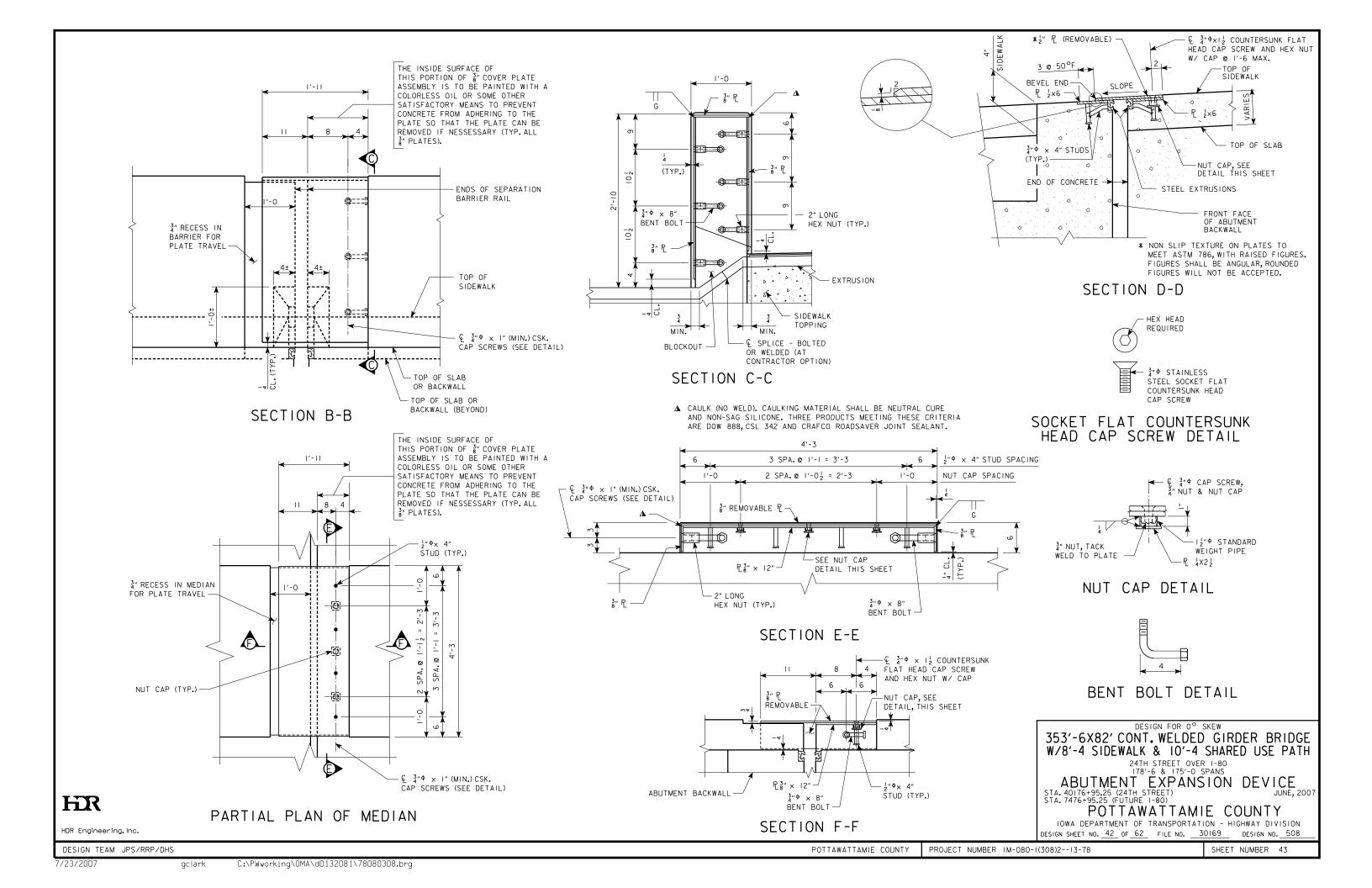
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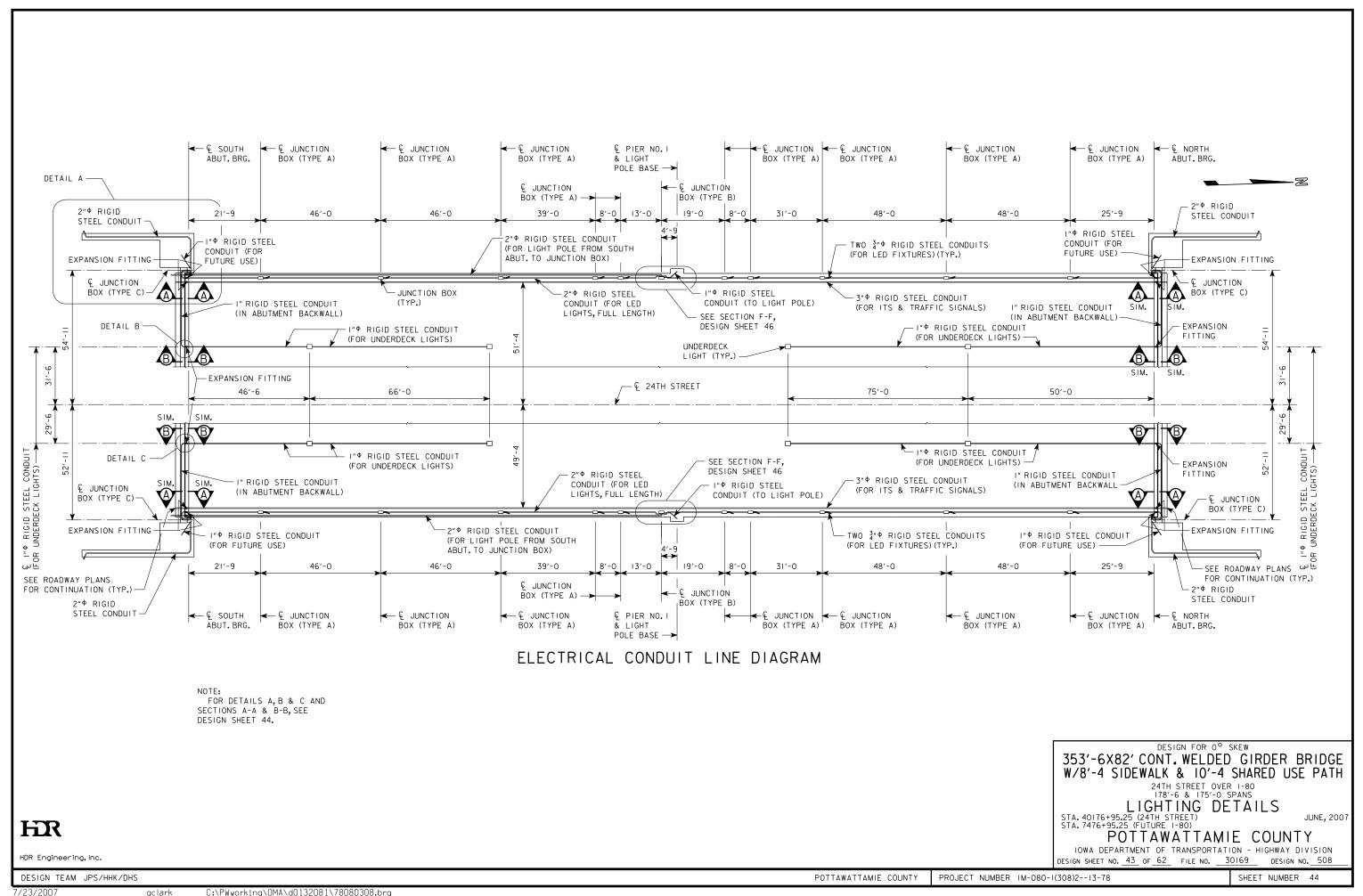
POTTAWATTAMIE COUNTY

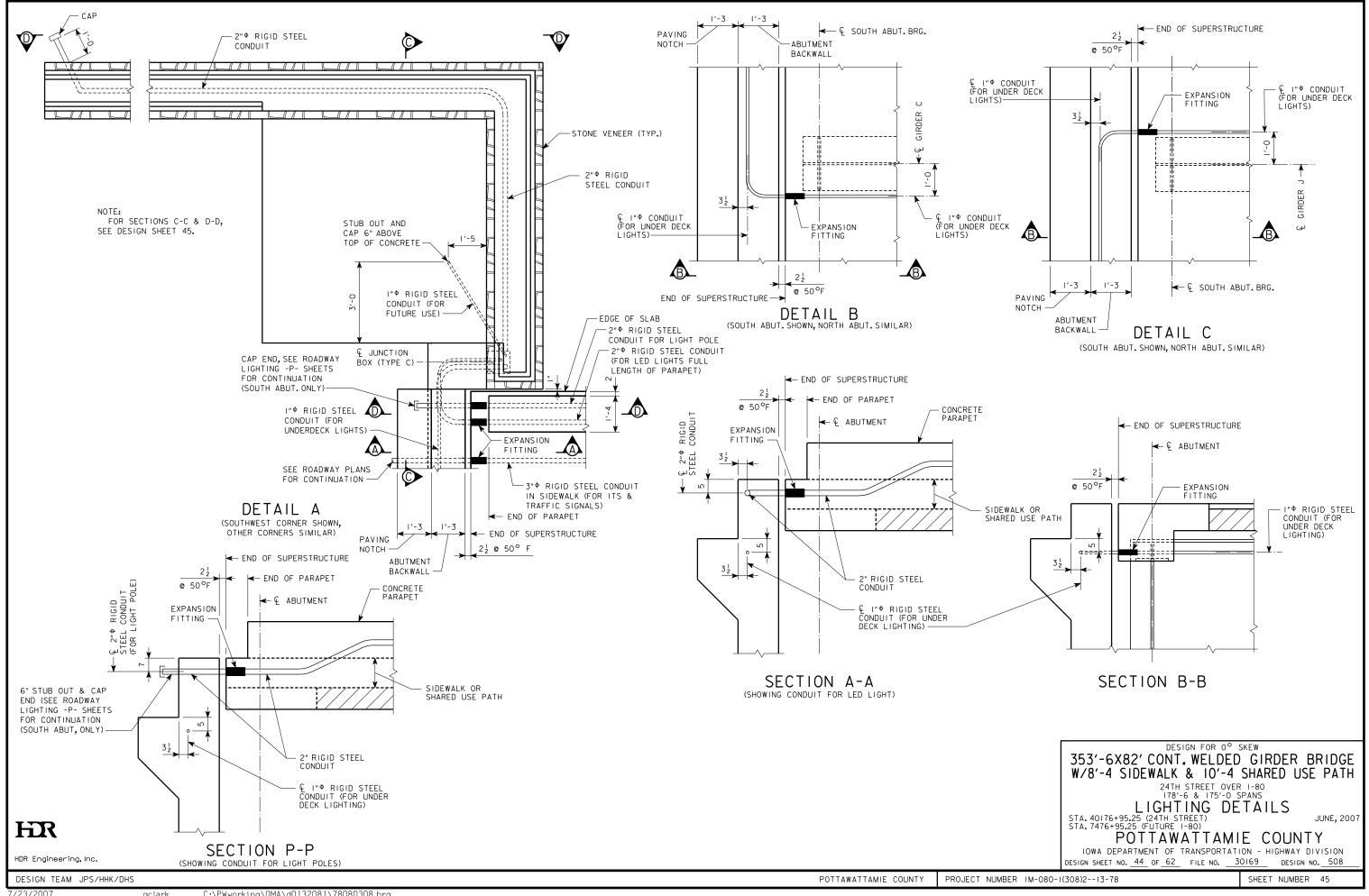
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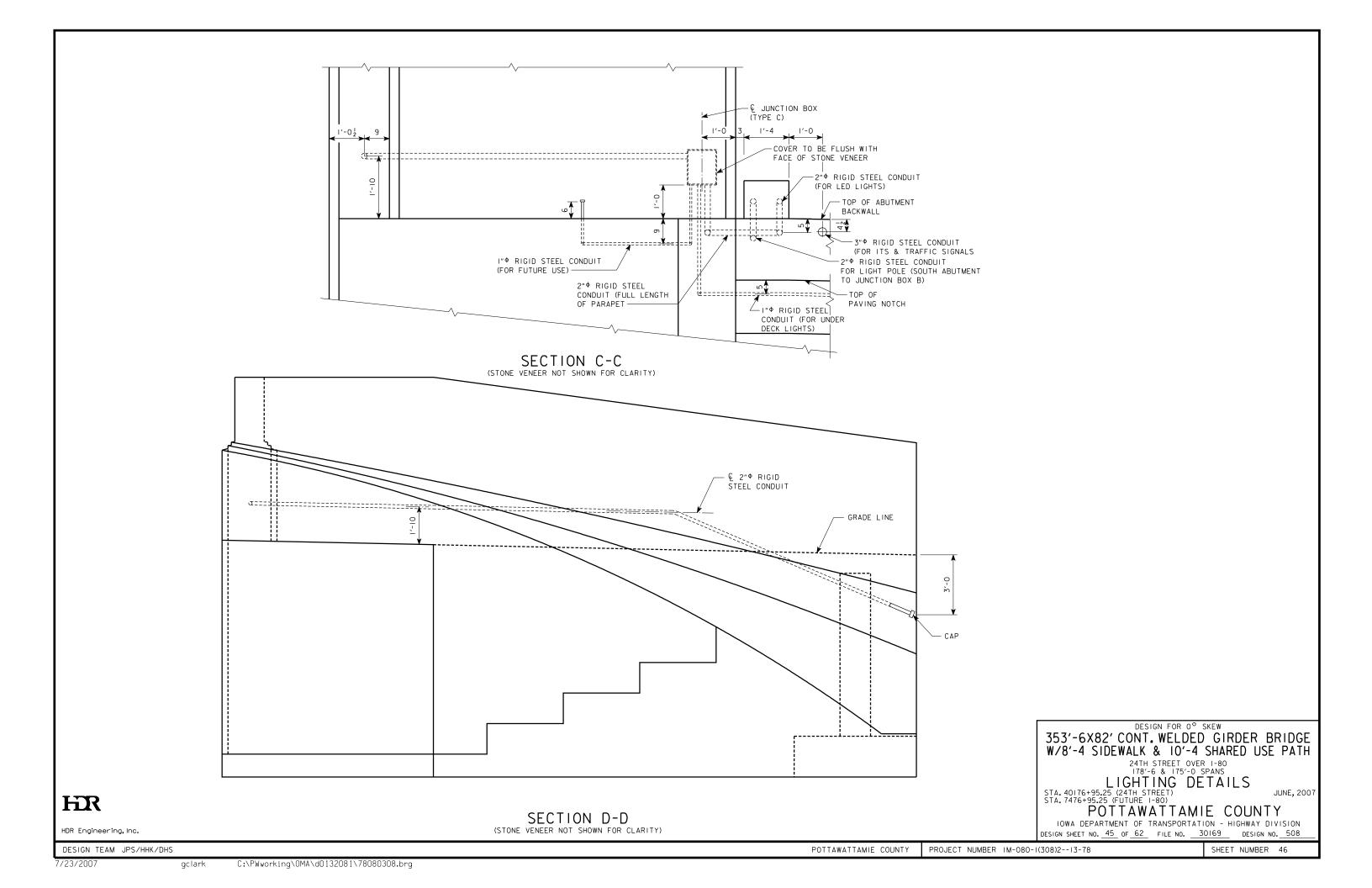
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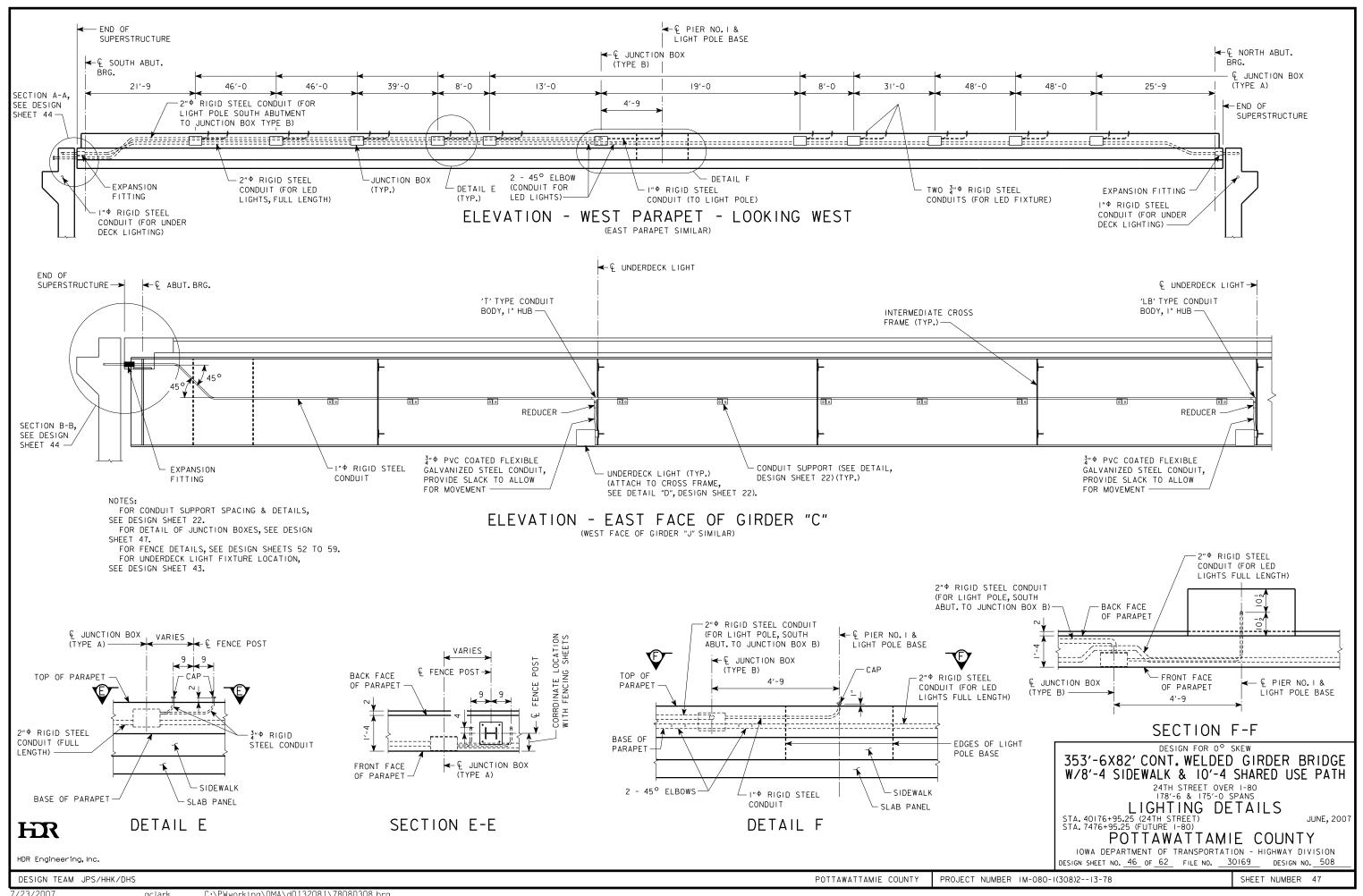
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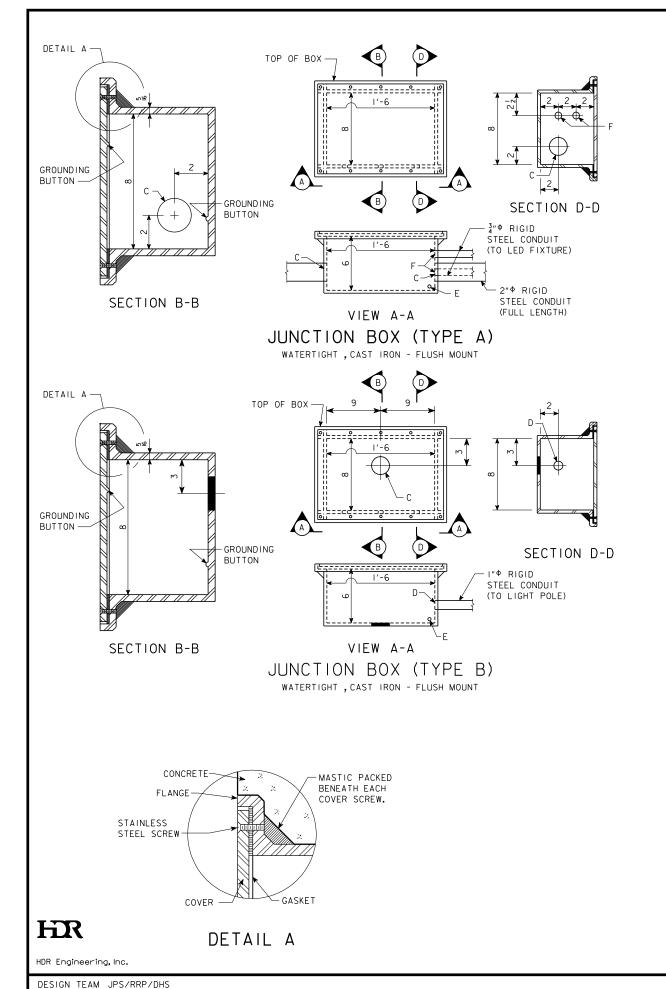


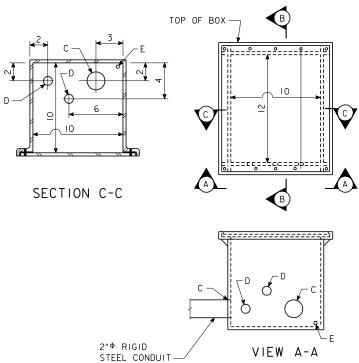


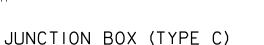




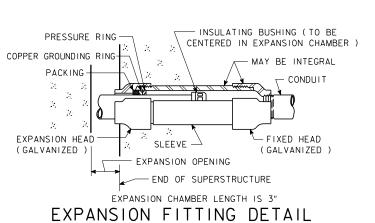








WATERTIGHT , CAST IRON - FLUSH MOUNT



(12 REQUIRED)

BOSSED FOR HOLE FOR CONDUIT SIZE C 2" PRIGID STEEL D I" PRIGID STEEL NONE E ½"Φ COPPER PIPE NONE F ³/₄"Φ RIGID STEEL NONE

- GROUNDING

FOR ADDITIONAL NOTES

SEE DESIGN SHEET 48.

BUTTON

-¹"∮ COPPER DRAIN

CUT

SECTION B-B

FLUSH

NOTE: THE GROUNDING BUTTONS ARE TO BE BLIND DRILLED AND TAPPED FOR 8" 4 × 0'-04 BOLTS.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS LIGHTING DETAILS STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 47 OF 62 FILE NO. 30169 DESIGN NO. 508

POTTAWATTAMIE COUNTY

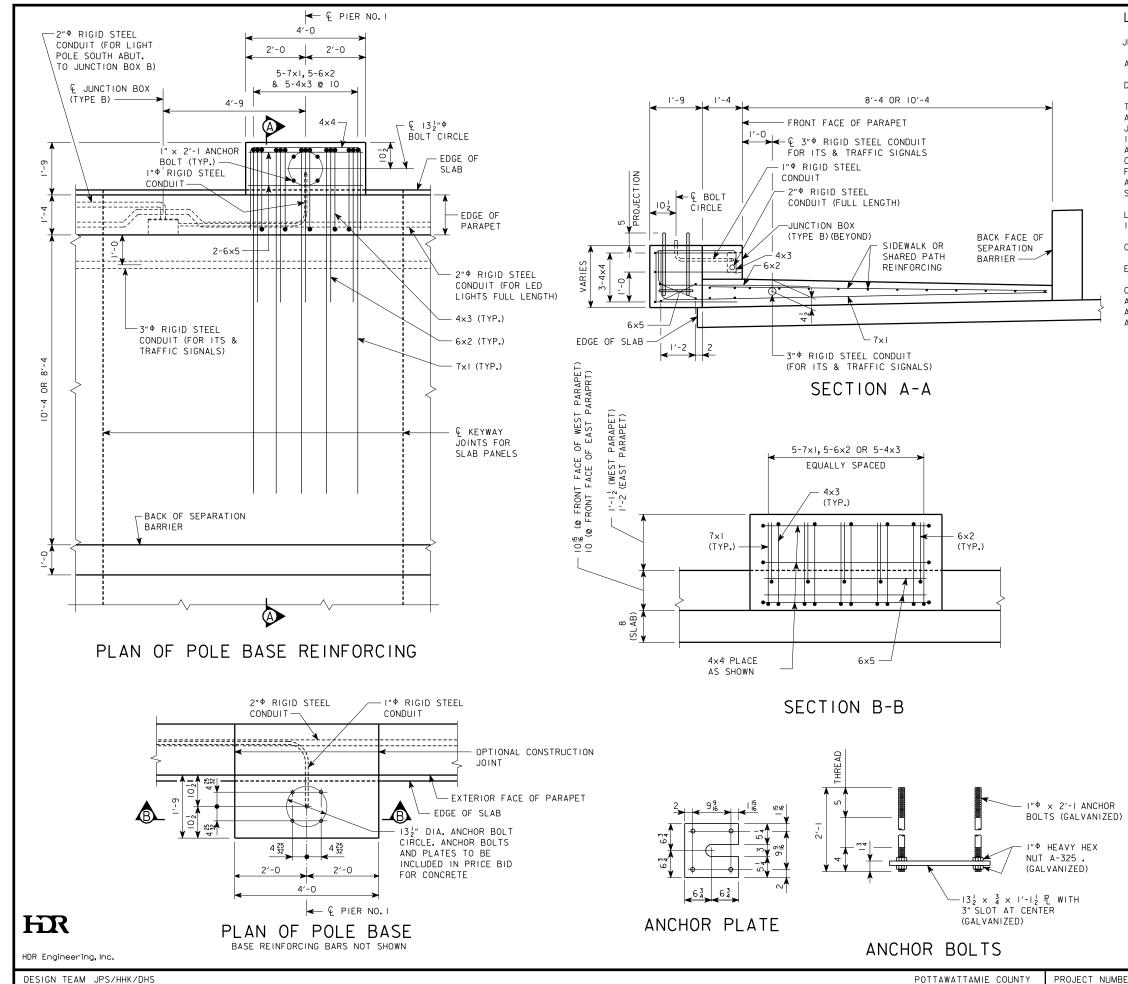
PROJECT NUMBER IM-080-1(308)2--13-78

DETAIL A

GROUNDING

BUTTON —

STONE VENEER -



LIGHTING NOTES:

SEE RM-37 STANDARD ROAD PLAN FOR ADDITIONAL INFORMATION ON JUNCTION BOXES.

CONSTRUCTION SHALL CONFORM TO THE CURRENT IOWA D.O.T. STANDARD AND SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS.

CONDUIT INSTALLATION SHALL COMPLY WITH THE ARTICLE "ELECTRICAL DUCTS", SECTION 2523.

ALL "C" ENTRANCE HOLES IN JUNCTION BOXES SHALL BE DRILLED AND TAPPED FOR THE SPECIFIED CONDUIT SIZE. ALL OTHER HOLES SHALL HAVE A CONCRETE - TIGHT SLIP FIT. CONDUIT ENDS SHALL NOT PROTRUDE INTO JUNCTION BOX MORE THAN 4". DRAIN PIPE END SHALL BE FLUSH WITH INSIDE SURFACE OF BOX. GROUNDING BUTTONS SHALL BE LOCATED APPROXIMATELY 3" FROM THE INSIDE SURFACE OF THE BOX WALL, AND NOT CLOSER THAN 3" TO THE EDGE OF ANY HOLE IN THE BOX FLOOR. HOLES FOR DRAIN PIPE SHALL BE PLACED IN THE LOW CORNER OF THE BOX, WITH A MINIMUM CLEARANCE OF I" BETWEEN THE EDGE OF THE HOLE AND THE INSIDE SURFACE OF THE BOX WALL. TYPICAL DETAILS ARE SHOWN ON DESIGN SHEET 47.

THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS CONSIDERED INCIDENTAL TO THE COST OF THE "CONCRETE BARRIER, PARAPET".

COST OF FURNISHING AND INSTALLING POLES, LIGHTS AND LIGHTING CONDUCTOR IS NOT A PART OF THIS ESTIMATE.

EXPANSION FITTING SHALL BE AS SPECIFIED OR AS APPROVED BY THE ENGINEER. TYPICAL DETAILS ARE SHOWN ON DESIGN SHEET 47.

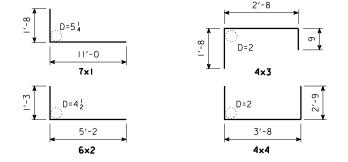
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F-1554, GRADE 105, CLASS 2A. HEAVY HEX NUTS SHALL MEET THE REQUIREMENTS OF THE ASTM A-563-DH, CLASS 2B. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F-436. ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED AND SHALL MEET ALL REQUIREMENTS AS SPECIFIED IN IM 453.08.

ALL REINFORCING STEEL IS TO BE EPOXY COATED AND GRADE 60.

EPOXY REINFORCING STEEL-ONE BASE

BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
7×I	SIDEWALK ANCHORS		5	12'-8	129
6×2	SIDEWALK ANCHORS		5	6′-5	48
4×3	POLE BASE ANCHOR		5	5′-1	17
4×4	POLE BASE HAIRPIN		3	9'-2	18
6×5	POLE BASE LONGITUDINAL	_	4	3′-8	22

TOTAL WEIGHT (LBS.) 234



NOTE: ALL DIMENSIONS ARE OUT TO OUT . D = PIN DIAMETER .

LIGHTING QUANTITIES

ITEM	AMOUNT
STRUCTURAL CONCRETE (HIGH PERFORMANCE) (2 BASES @ 0.5 CU. YD. EACH)	I.O CU. YD.
REINFORCING STEEL - EPOXY COATED (2 BASES @ 234 LBS.EACH)	468 LBS.

NOTES:

FOR LOCATION AND LENGTHS OF CONDUITS NEEDED SEE DESIGN SHEETS 43-47.

TOTAL QUANTITIES FOR CONCRETE AND REINFORCING STEEL FOR POLE BASES ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

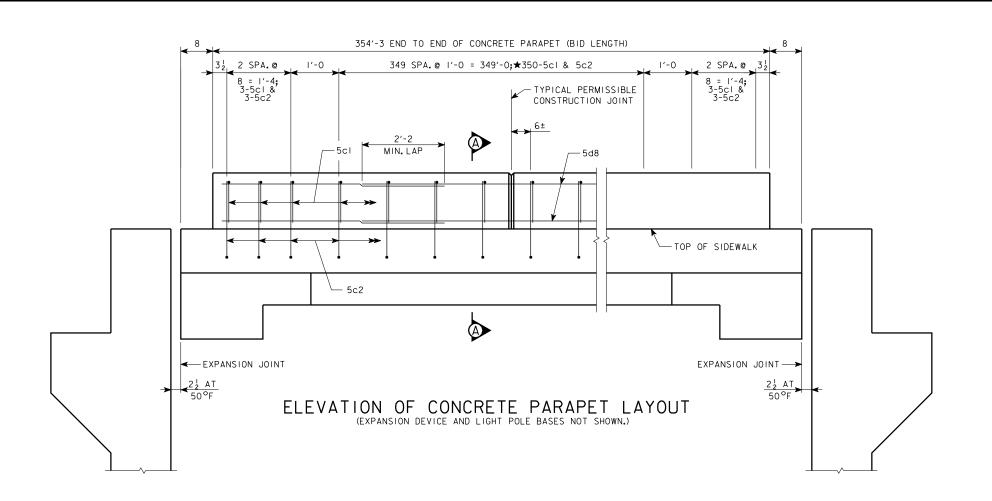
LIGHT POLE BASE DETAILS
40176+95.25 (24TH STREET)
JUNE, 2007

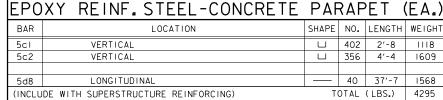
STA. 40176+95.25 (24TH STREET STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

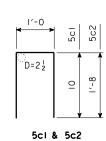
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 48 OF 62 FILE NO. 30169 DESIGN NO. 508

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BENT BAR DETAILS



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.

CONCRETE PLACEMENT SUM	MARY	
SECTION		TOTAL
WEST CONCRETE PARAPET 354.25' @ 0.0556 CU. Y	D. PER FT.	19.7
EAST CONCRETE PARAPET 354.25' @ 0.0578 CU. Y	D. PER FT.	20.5
TOTAL	_ (CU. YD.)	40.2

CONCRETE PARAPET QUANTITIES ITEM UNIT QUANTITY CONCRETE BARRIER, PARAPET L.F. 708.5

PARAPET NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

THE PERMISSIBLE CONSTRUCTION JOINTS ARE TO BE PLACED BETWEEN VERTICAL BARS AT A MINIMUM SPACING OF 20 FEET. CONSTRUCTION JOINT CONTACT SURFACES ARE TO BE COATED WITH AN APPROVED BOND BREAKER. CONSTRUCTION JOINT SHALL BE SPACED A MINIMUM OF I FOOT FROM ANY 9 FENCE POST.

COST OF THE JOINT SEALER AND BOND BREAKER SHALL BE CONSIDERED INCIDENTAL TO OTHER CONSTRUCTION.

★ PLACE I ADDITIONAL 5cI BAR AT EACH & FENCE POST BASE.

ALL PARAPET REINFORCING STEEL IS TO BE EPOXY COATED.

THE CONCRETE PARAPET IS TO BE BID ON A LINEAR FOOT BASIS MEASURED FROM END TO END OF PARAPET. THE NUMBER OF LINEAR FEET OF PARAPET INSTALLED WILL BE PAID FOR AT THE CONTRACT PRICE PER LINEAR FOOT BASED ON PLAN QUANTITIES. PRICE RID FOR "CONCRETE BARRIER PARAPET"

INSTALLED WILL BE PAID FOR AT THE CONTRACT PRICE PER LINEAR FOOT BASED ON PLAN QUANTITIES. PRICE BID FOR "CONCRETE BARRIER, PARAPET" SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIAL, EXCLUDING REINFORCING STEEL, AND ALL OF THE EQUIPMENT AND LABOR REQUIRED TO ERECT THE PARAPET IN ACCORDANCE WITH THESE PLANS AND CURRENT SPECIFICATIONS. IF CONDUIT IS REQUIRED IN THIS PLAN THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS CONSIDERED INCIDENTAL TO THE COST OF THE PARAPET, ALL PARAPET REINFORCING STEEL IS TO BE INCLUDED WITH THE SUPERSTRUCTURE REINFORCING STEEL.

THE JOINT SEALER SHALL BE LIGHT GRAY NONSAG LATEX CAULKING SEALER MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED. TOP OF THE PARAPET IS TO BE PARALLEL TO THE THEORETICAL & GRADE. CROSS SECTIONAL AREA OF THE WEST PARAPET = 1.50 SQUARE FEET. CROSS SECTIONAL AREA OF THE EAST PARAPET = 1.56 SQUARE FEET.

CROSS SECTIONAL AREA OF THE EAST PARAPET = 1.56 SQUARE FEET.

FINISHED PARAPET CONCRETE SHALL BE SMOOTH AND SHOW NO WOOD GRAIN

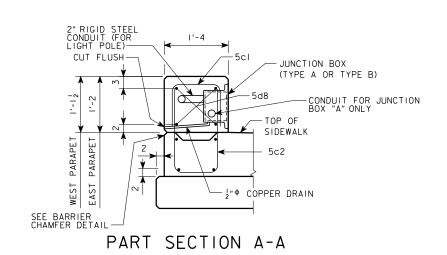
OR OTHER TEXTURE FROM THE FACE OF THE FORMS USED. ALL COSTS FOR REPAIR

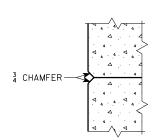
OR COVERING OF WOOD GRAIN OR OTHER TEXTURES ON THESE SURFACES SHALL

BE THE RESPONSIBILITY OF THE CONTRACTOR.

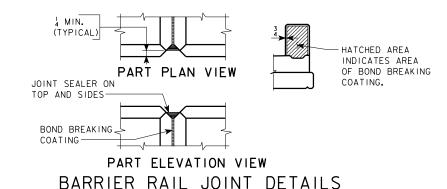
FOR JUNCTION BOX LOCATIONS AND LIGHTING DETAILS, SEE DESIGN SHEETS 43 TO 48.

FOR FENCE POST LOCATIONS, SEE DESIGN SHEET 52.





BARRIER CHAMFER DETAIL



DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80

CONCRETE PARAPET RAIL

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

TAWATTAMIF COUNTY

POTTAWATTAMIE COUNTY

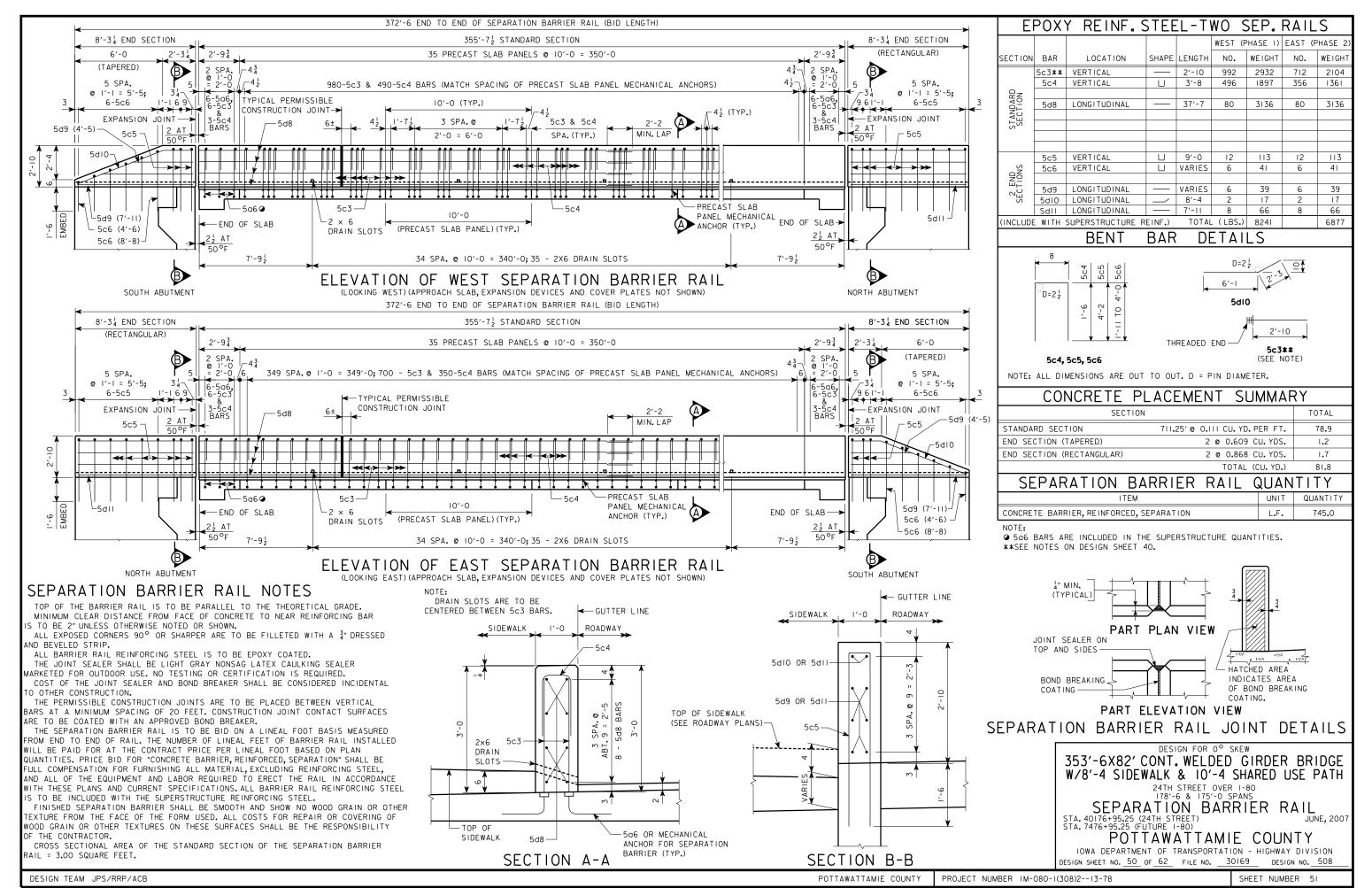
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 49 OF 62 FILE NO. 30169 DESIGN NO. 508

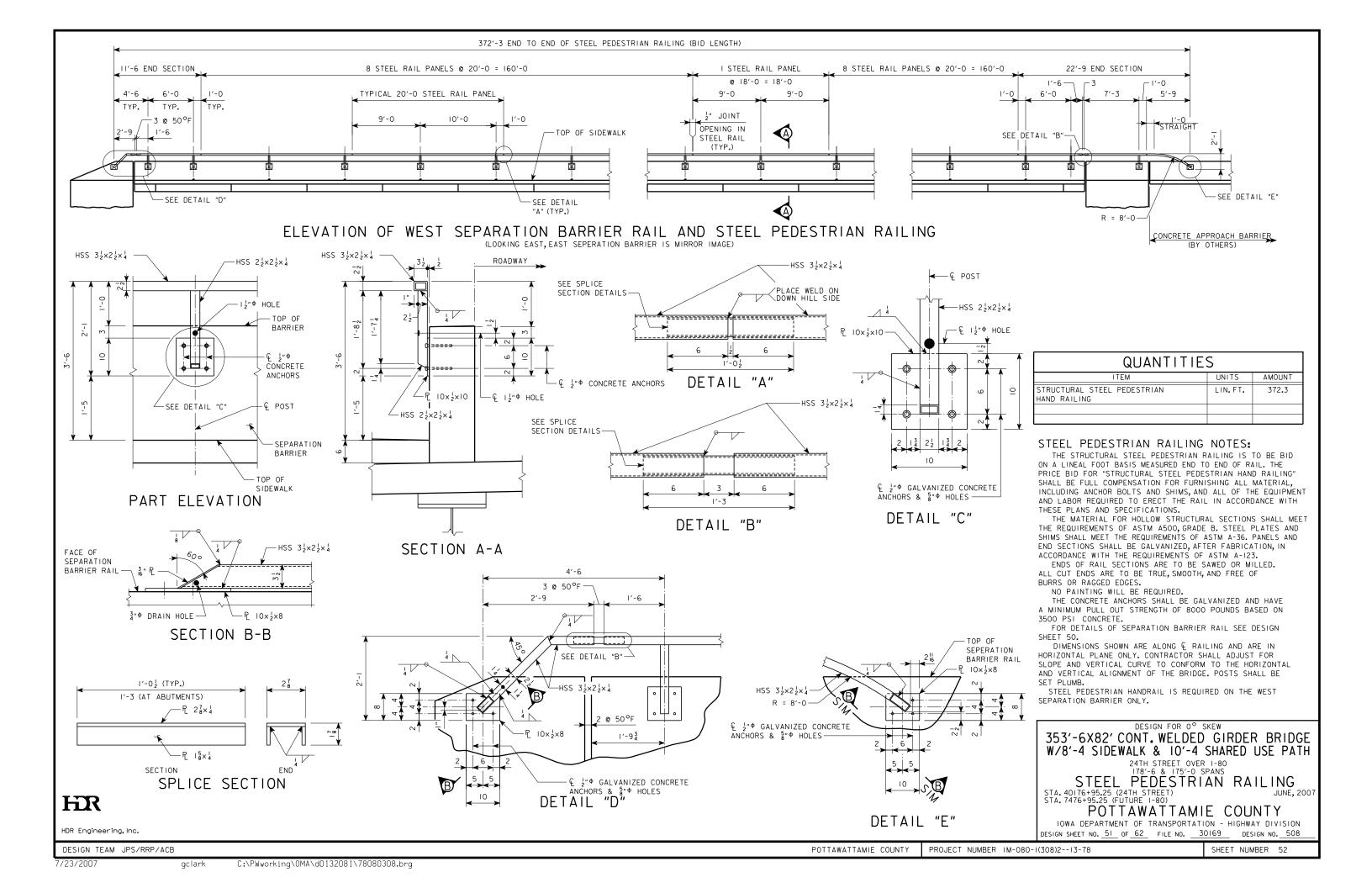
POTTAWATTAMIE COUNTY PROJECT NUMBER IM-080-1(308)2--13-78

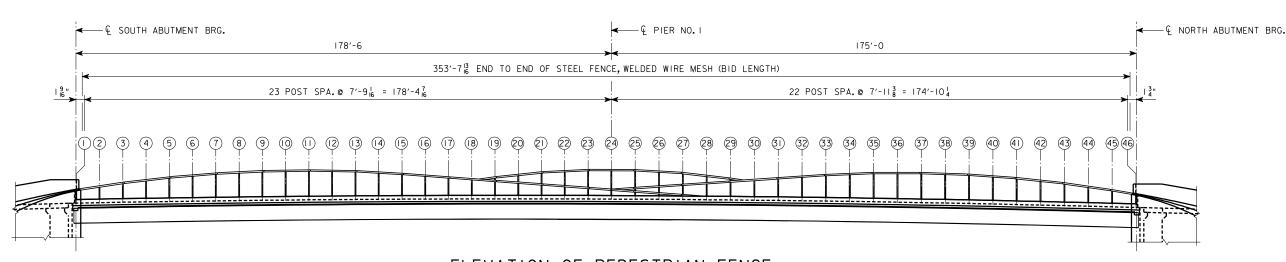
SHEET NUMBER 50

JUNE. 2007

DESIGN TEAM JPS/RRP/ACB







ELEVATION OF PEDESTRIAN FENCE

(OUTSIDE OF EAST FENCE SHOWN, WEST FENCE SIMILAR - OPPOSITE HAND) (FENCE FABRIC NOT SHOWN)

FENCE NOTES:

ALL W5X19 POSTS AND WT2X6.5 ARE TO BE SET PLUMB.

ANCHORAGES SHALL BE ACCURATELY PLACED TO PROVIDE CORRECT ALIGNMENT OF FENCE. ANCHORS ARE TO BE 3" DIAMETER THREADED RODS CONFORMING TO ASTM A709M, GRADE 36. ANCHORS SHALL USE ONE OF THE FOLLOWING

1) HILTH HIT HY150/HIT-ICE WITH $7^1_2\Phi$ INCH MINIMUM EMBEDMENT DEPTH. 2) SIMPSON STRONG-TIE ACRYLIC-TIE WITH $7^1_2\Phi$ INCH MINIMUM EMBEDMENT DEPTH.

3) WEJ-IT INJECT-TITE WITH 9; INCH MINIMUM EMBEDMENT DEPTH.

4) APPROVED EQUAL.

ALL ANCHOR HARDWARE IS TO BE GALVANIZED PER THE STANDARD SPECIFICATIONS.

STRUCTURAL STEEL POSTS SHALL COMPLY WITH ASTM A572, GRADE 50. STRUCTURAL STEEL HOLLOW STRUCTURAL SECTIONS (HSS) SHALL COMPLY WITH ASTM A500, GRADE B. ALL OTHER STRUCTURAL STEEL MATERIALS SHALL COMPLY WITH ASTM A36. GRADE 36 MINIMUM.

ALL BURRS AND SHARP CORNERS OF STEEL FENCE COMPONENTS SHALL BE GROUND SMOOTH PRIOR TO GALVANIZING AND PAINTING.

ALL POSTS, POST BASE PLATES & SHIM PLATES, HSS8X6X 6 RAILS AND TYPE 2 LATTICE FENCE MATERIALS ARE TO BE PAINTED AFTER GALVANIZING IN ACCORDANCE WITH THE "SUPPLEMENTAL SPECIFICATIONS FOR CLEANING, SURFACE PREPARATION AND PAINTING OF GALVANIZED SURFACES". PAINT COLOR SHALL CONFORM TO FEDERAL STANDARD 595B, COLOR NUMBER 20040 (DARK BRONZE). PAINT SHALL BE EXCLUDED FROM ALL SURFACES OF THE STEEL ANGLE PANEL MOUNTING TABS FOR THE TYPE I FENCE BY MEANS OF MASKING. PAINT EDGES SHALL BE ALONG CLEAN STRAIGHT LINES AT MASKED SURFACES.

PROTECT ALL PAINTED SURFACES IMMEDIATELY AFTER PAINT HAS CURED. PROTECTION METHOD SHALL BE ADEQUATE TO PREVENT DAMAGE DURING STORAGE, HANDLING, SHIPPING TO THE INSTALLATION SITE, AND DURING THE INSTALLATION OF THE FENCING, DO NOT REMOVE THE PROTECTION UNTIL POTENTIAL DAMAGE TO THE PAINT IS LIMITED TO FINAL ASSEMBLY SURFACES ONLY. PERFORM TOUCH-UP REPAIR OF PAINT IN ACCORDANCE WITH THE "SUPPLEMENTAL SPECIFICATION FOR CLEANING, SURFACE PREPARATION AND PAINTING OF GALVANIZED SURFACES." DO NOT PAINT ANCHORAGES OR FINAL INSTALLATION HARDWARE.

WIRE MESH PANELS AND ASSOCIATED CHANNEL AND ANGLE FRAME ASSEMBLIES FOR TYPE I FENCING SHALL NOT BE PAINTED, BUT SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. ALL FENCE HARDWARE ASSOCIATED WITH THE TYPE I FENCE SHALL BE GALVANIZED PER THE STANDARD SPECIFICATIONS.

NO SINGLE WIRE OF THE WELDED WIRE MESH SHALL BE WELDED TO THE FRAME AT BOTH ENDS OF THE WIRE. THE WELDED WIRE MESH PANELS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A 123. GOOD STANDARD PRACTICES SHALL BE FOLLOWED IN ACCORDANCE WITH ASTM A143 AND ASTM A 384. PREPARE THE FABRICATED PANEL SURFACES BY ABRASIVE BLAST CLEANING TO A MINIMUM OF SSPC-SP-6 "COMMERCIAL BLAST CLEANING" PRIOR TO HOT-DIP GALVANIZING.

AFTER FABRICATION AND GALVANIZING, ALL FENCE MEMBERS SHALL BE STRAIGHT AND TRUE TO A TOLERANCE OF "IN 10', ANY STRAIGHTENING REQUIRED SHALL BE ACCOMPLISHED BY MECHANICAL MEANS WITHOUT DAMAGE TO THE ZINC COATING.

CAULK FOR BASE PLATES SHALL BE WHITE NONSAG LATEX CAULK MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED. EXCESS CAULK SHALL BE COMPLETELY REMOVED FROM THE SURROUNDING CONCRETE

ALL COSTS ASSOCIATED WITH THE FENCE, INCLUDING THE ANCHORAGES, SHALL BE INCLUDED IN THE PRICE BID FOR "STEEL FENCE, WELDED WIRE MESH."

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

PEDESTRIAN FENCE ELEVATION

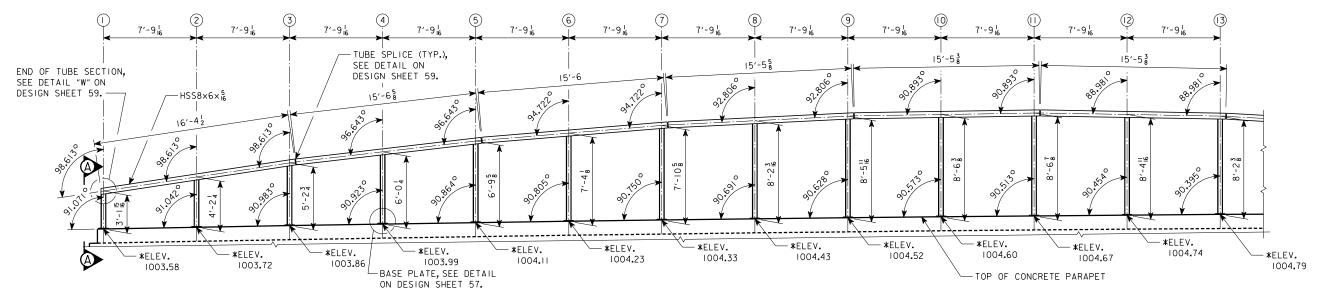
STA. 40176+95.25 (24TH STREE STA. 7476+95.25 (FUTURE I-80)

POTTAWATTAMIE COUNTY IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

SHEET NUMBER 53

DESIGN SHEET NO. 52 OF 62 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM ATN/HHK/ACB POTTAWATTAMIE COUNTY PROJECT NUMBER IM-080-1(308)2--13-78



*ELEVATION IS GIVEN AT THE TOP OF THE CONCRETE PARAPET AT THE & OF THE POST.

PART ELEVATION OF PEDESTRIAN FENCE
(FENCE FABRIC NOT SHOWN, OUTSIDE OF EAST FENCE SHOWN, WEST FENCE SIMILAR - OPPOSITE HAND)

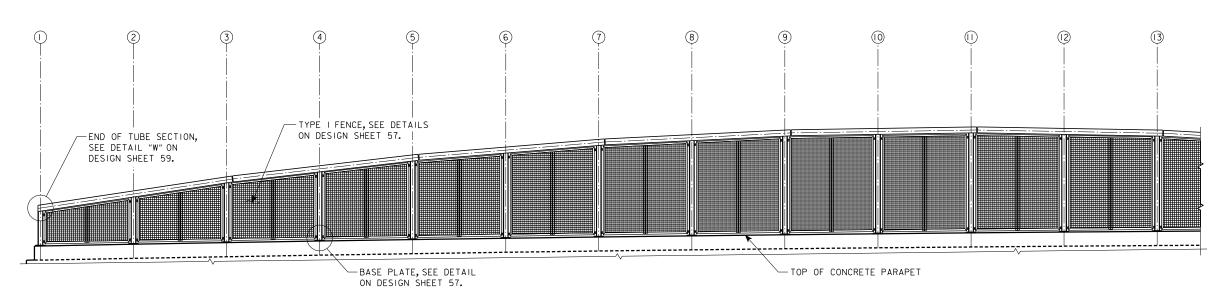
NOTE:

VERTICAL POST DIMENSIONS ARE

MEASURED FROM TOP OF CONCRETE

PARAPET TO THE CENTER OF HSS8×6×6.

FOR SECTION A-A, SEE DESIGN SHEET 56.



PART ELEVATION OF PEDESTRIAN FENCE

(OUTSIDE OF EAST FENCE SHOWN, WEST FENCE SIMILAR - OPPOSITE HAND)

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

PEDESTRIAN FENCE DETAILS

... 40176+95.25 (24TH STREET)

JUNE, 2007

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

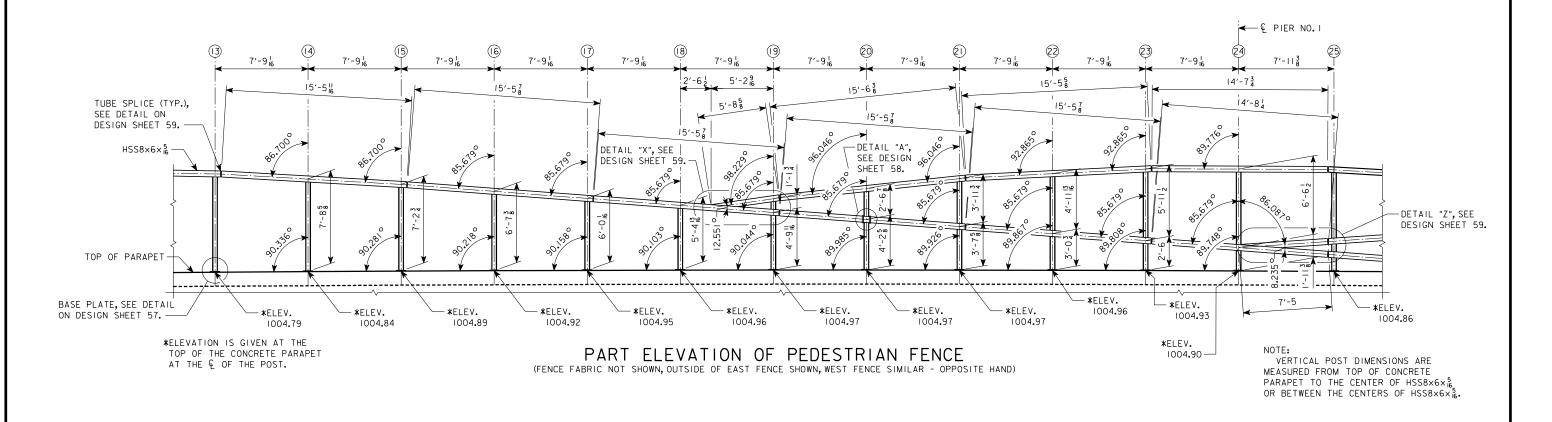
POTTAWATTAMIE COUNTY
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

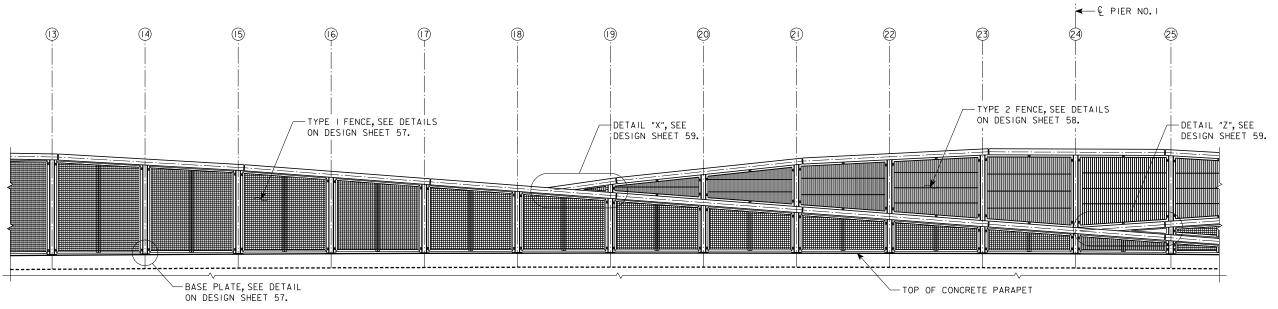
DESIGN SHEET NO. 53 OF 62 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM ATN/HHK/ACB

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-1(308)2--13-78





PART ELEVATION OF PEDESTRIAN FENCE

(OUTSIDE OF EAST FENCE SHOWN, WEST FENCE SIMILAR - OPPOSITE HAND)

DESIGN FOR O° SKEW 353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER 1-80 178'-6 & 175'-0 SPANS

PEDESTRIAN FENCE DETAILS JUNE, 2007

STA. 40176+95.25 (24TH STREET STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

DESIGN SHEET NO. 54 OF 62 FILE NO. 30169 DESIGN NO. 508

HR

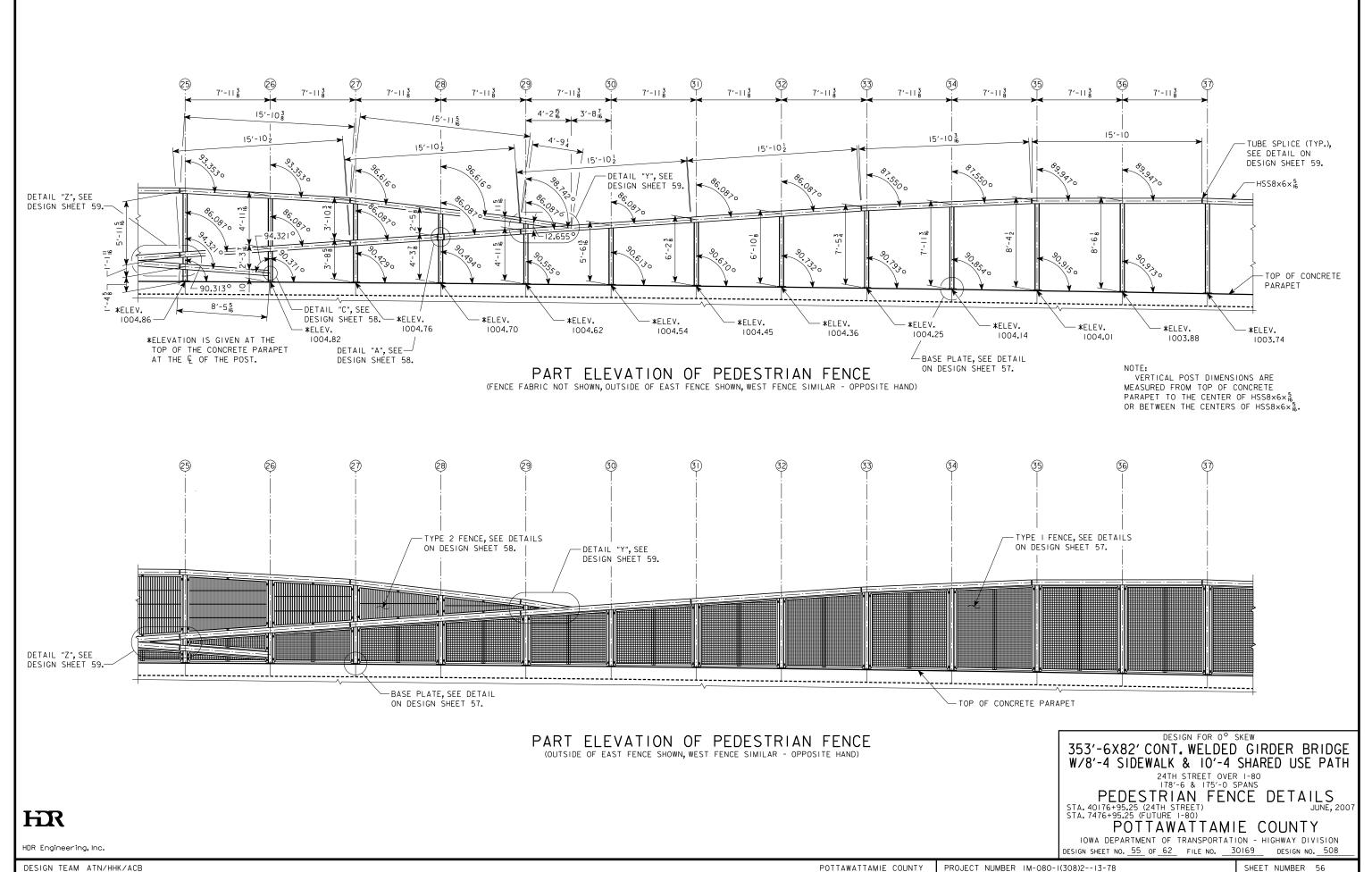
HDR Engineering, Inc.

DESIGN TEAM ATN/HHK/ACB

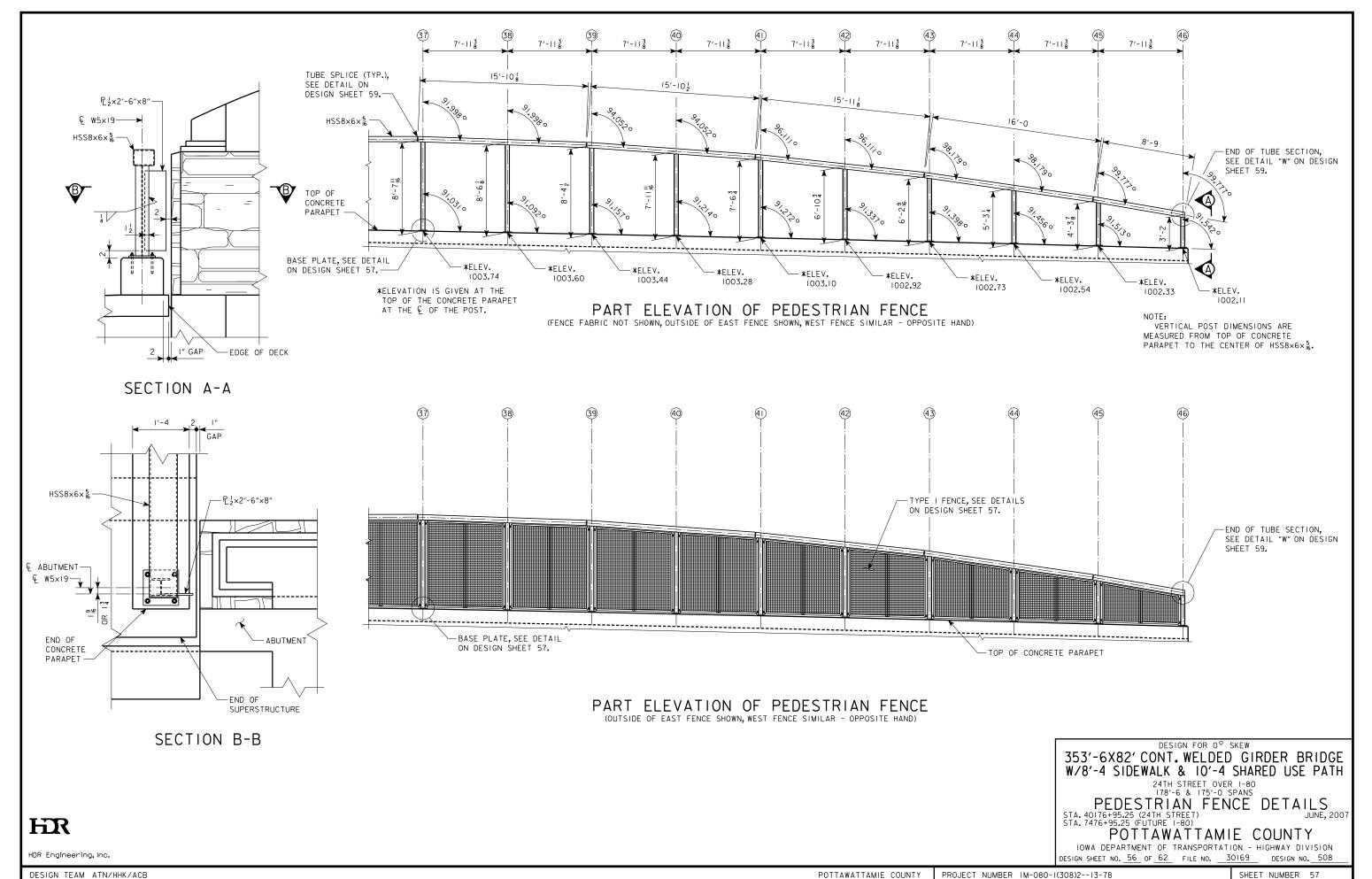
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POTTAWATTAMIE COUNTY

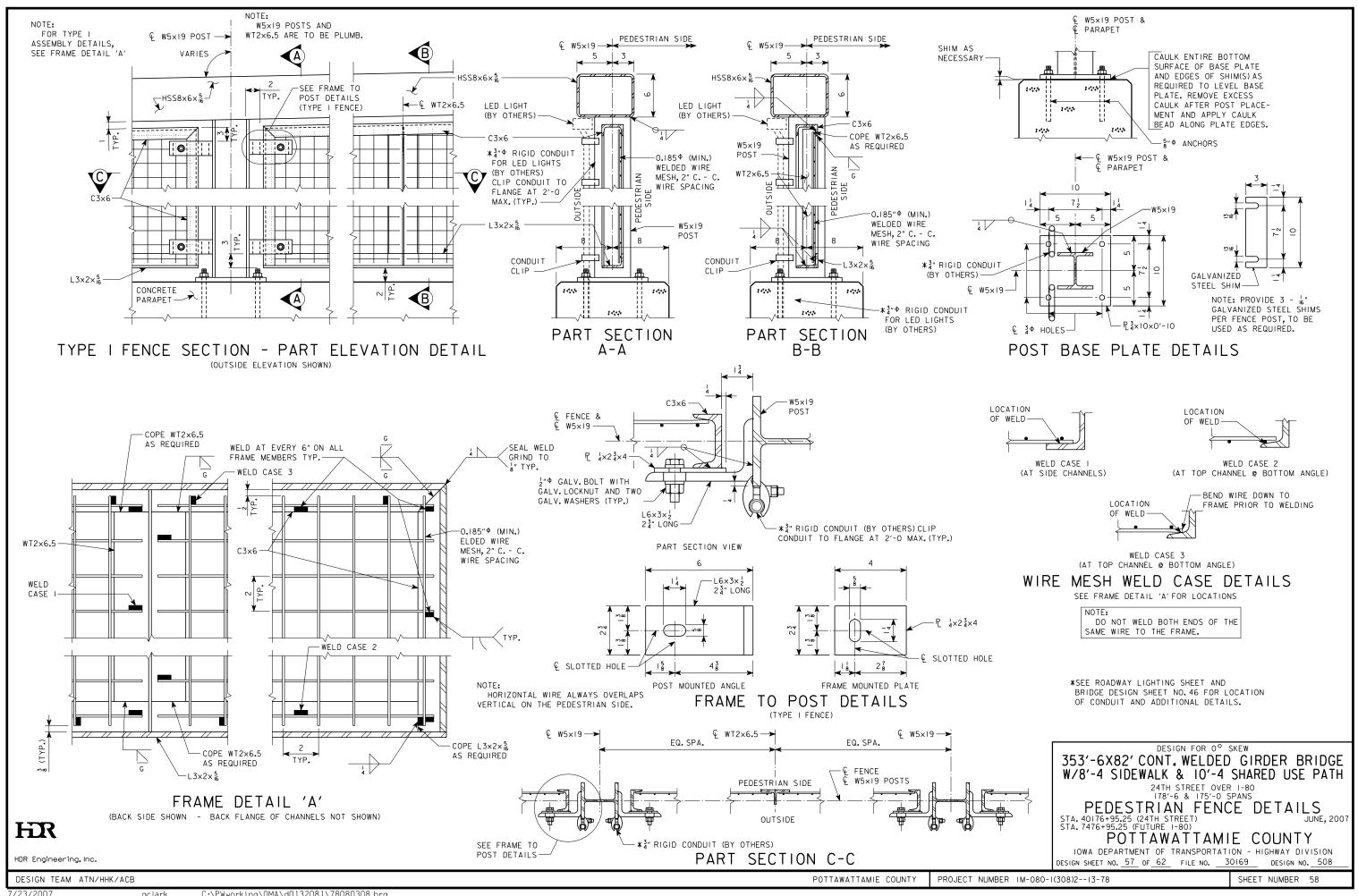
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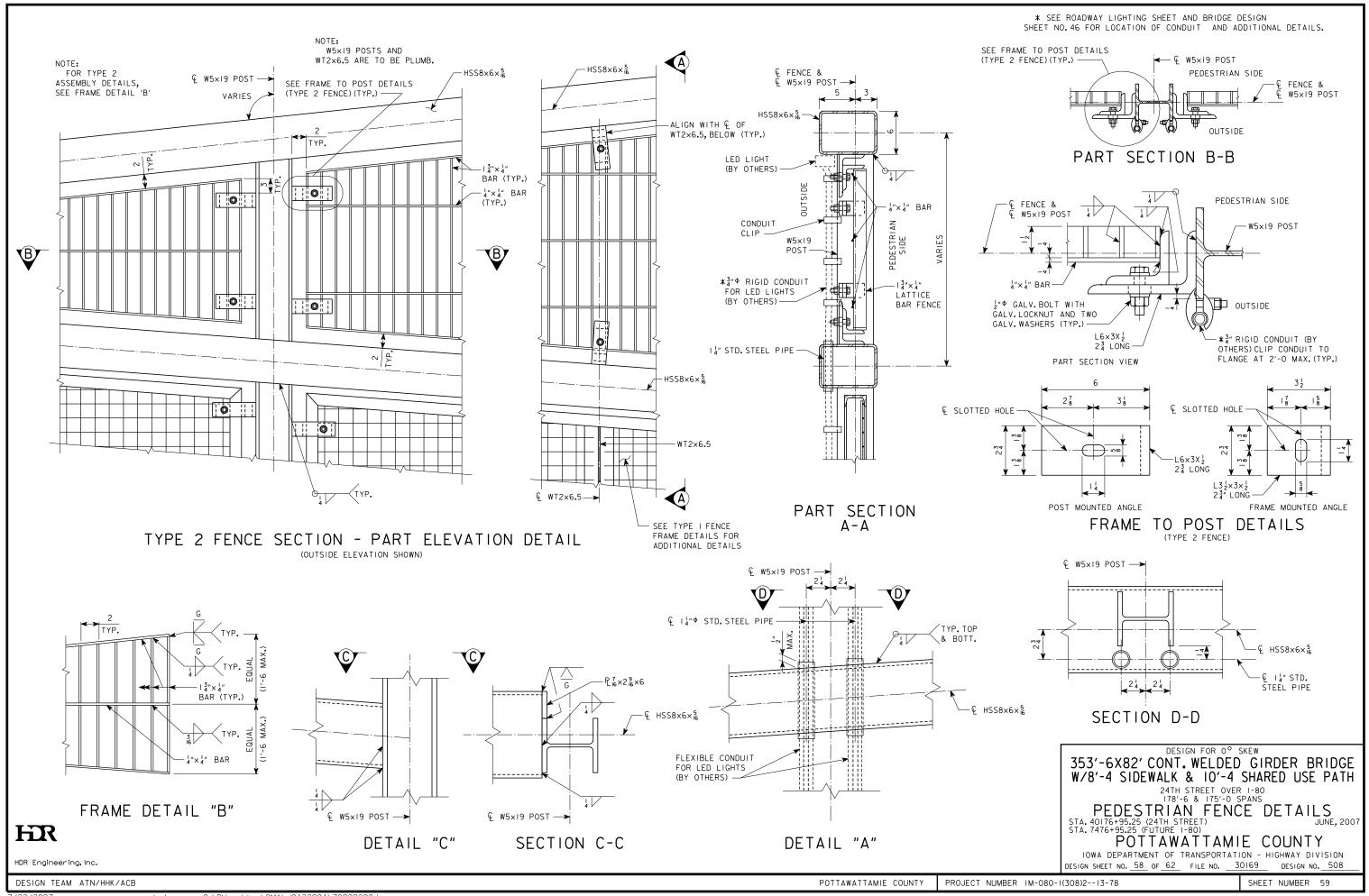


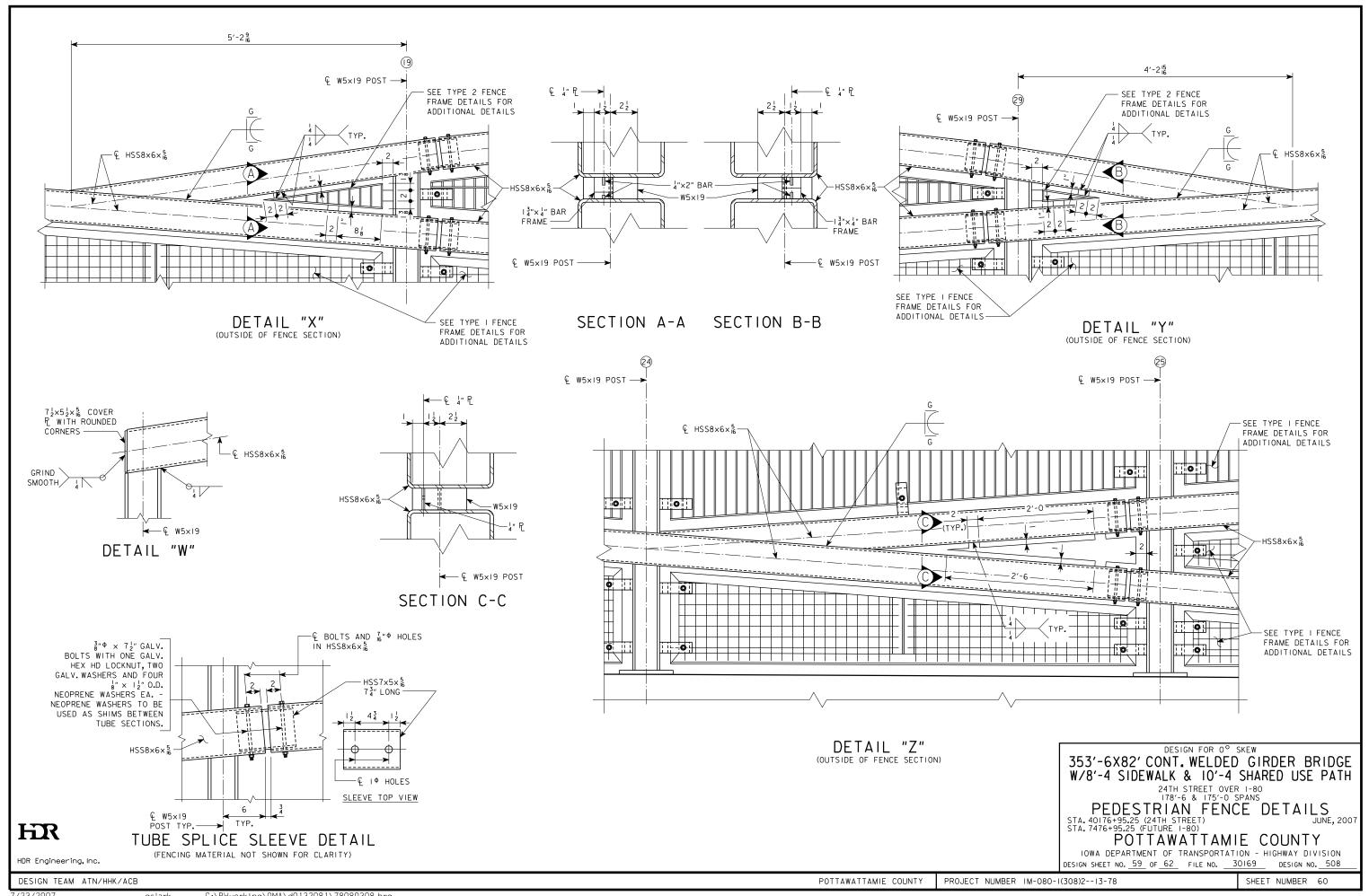
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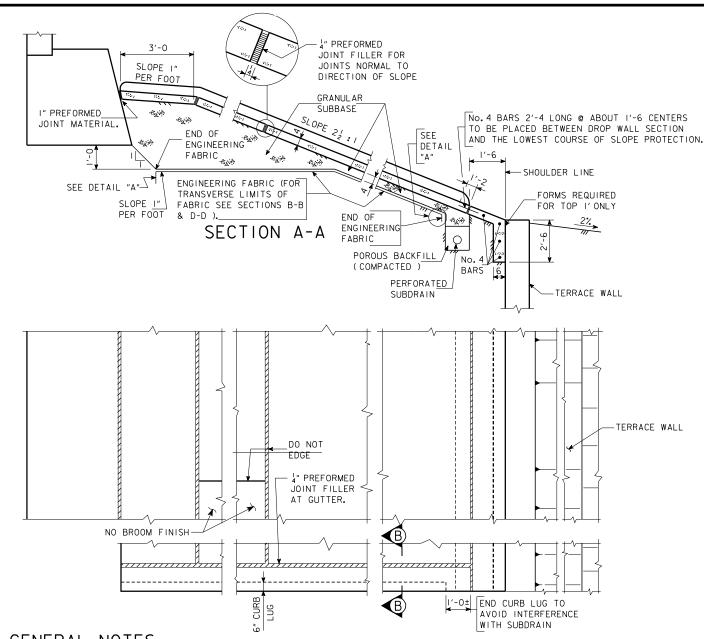


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GENERAL NOTES:

THIS PLAN SHEET SHOWS DETAILS FOR PLACING A PORTLAND CEMENT CONCRETE SLOPE PROTECTION UNDER OVERHEAD STRUCTURES.

THE CURRENT SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION SHALL APPLY WITH MODIFICATIONS OR ADDITIONS LISTED BELOW :

FINISH - CLASS I, FLOATED SURFACE FINISH.

CURE - CURE AS PER CURRENT SPECIFICATIONS.

GRANULAR SUBBASE - THIS PREWETTED MATERIAL SHALL BE DEPOSITED BY A METHOD APPROVED BY THE ENGINEER AND BE THOROUGHLY TAMPED OR VIBRATED TO INSURE COMPACTION, FINISHED SHAPE SHALL BE AS SHOWN IN SECTION A-A.

FORESLOPE PREPARATION - THE BRIDGE BERM FORESLOPE SHALL BE COMPACTED AND SHAPED AS SHOWN IN SECTION A-A ON THIS SHEET. THE BERM FORESLOPE SHALL BE FIRM WHEN THE ENGINEERING FABRIC AND GRANULAR SUBBASE ARE PLACED.

ENGINEERING FABRIC SHALL MEET REQUIREMENTS OF 4196.01 B. IF THE ENGINEERING FABRIC IS LAPPED, THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.

HR

HDR Engineering, Inc.

DESIGN TEAM RRP/JPS/ACB

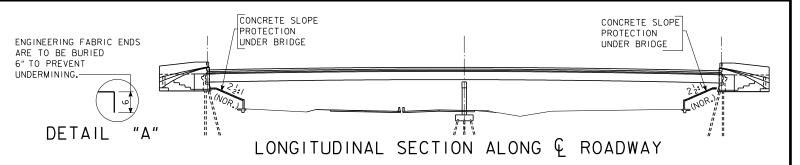
THE CAST IN PLACE CONCRETE IS TO BE POURED IN APPROXIMATELY 10' WIDE COURSES, BUT ALL COURSES ON ONE SLOPE SHOULD HAVE APPROXIMATELY EQUAL WIDTHS. ADJACENT COURSES SHALL NOT BE POURED WITHIN 15 HOURS OF ONE ANOTHER. THE JOINTS IN THE DIRECTION OF THE SLOPE ARE TO BE STAGGERED ABOUT | BLOCK WIDTH.

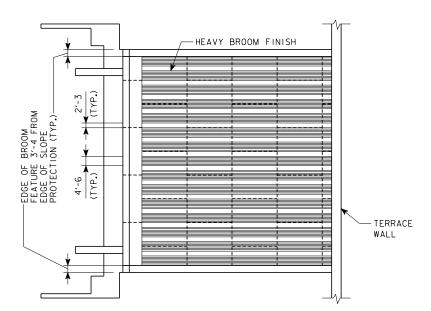
PAYMENT FOR "CONCRETE SLOPE PROTECTION" WILL BE MADE ON A SQUARE YARD BASIS FOR SLOPE PROTECTION CONSTRUCTED. THE UNIT PRICE BID PER SQUARE YARD IS TO INCLUDE COSTS OF ALL MATERIALS AND LABOR REQUIRED TO CONSTRUCT THE SLOPE PROTECTION AS SHOWN ON THESE PLANS. THE DISPOSAL OF EXCESS SOIL FROM SHAPING OR TRENCHING, AS DIRECTED BY THE ENGINEER, SHALL BE CONSIDERED INCIDENTAL TO PLACING THE CONCRETE SLOPE PROTECTION.

WHERE EROSION CONTROL WORK IS COMPLETED THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY PLANT MATERIALS DESTROYED ADJACENT TO SLOPE PROTECTION AREA. THE CONTRACTOR SHALL REPLANT, RESEED AND REMULCH ALL AREAS IN ACCORDANCE WITH SECTION 2601 OF THE CURRENT STANDARD SPECIFICATIONS, AT HIS EXPENSE.

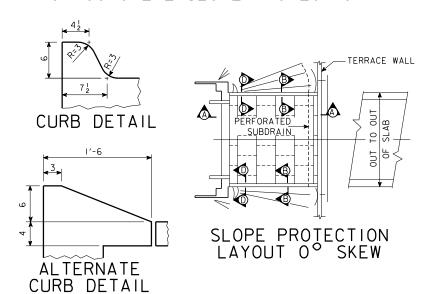
THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS AS DETAILED ON THE SUBDRAIN DETAILS SHEET.

THE HEAVY BROOM FINISH FEATURES SHALL BE PARALLEL TO THE BRIDGE DECK CENTERLINE, IN APPROXIMATELY EQUAL SPACED WIDTHS OF 4'-6, WITH CLEAR SPACINGS BEING HALF THE WIDTH OF THE BROOMED SECTIONS.





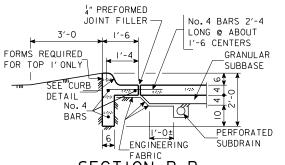
PLAN VIEW OF BROOM FINISH PATTERN ON CONCRETE SLOPE PROTECTION



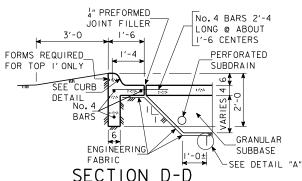
THE INTENT OF THE ARCHITECTURAL BROOM FINISH IS TO CREATE A HIGHLY VISUAL AND TEXTURAL CONTRAST BETWEEN THE BROOMED AND FLOATED CONCRETE SURFACES.

THE BROOM FINISH SHALL CONSIST OF TWO OR MORE PASSES WITH A PROVIDE A ROUGH TEXTURE, OF APPROXIMATELY 4 INCH IN DEPTH, BUT SHALL NOT RESULT IN EXPÓSURE OR DISLOCATION OF THE COARSE AGGREGATE IN THE CONCRETE.

FINISHED SHALL BE CLASS I, FLOATED SURFACE FINISH.



SECTION B-B (THROUGH 4" THICK GRANULAR SUBBASE)



(THROUGH VARIABLE THICKNESS GRANULAR SUBBASE)

ESTIMATED	QUANTIT	IES
DESCRIPTION	LOCATION	QUANTITY
CONCRETE SLOPE PROTECTION	SOUTH ABUT.	308 SQ.YDS.
CONCRETE SLOPE PROTECTION	NORTH ABUT.	264 SQ.YDS.
	TOTAL	572 SQ.YDS.

ITEMS TO BE INCLUDED IN "CONCRETE SLOPE PROTECTION": ENGINEERING FABRIC

GRANULAR SUBBASE CLASS "C" STRUCTURAL CONCRETE

#4 REINFORCING

PREFORMED JOINT FILLER EXCAVATION, SHAPING AND COMPACTING

COMMERCIAL BITUMINOUS PATCHING MATERIAL.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

CONCRETE SLOPE PROTECTION

STA. 7476+95.25 POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 60 OF 62 FILE NO. 30169 DESIGN NO. 508

STIFF FINISHING BROOM OR RAKE. BROOMING SHALL BE SUFFICIENT TO

THE PORTIONS OF THE CONCRETE SLOPE PROTECTION NOT BROOM

CONCRETE SLOPE PROTECTION (STUB ABUTMENTS)

STANDARD SHEET (MODIFIED)

POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-I(308)2--13-78

ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

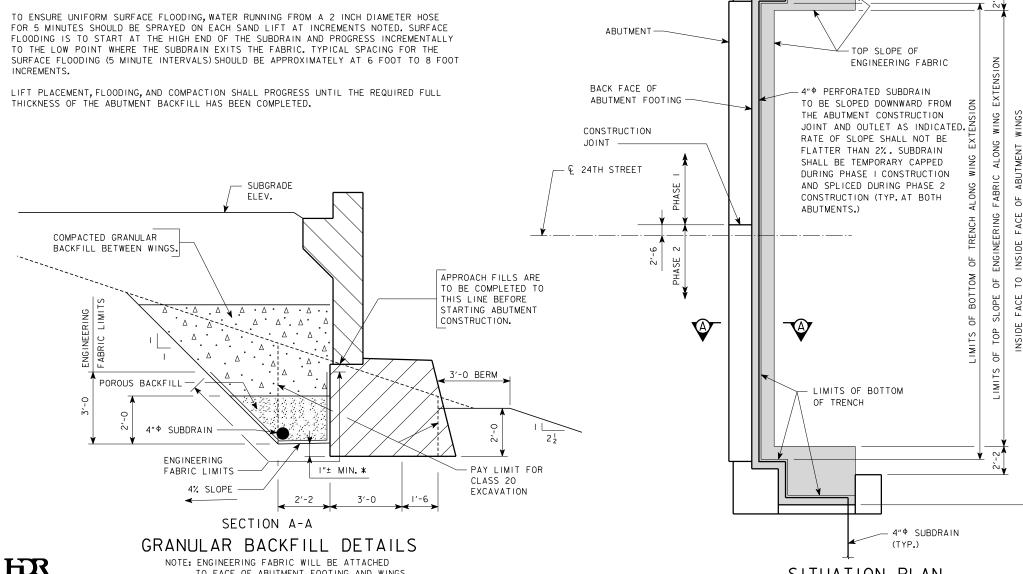
AFTER THE SUBGRADE HAS BEEN SHAPED THE GEOTEXTILE FABRIC IS TO INSTALLED IN ACCORDANCE WITH THE DETAILS SHOWN. THE FABRIC IS INTENDED TO BE INSTALLED IN THE BASE OF THE EXCAVATION AND EXTENDED VERTICALLY UP THE ABUTMENT BACKWALL, ABUTMENT WING WALLS, AND EXCAVATION FACE TO A HEIGHT THAT WILL BE APPROXIMATELY I FOOT HIGHER THAN THE HEIGHT OF THE POROUS BACKFILL PLACEMENT. THE STRIPS OF THE FABRIC PLACED SHALL OVERLAP APPROXIMATELY I FOOT AND SHALL BE PINNED IN PLACE. THE FABRIC SHALL BE ATTACHED TO THE ABUTMENT BY USING LATH FOLDED IN THE FABRIC AND SECURED TO THE CONCRETE WITH SHALLOW CONCRETE NAILS. THE FABRIC PLACED AGAINST THE EXCAVATION FACE SHALL BE PINNED.

WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN IS TO BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

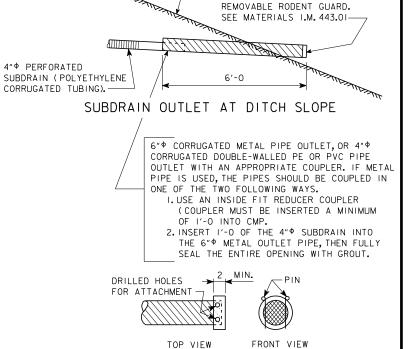
THE REMAINING WORK INVOLVES BACKFILLING WITH GRANULAR BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE GRANULAR BACKFILL MATERIAL SHALL HAVE 4% OR LESS PASSING THE #200 SIEVE (I.E. WASHED CONCRETE SAND). THE GRANULAR BACKFILL WILL REQUIRE PLACEMENT IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND THEN FOLLOWED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LÍMIT THE LOOSE LIFTS TO NO MORE THAN 2 FOOT OF THICKNESS.

FOR 5 MINUTES SHOULD BE SPRAYED ON EACH SAND LIFT AT INCREMENTS NOTED. SURFACE FLOODING IS TO START AT THE HIGH END OF THE SUBDRAIN AND PROGRESS INCREMENTALLY TO THE LOW POINT WHERE THE SUBDRAIN EXITS THE FABRIC. TYPICAL SPACING FOR THE SURFACE FLOODING (5 MINUTE INTERVALS) SHOULD BE APPROXIMATELY AT 6 FOOT TO 8 FOOT INCREMENTS.



ABUTMENT WING (TYP.)

TECHNICAL DATA INFORMATION - GEOTEXTILE FABRIC MINIMUM AVERAGE ROLL VALUE MECHANICAL PROPERTIES TEST METHOD UNIT CD TENSILE STRENGTH (AT 5% STRAIN) | ASTM D 4595 | kN/m (LBS/FT) | 19.8 (1356) | 19.8 (1356) PERMEABILITY ASTM D 4491 CM/SEC 0.038 L/MIN/m² 733 FLOW RATE ASTM D 4491 (GAL/MIN/FT²) (18) % STRENGTH UV RESISTANCE (AT 500 HOURS) ASTM D 4355 70 RETAINED



DITCH SL OPF

LINE -

REMOVABLE RODENT GUARD DETAILS OUTLET DETAILS

SUBDRAIN NOTES:

THIS PLAN SHEET SHOWS DETAILS FOR PLACING ALL SUBDRAINS AND SUBDRAIN OUTLETS REQUIRED FOR THIS STRUCTURE.

THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS AROUND THE ABUTMENT AS DETAILED ON THIS SHEET. THE SUBDRAINS SHALL BE 4" IN DIAMETER AND MEET THE REQUIREMENTS OF SECTION 4143.01 B OF THE CURRENT I.D.O.T. STANDARD SPECIFICATION.

THE SUBDRAIN OUTLET SHALL CONSIST OF A 6'-O LENGTH OF PIPE WITH A REMOVABLE RODENT GUARD AS DETAILED ON THIS SHEET.

THE COST OF FURNISHING AND PLACING SUBDRAINS (INCLUDING EXCAVATION), GRANULAR BACKFILL, POROUS BACKFILL, AND SUBDRAIN OUTLET IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)". NO EXTRA PAYMENT WILL

THE DIMENSIONS SHOWN FOR THE PROPOSED SUBDRAINS ARE BASED ON THE PROPOSED GRADING LAYOUT OF BRIDGE BERMS. THE DIMENSIONS SHOWN ARE FOR ESTIMATING ONLY. REQUIRED LENGTHS AND GENERAL LOCATIONS OF SUBDRAINS ARE SUBJECT TO CHANGE DUE TO FIELD ADJUSTMENTS OF THE GRADING LAYOUT.

DESIGN FOR O° SKEW

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80 178'-6 & 175'-0 SPANS

ABUTMENT BACKFILL AND SUBDRAIN DETAILS

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 61 OF 62 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM RRP/JPS/DHS

POTTAWATTAMIE COUNTY

SITUATION PLAN

SHOWING SUBDRAIN LOCATIONS

SUBDRAIN OUTLET

TO DAYLIGHT (TYP.)

PROJECT NUMBER IM-080-I(308)2--13-78

SHEET NUMBER 62

HDR Engineering, Inc.

* DIMENSION VARIES DUE

TO 2% SUBDRAIN SLOPE.

TO FACE OF ABUTMENT FOOTING AND WINGS.

COLORED SEALER COATINGS NOTES:

PRIOR TO BEGINNING ANY CONCRETE SEALER COATING WORK ON THE ACTUAL PROJECT, THE MOCKUP PANEL CREATED TO DEMONSTRATE THE SIMULATED STONE TEXTURES ON THE PIERS SHALL BE USED TO DEMONSTRATE THE PREPARATION, APPLICATION METHODS AND RESULTS FOR CONCRETE STAINING. BEGIN WORK ON THE ACTUAL COATING APPLICATION ONLY AFTER FINAL APPROVAL BY THE ENGINEER.

ALL CONCRETE SURFACES NOTED AND SHOWN IN ON THIS SHEET WILL RECEIVE APPLICATION OF COLORED SEALER COATING. CONCRETE COATING WORK SHALL BE IN ACCORDANCE WITH THE DEVELOPMENTAL SPECIFICATION, "COLORED SEALER COATING FOR STRUCTURAL CONCRETE".

THERE ARE THREE COLORS OF CONCRETE SEALER COATING TO BE USED ON THE PIERS AND ONE COLOR TO BE USED ON THE ABUTMENTS. IN ADDITION, THE SIMULATED STONE TEXTURES ON THE PIERS WILL RECEIVE A THREE-COAT SYSTEM OF PENETRATING STAIN TO SIMULATE THE NATURAL TONES AND VARIATIONS OF THE STONE. APPEARANCE OF THE SIMULATED STONE TEXTURE IS INTENDED TO CLOSELY MATCH THE STONE VENEER USED ON THE ABUTMENT WINGS. SEE DETAILS ON THIS DESIGN SHEET FOR SPECIFIC COLOR LOCATIONS AND LIMITS. "COLOR NO. I" SHALL BE A LIGHT BUFF COLOR, "COLOR NO. 2" AND "COLOR NO. 3" SHALL BE IN THE SAME COLOR FAMILY AS "COLOR NO. I" WITH INCREASINGLY DARKER HUES. "COLOR NO. 4" SHALL BE A FULL RANGE OF NATURAL STONE COLORS WITH A FIRST COAT COLOR OF LIGHT OR MEDIUM BUFF AND INCLUDING SUBTLE COLOR VARIATIONS, MINERAL OXIDATION AND STAINING. THE FINAL COLORATION OF THE CONCRETE SURFACE SHALL ACCURATELY SIMULATE THE APPEARANCE OF REAL STONE INCLUDING THE MULTIPLE COLORS, SHADES, FLECKING, AND VEINING THAT ARE APPARENT IN REAL LIMESTONE. USE AT LEAST THREE COLOR SHADES TO SIMULATE THE APPEARANCE OF STONE. BEGIN WITH A BASE COLOR APPLICATION OF LIGHT BUFF. APPLY A SLIGHTLY LIGHTER OR DARKER BASE COLOR TO RANDOM STONES PRIOR TO ADDING THE COLOR VARIATIONS. WHEN ALL STONE COLORS HAVE BEEN APPLIED, APPLY A LIGHT GREY COLOR TO THE SIMULATED MORTARED JOINTS. THE GREY COLOR SHALL MATCH OR BE SLIGHTLY LIGHTER THAN THE PLAIN CONCRETE SURFACES OF THE STRUCTURE. APPLY JOINT COLOR NEATLY AND ONLY TO THE BOTTOM SURFACE OF SIMULATED JOINTS. SUBMIT PRODUCT SPECIFICATION SHEETS AND COATED CONCRETE SAMPLES AS DESCRIBED IN THE DEVELOPMENTAL

MASK ADJACENT CONCRETE SURFACES THAT WILL NOT RECEIVE COATING. NO OVERSPRAY OR CONTAMINATION OF ADJACENT SURFACES IS ALLOWED.

AFTER ALL COLORED SEALER COATING WORK IS COMPLETED, THE TEXTURE MOCKUP PANEL SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND BE REMOVED FROM THE SITE.

COLORED SEALER COATING SURFACE AREA TABULATION:

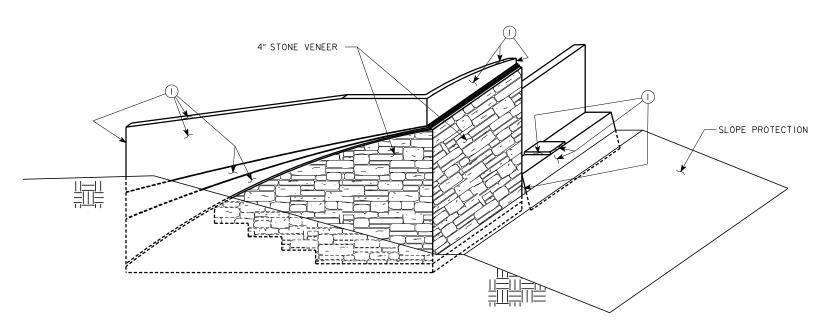
COLOR NO. 1
PIER 277.6 SY
ABUTMENTS 285.3 SY
SUPERSTRUCTURE 908.3 SY

COLOR NO. 3 PIER 6.2 SY

COLOR NO. 4
SIMULATED STONE ON PIER 228.0 SY

TOTAL 1711.0 SY

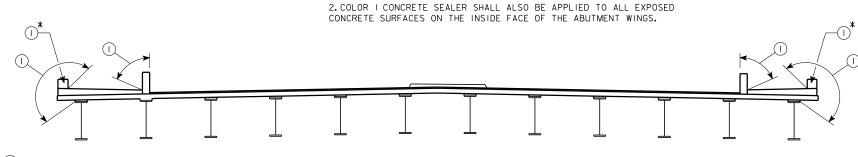
ALL COSTS ASSOCIATED WITH CONCRETE SEALER COATING ARE TO BE INCLUDED IN THE BID ITEM "COLORED SEALER COATING FOR STRUCTURAL CONCRETE".

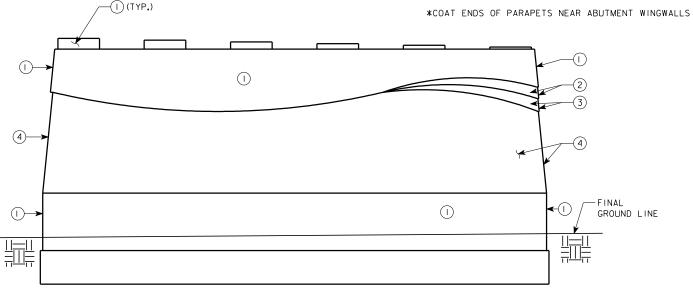


ABUTMENT WINGS

(OUTSIDE)

NOTES:
I.LIMITS OF COLOR SEALER COAT FOR CONCRETE SHALL EXTEND TO A
MINIMUM OF SIX INCHES BELOW FINISHED GRADE.





CONCRETE SEALER COATING SHALL EXTEND TO CONCRETE SURFACES ON EACH END OF THE PIERS, EXTEND APPLICATION OF SEALER TO A MINIMUM OF 6" BELOW FINISHED GRADE.

DESIGN FOR O° SKEW

(X) = COLOR NUMBER

LEGEND:

353'-6X82' CONT. WELDED GIRDER BRIDGE W/8'-4 SIDEWALK & 10'-4 SHARED USE PATH

24TH STREET OVER I-80

LIMITS OF CONCRETE STAINING

STA. 40176+95.25 (24TH STREET) STA. 7476+95.25 (FUTURE 1-80)

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 62 OF 62 FILE NO. 30169 DESIGN NO. 508

HR

HDR Engineering, Inc.

nuk Engineering, inc.

DESIGN TEAM PER/HHK/ACB

C:\PWworking\OMA\d0132081\78080308.brg

POTTAWATTAMIE COUNTY PROJECT NUMBER IM-080-I(308)2--13-78

SPECIFICATIONS:

AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2002.

CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2001, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, INCLUDING SUPPLEMENTAL SPECIFICATIONS FOR "COLORED SEALER FOR STRUCTURAL

CONCRETE"

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2002. REINFORCING STEEL IN ACCORDANCE WITH SECTION 8, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 8, f'c = 3,500 psi.

GENERAL NOTES:

THIS DESIGN IS FOR THE CONSTRUCTION OF SIX NEW CAST IN PLACE RETAINING WALLS ALONG THE NORTH SIDE AND SOUTH SIDE OF 1-80 AT 24TH STREET.

THE RETAINING WALLS ARE DESIGNED FOR AN ACTIVE EARTH PRESSURE OF 60 psf. THE ALLOWABLE SOIL BEARING IS 2,500 psf. THE SOIL FRICTION ANGLE = 30 DEGREES. THE CITY AND UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE CONTRACTOR OF THE CONSTRUCTION STARTING DATE.

AQUILA GAS LINE IS LOCATED AT APPROXIMATELY 1.5 FEET BELOW THE BOTTOM OF THE CONCRETE FOOTING SHEAR KEY FOR RETAINING WALLS NO & SO. THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION ACTIVITIES WITH THE UTILITY PRIOR TO ANY CONSTRUCTION ACTIVITY ASSOCIATED WITH THE WALLS.

THE CONTRACTOR IS ENCOURAGED TO TAKE FULL ADVANTAGE OF SPECIFICATION 1105.15 VALUE ENGINEERING INCENTIVE PROPOSAL. A PAMPHLET AND CONCEPTUAL PROPOSAL FORM WILL BE AVAILABLE AT THE PRECONSTRUCTION CONFERENCE.

COPIES OF THE ORIGINAL DESIGN PLANS AND SHOP DRAWINGS OF THE EXISTING BRIDGE WILL BE AVAILABLE TO THE CONTRACTOR. CONTACT THE OFFICE OF CONTRACTS-HIGHWAY DIVISION-IOWA D.O.T.-AMES.

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE SITES FOR EXCESS EXCAVATED MATERIAL, NO PAYMENT FOR OVERHAUL WILL BE ALLOWED FOR MATERIAL HAULED TO THESE SITES.

ALL STRUCTURAL CONCRETE TO BE CLASS "C".

EXCAVATION OR EMBANKMENT QUANTITIES ARE INCLUDED IN THE ROADWAY PLANS ASSOCIATED WITH THIS CONTRACT.

ALL COARSE AGGREGATE FOR STRUCTURAL CONCRETE SHALL BE CRUSHED LIMESTONE. SPECIAL BACKFILL SHALL BE IN ACCORDANCE WITH SECTION 4132.01 OF THE STANDARD SPECIFICATIONS EXCEPT UNCLASSIFIED RECLAIMED HOT MIX ASPHALT (HMA) SHALL NOT BE ALLOWED IN THE GRADATION.

FAINT LINES INDICATE THE EXISTING BRIDGE.

STORM DRAINAGE DETAILS AND QUANTITIES ARE NOT A PART OF THIS PROJECT BUT ARE INCLUDED IN THE ROADWAY PLANS ASSOCIATED WITH THIS CONTRACT.

GRADING PLANS AND QUANTITIES ARE NOT A PART OF THIS PROJECT BUT ARE INCLUDED IN THE ROADWAY PLANS ASSOCIATED WITH THIS CONTRACT.

THE NEW 24TH BRIDGE (DESIGN NO. 508) IS BEING CONSTRUCTED IN PHASES. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER, THE WALL CONSTRUCTION SCHEDULE AND CONSTRUCTION METHOD TAKING INTO ACCOUNT THE BRIDGE CONSTRUCTION. EXCAVATION FOR THE WALLS SHALL NOT IMPACT ANY PART OF THE NEWLY CONSTRUCTED BRIDGE OR ANY PART OF THE EXISTING BRIDGE CARRYING TRAFFIC. IF TEMPORARY SHORING (SHEET PILE OR OTHER) IS NECESSARY TO MAINTAIN STABILITY OF THE NEW STRUCTURE AND/OR EXISTING STRUCTURE CARRYING TRAFFIC. THE CONTRACTOR WILL SUBMIT A SHORING PLAN TO THE ENGINEER FOR APPROVAL. COST OF SHORING, IF REQUIRED, WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION AND NO DIRECT PAYMENT WILL BE MADE. ALL MATERIAL USED FOR SHORING SHALL REMAIN THE PROPERTY OF THE CONTRACTOR. SHORING IS TO BE REMOVED ONLY AFTER BACK FILLING HAS BEEN COMPLETED.

TEMPORARY SHORING, IF ANY, OF THE CIP WALLS SHALL REMAIN IN PLACE UNTIL 1/3 OF THE TOTAL SOIL HEIGHT IS PLACED TO PREVENT OVERTURNING.

ALL ELEVATIONS ON THESE PLANS SHOWN IN FEET.

ALL STATIONS SHOWN IN FEET.

ALL COSTS ASSOCIATED WITH EXPANSION AND CONTROL JOINTS SHALL BE CONSIDERED INCIDENTAL TO THE BID ITEM "STRUCTURAL CONCRETE (MISCELLANEOUS)".

WORK PERFORMED TO CREATE TEXTURED CONCRETE SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR FORMWORK AND THE FOLLOWING:

FORM THE TEXTURED CONCRETE SURFACE USING A FORM LINER SYSTEM MADE OF HIGH-STRENGTH URETHANE ELASTOMER OR FLEXIBLE FOAM MATERIALS CAPABLE OF WITHSTANDING ANTICIPATED CONCRETE POUR PRESSURES WITHOUT LEAKAGE OR CAUSING PHYSICAL DEFECTS. FORM LINERS SHALL EASILY ATTACH TO FORMS AND BE REMOVABLE WITHOUT CAUSING CONCRETE SURFACE DAMAGE. IF RECOMMENDED BY THE FORM LINER MANUFACTURER, USE STRUCTURAL BACKERS TO PREVENT DEFORMATION OF THE LINER DURING LOADING OF THE FORMS. THE LINERS SHALL BE DESIGNED TO FORM SURFACES CONFORMING TO THE DESIGN INTENT INCLUDING THE SHAPE, LINES AND DIMENSIONS SHOWN IN THE PLANS AND TO AVOID VISIBLE PATTERN REPEATS. MATCH PATTERN FEATURES AT FORM LINER JOINTS TO MINIMIZE PATTERN REPEATS AND MAKE THE FORMED CONCRETE SURFACE APPEAR UNIFORM AND CONTINUOUS WITHOUT VISIBLE SEAMS AND FORM MARKS. WHEN JOINTS ARE UNAVOIDABLE, MAKE JOINTS ALONG MAIN FEATURES OF THE PATTERN IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

FORM LINER EDGES FOLLOWING CURVES ARE TO BE CUT CLEANLY AND PARALLEL TO THE CURVE. USE ADEQUATE BLOCKING, SEALING AND OTHER MEANS IN ORDER TO MAINTAIN THE APPROPRIATE DEPTH AND CHARACTER OF TEXTURE AT CUT EDGES OF LINERS AND TO PREVENT MORTAR LEAKAGE.

LAY OUT INDIVIDUAL SIMULATED STONE LINERS WITHIN FORMS SO THAT NO VERTICAL MORTAR JOINTS ARE ALIGNED ON ADJACENT COURSES. DO NOT MIX FORM LINERS FROM DIFFERENT MANUFACTURERS WHEN FORMING THE TEXTURED CONCRETE ON THIS PROJECT.

DURING LOADING OF FORMS WITH CONCRETE, TAKE EXTRA CARE TO ADEQUATELY VIBRATE CONCRETE IN ORDER TO MAINTAIN ALL INTENDED FEATURES OF THE FORM LINER IN THE FINAL SURFACE AND TO PREVENT VOIDS. FOLLOWING REMOVAL OF FORMS, FINISH MINOR DEFECTS TO BLEND WITH THE BALANCE OF THE SURFACE TEXTURE. THE COMPLETED SURFACE SHALL BE FREE OF BLEMISHES, SURFACE VOIDS AND CONSPICUOUS FORM MARKS TO THE SATISFACTION OF THE ENGINEER. THE CONTRACTOR SHALL CORRECT, AT HIS OWN COST, ANY SURFACE DEFECTS.

VERIFY THAT RELEASE AGENTS USED ARE COMPATIBLE WITH FORM LINER MATERIAL, AND ARE NON-STAINING, APPLY RELEASE AGENT IN ACCORDANCE WITH THE FORM LINER MANUFACTURER*S RECOMMENDATIONS. RELEASE AGENTS MUST ALSO BE COMPATIBLE WITH

THE PROPOSED CONCRETE STAINS TO BE USED TO COLOR THE CONCRETE.

IF USED, FORM TIES SHALL BE MADE OF NON-CORROSIVE MATERIALS WHEN THE PORTION PERMANENTLY EMBEDDED IN THE CONCRETE IS LESS THAN 1-1/2 INCHES FROM THE FINISHED SURFACE. POSITION FORM TIES AND ACCESSORIES IN STONE PATTERN MORTAR JOINTS AND AT HIGH POINTS OF FINISHED WALL.

STRIP FORMWORK IN ACCORDANCE WITH LINER MANUFACTURER*S RECOMMENDATIONS AFTER THE CONCRETE HAS SUFFICIENT STRENGTH TO AVOID SURFACE DAMAGE. CLEAN AND REPAIR FORM LINER SURFACES PRIOR TO REUSE. DO NOT USE SPLIT, FRAYED, DELAMINATED OR OTHERWISE DAMAGED FORM LINERS.

CONSTRUCT A 4-FOOT HIGH, BY 10-INCH WIDE (MIN.), BY 8-FOOT LONG MOCKUP PANEL IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND THESE PLANS. CAST THE MOCKUP PANEL ON SITE, USING THE SAME FORMING METHODS, PROCEDURES, FORM LINER, AND CONCRETE MIXTURE PROPOSED FOR THE PRODUCTION WORK. TEXTURED FACE SHALL BE VERTICAL DURING THE CASTING PROCESS. A SINGLE MAT OF NO.5 REINFORCING BARS IN TWO DIRECTIONS SHALL BE SET 2 INCHES CLEAR TO THE BOTTOM OF THE TEXTURED FACE. IF THE MOCKUP PANEL IS REJECTED, CONSTRUCT A NEW MOCKUP PANEL AS DIRECTED BY THE ENGINEER. BEGIN CONCRETE TEXTURE PRODUCTION WORK ONLY AFTER THE MOCKUP HAS BEEN APPROVED BY THE ENGINEER.

AFTER CURING FOR A MINIMUM OF 28 DAYS, THE MOCKUP PANEL WILL ALSO BE USED TO DEMONSTRATE THE COLORED SEALER COATING APPLICATION. SEE NOTES ON RETAINING WALL DETAIL SHEET FOR FURTHER INFORMATION REGARDING COLORED SEALER.

ALL COSTS ASSOCIATED WITH CONCRETE TEXTURING AND FORM LINERS INCLUDING CONSTRUCTING AND REMOVING THE MOCKUP PANEL ARE TO BE INCLUDED IN THE BID ITEM, "STRUCTURAL CONCRETE (MISCELLANEOUS)".

STONE FORM LINER FOR TERRACE WALLS SHALL SIMULATE A PATTERN OF SMALL ASHLAR STONES WITH SAWED EDGES ON ALL FOUR SIDES AND A SPLIT OR SNAPPED EXPOSED FACE. STONE SIZES SHALL RANGE FROM 3" X 6" MINIMUM TO 14" X 28" MAXIMUM, MAXIMUM DEPTH OF RELIEF IN THE FORM LINER SHALL BE 12 AND JOINT WIDTH SHALL BE 2. THE FORMLINER
PATTERN AND MANUFACTURER USED FOR THE TERRACE WALLS SHALL MATCH THE FORM LINER USED FOR PIER NO. 1 OF BRIDGE DESIGN 508, SHOWN ELSEWHERE IN THESE PLANS. ACCEPTABLE PATTERNS AND MANUFACTURERS ARE:

PATTERN #12020-TOLLWAY ASHLAR CUSTOM ROCK 2020 W. 7TH STREET ST. PAUL, MN 55116 PATTERN #17000-FLORIDA ASHLAR FITZGERALD FORMLINER 1341 EAST PAMONA STREET

PATTERN #905-SMALL AGED ASHLAR STONE ARCHITECTURAL POLYMERS 1220 LITTLE GAP ROAD PALMERTON, PA 18071

PATTERN #1515-SC ASHLAR SPEC FORMLINERS, INC. 530 EAST DYER ROAD SANTA ANA, CA 92707

SANTA ANA, CA 92705

POLLUTION PREVENTION PLAN N IM_080_I (33 4) 2-I3-78

TRAFFIC CONTROL PLAN: SEE TRAFFIC CONTROL PLAN N IM_080_I (33 4) 2-I3-78

NOTE: ROADWAY QUANTITIES SHOWN IN IM_080_I (33 4) 2-I3-78

ESTIMATED RETAINING WALL QUANTITIES					
ITEM NO.	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUANTITY
I	2403-0100000	STRUCTURAL CONCRETE (MISCELLANEOUS)	CY	564.0	
2	2403-7302000	COLORED SEALER COAT-STRUCT CONC	SY	438.7	
3	2404-7775000	REINFORCING STEEL	LBS	36144	
4	2404-7775005	REINFORCING STEEL, EPOXY COATED	LBS	44082	

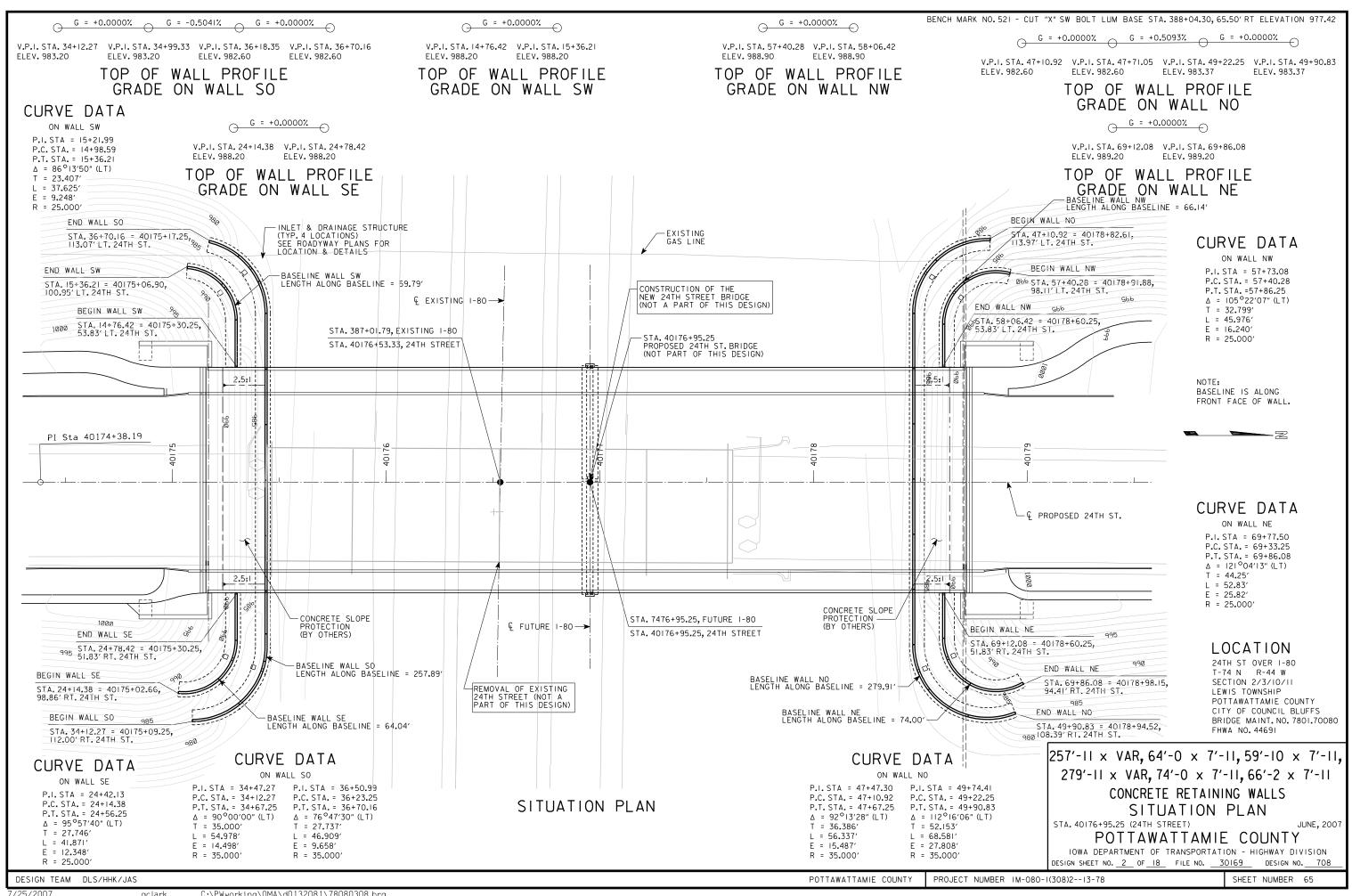
ITEM NO.	ESTIMATE REFERENCE INFORMATION
I	INCLUDES COST OF FURNISHING AND INSTALLING PREFORMED JOINT FILLER AND MATERIALS FOR EXPANSION AND CONTROL JOINTS. INCLUDES FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION) GRANULAR BACKFILL, POROUS BACKFILL AND SUBDRAIN OUTLETS AT RETAINING WALL. INCLUDES ALL COSTS ASSOCIATED WITH THE FORM LINER. 200.2 CY AT WALL NO, 44.9 CY AT WALL NW, 50.3 CY AT WALL NE, 184.4 CY AT WALL SO, 40.6 CY AT WALL SW, 43.6 CY AT WALL SE
2	INCLUDES ALL COSTS ASSOCIATED WITH FURNISHING AND APPLYING ANTI-GRAFFITI COATING.
3	12674 LBS AT WALL NO,2948 LBS AT WALL NW, 3256 LBS AT WALL NE, 11715 LBS AT WALL SO,2691 LBS AT WALL SW,2860 LBS AT WALL SE
4	15620 LBS AT WALL NO,3554 LBS AT WALL NW, 3914 LBS AT WALL NE, 14326 LBS AT SO WALL,3227 LBS AT WALL SW,3441 LBS AT WALL SE

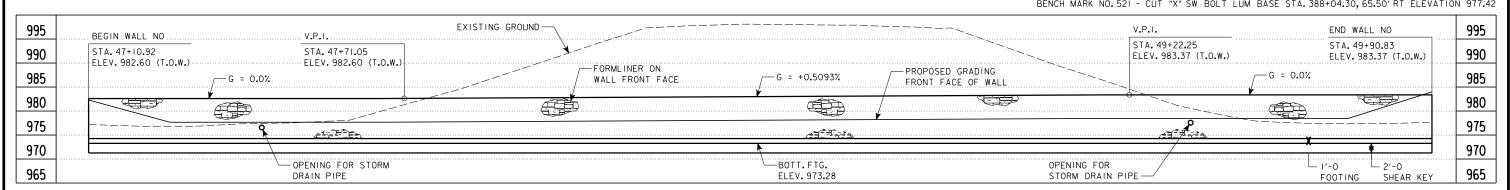
DES	SIGN HISTORY AT THIS SITE
DESIGN NO.	TYPE OF WORK
6665	ORIGINAL BRIDGE DESIGN
492	BEAM REPLACEMENT
696	REPAIR & OVERLAY
508	NEW 24TH STREET BRIDGE
708	TERRACE WALLS

 $|257'-11 \times VAR, 64'-0 \times 7'-11, 59'-10 \times 7'-11,$ $279'-11 \times VAR, 74'-0 \times 7'-11, 66'-2 \times 7'-11$ CONCRETE RETAINING WALLS GENERAL NOTES AND QUANTITIES STA. 40176+95.25 (24TH STREET) JUNE, 2007

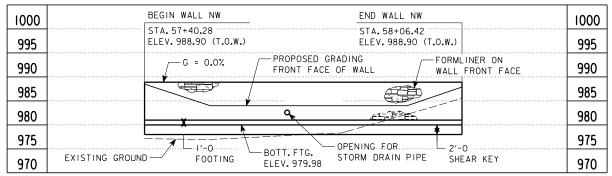
POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 1 OF 18 FILE NO. 30169 DESIGN NO. 708

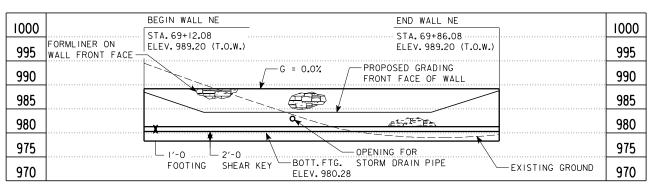




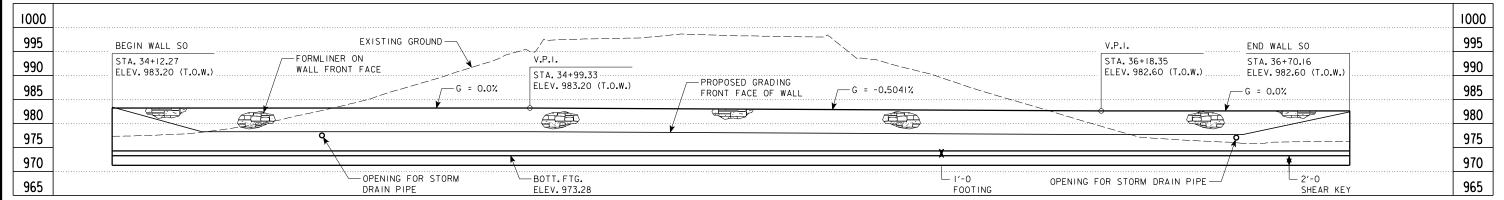
LONGITUDINAL SECTION ALONG B WALL NO



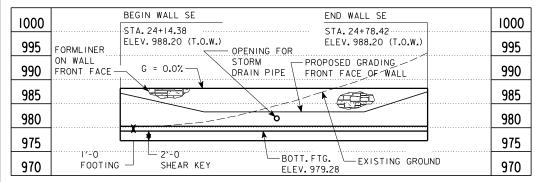
LONGITUDINAL SECTION ALONG B WALL NW



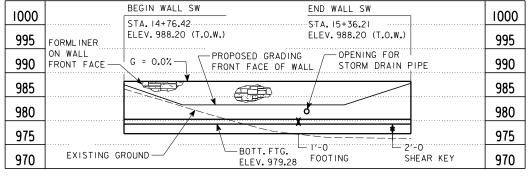
LONGITUDINAL SECTION ALONG B WALL NE



LONGITUDINAL SECTION ALONG B WALL SO



LONGITUDINAL SECTION ALONG B WALL SE



LONGITUDINAL SECTION ALONG B WALL SW

POTTAWATTAMIE COUNTY

FORMLINER PLACED ON WALL FRONT FACE FROM TOP OF WALL TO WITHIN APPROXIMATELY TWO INCHES FROM TOP OF FOOTING FROM BEGIN OF WALL TO END OF WALL (TYP.).

 $|257'-11 \times VAR, 64'-0 \times 7'-11, 59'-10 \times 7'-11,$ $279'-11 \times VAR, 74'-0 \times 7'-11, 66'-2 \times 7'-11$ CONCRETE RETAINING WALLS RETAINING WALL ELEVATIONS

STA. 40176+95.25 (24TH STREET)

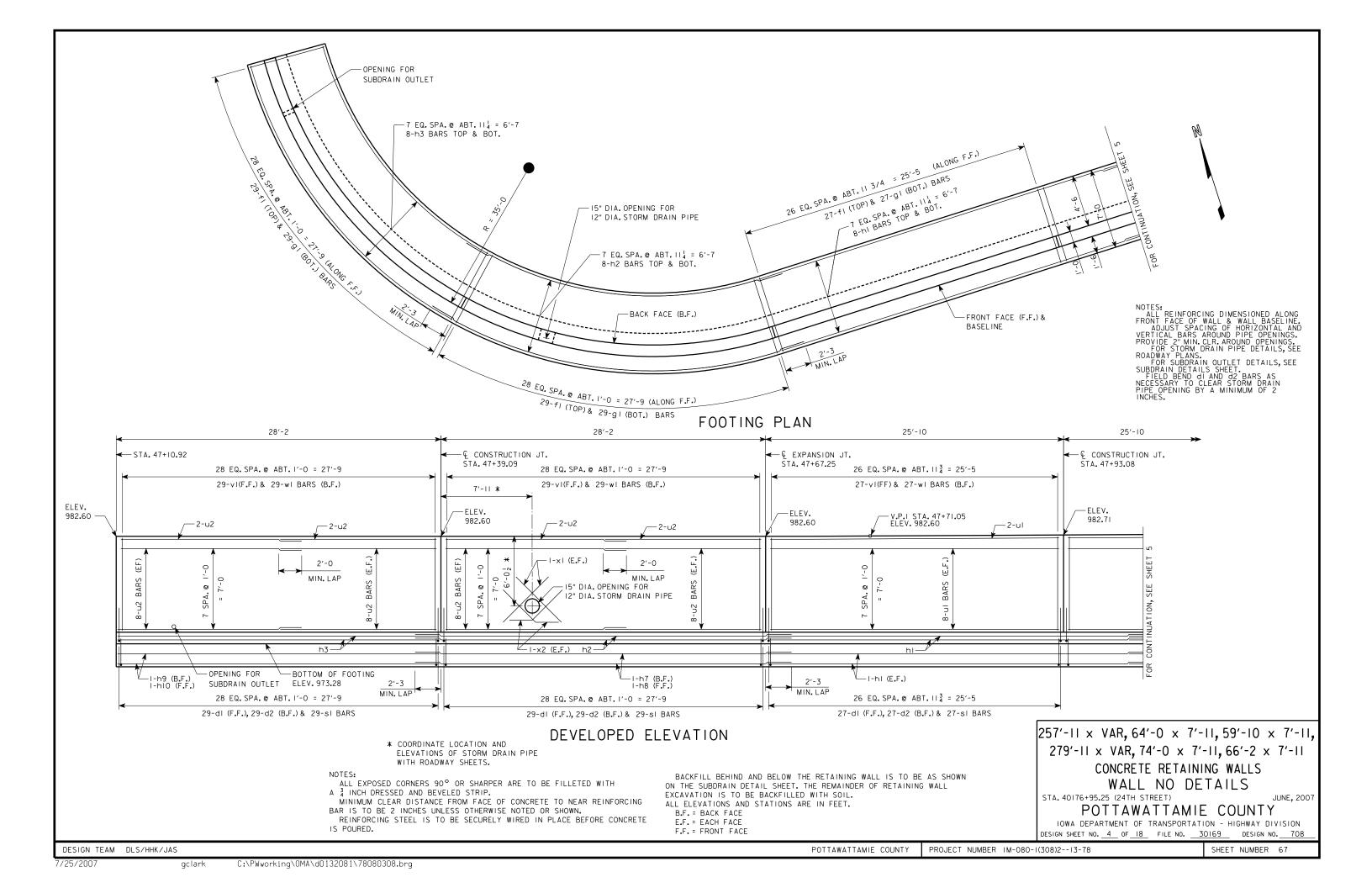
PROJECT NUMBER IM-080-I(308)2--I3-78

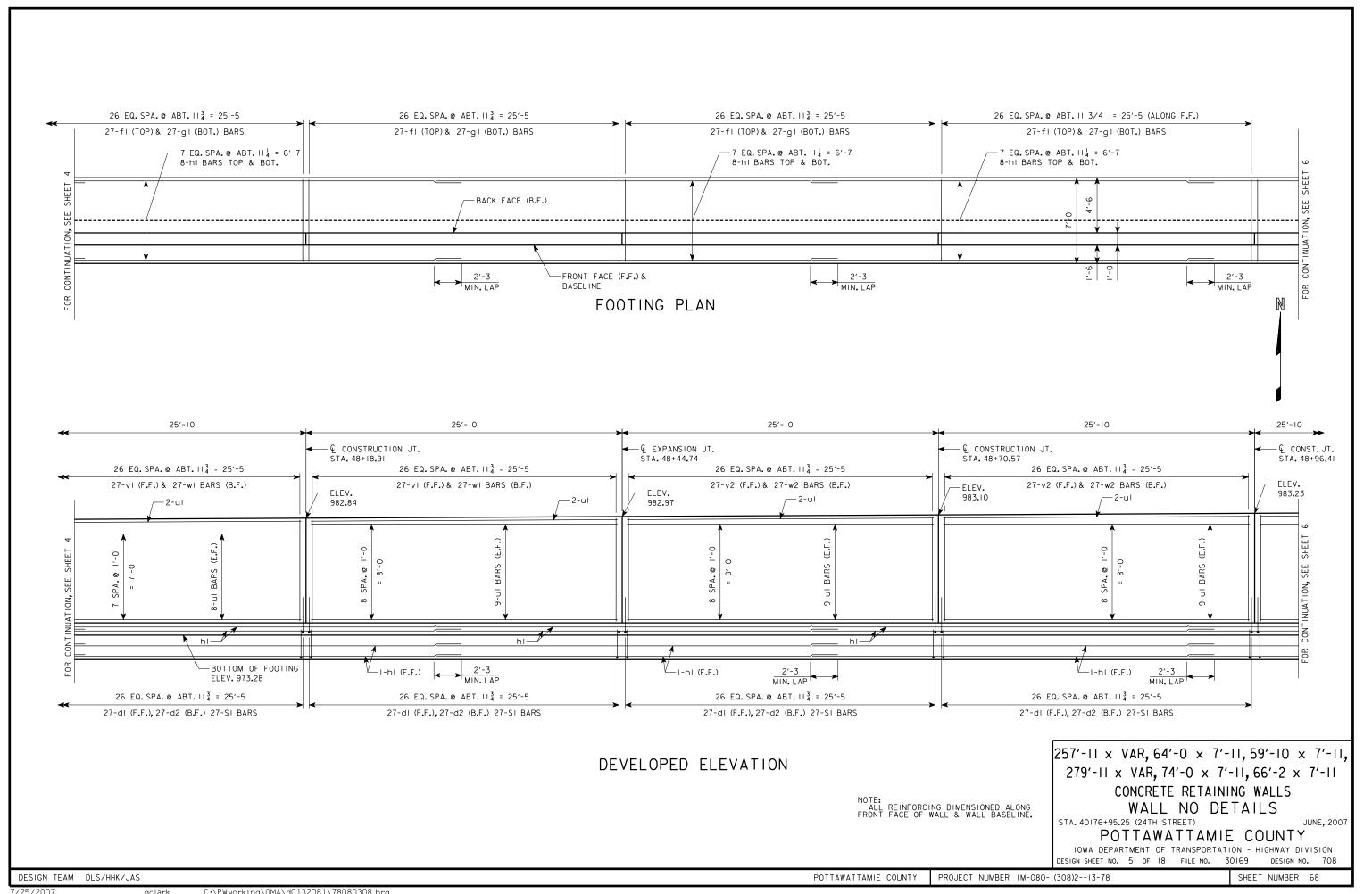
POTTAWATTAMIE COUNTY IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

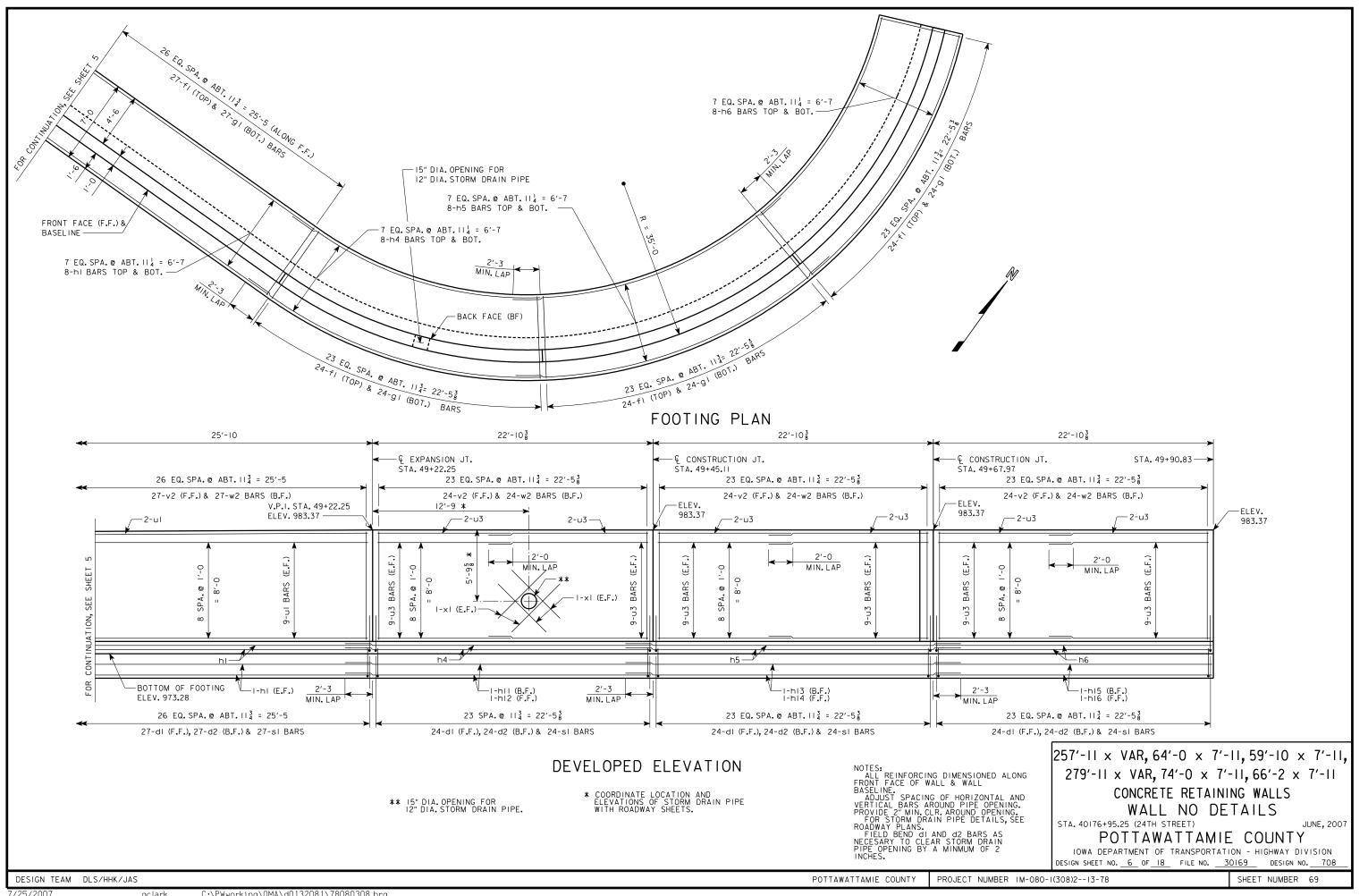
SHEET NUMBER 66

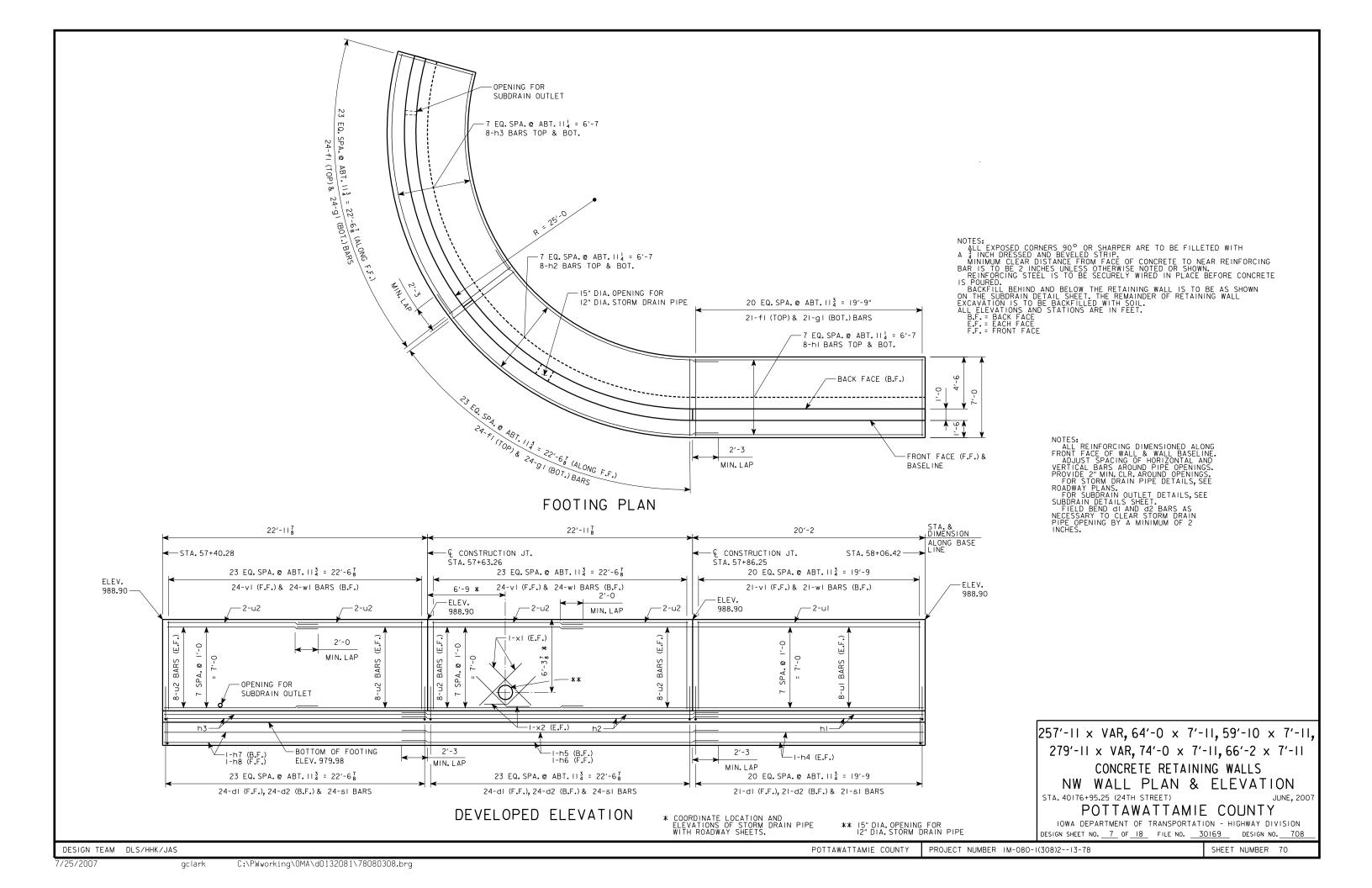
DESIGN SHEET NO. 3 OF 18 FILE NO. 30169 DESIGN NO. 708

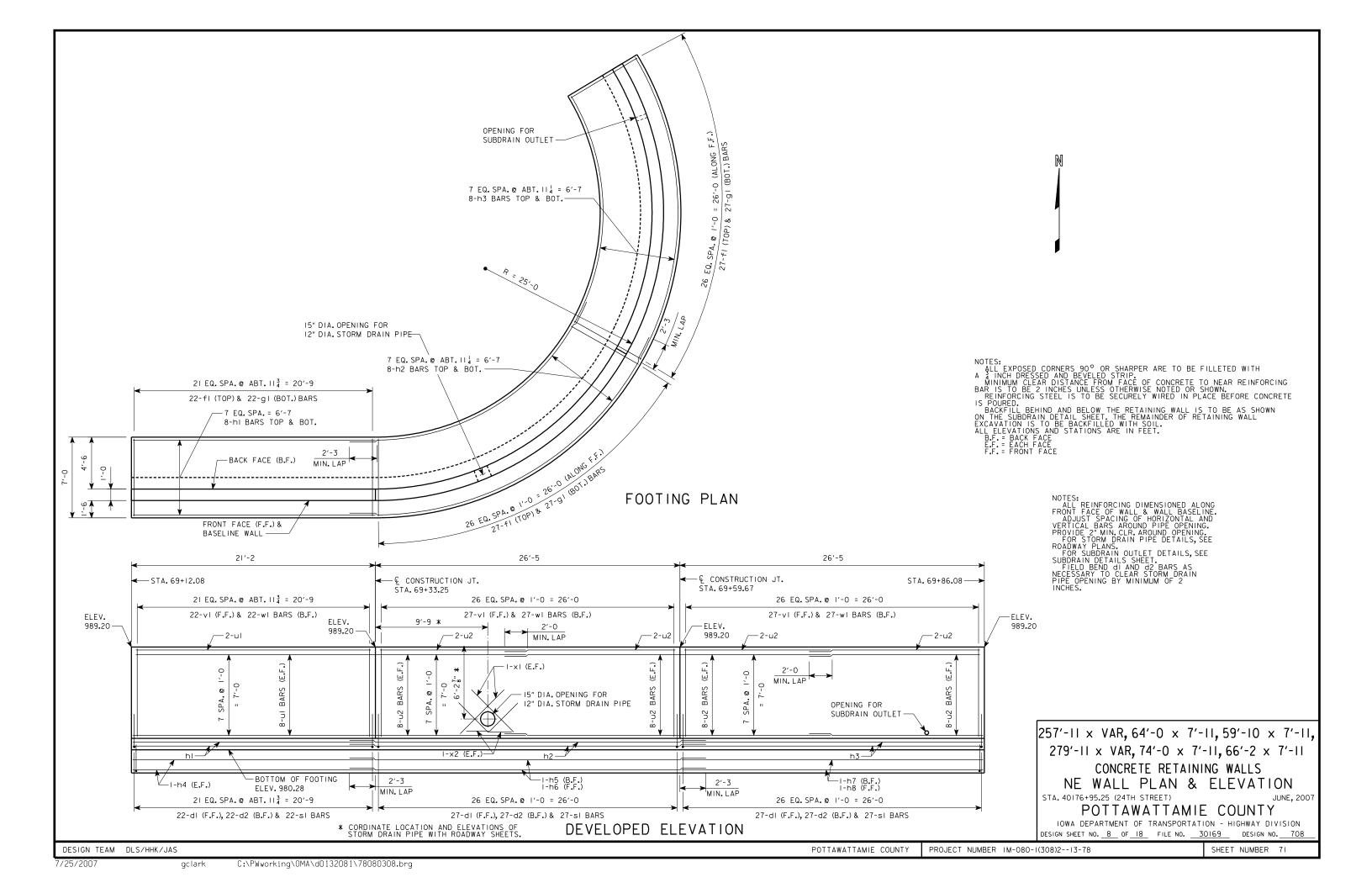
DESIGN TEAM DLS/HHK/JAS

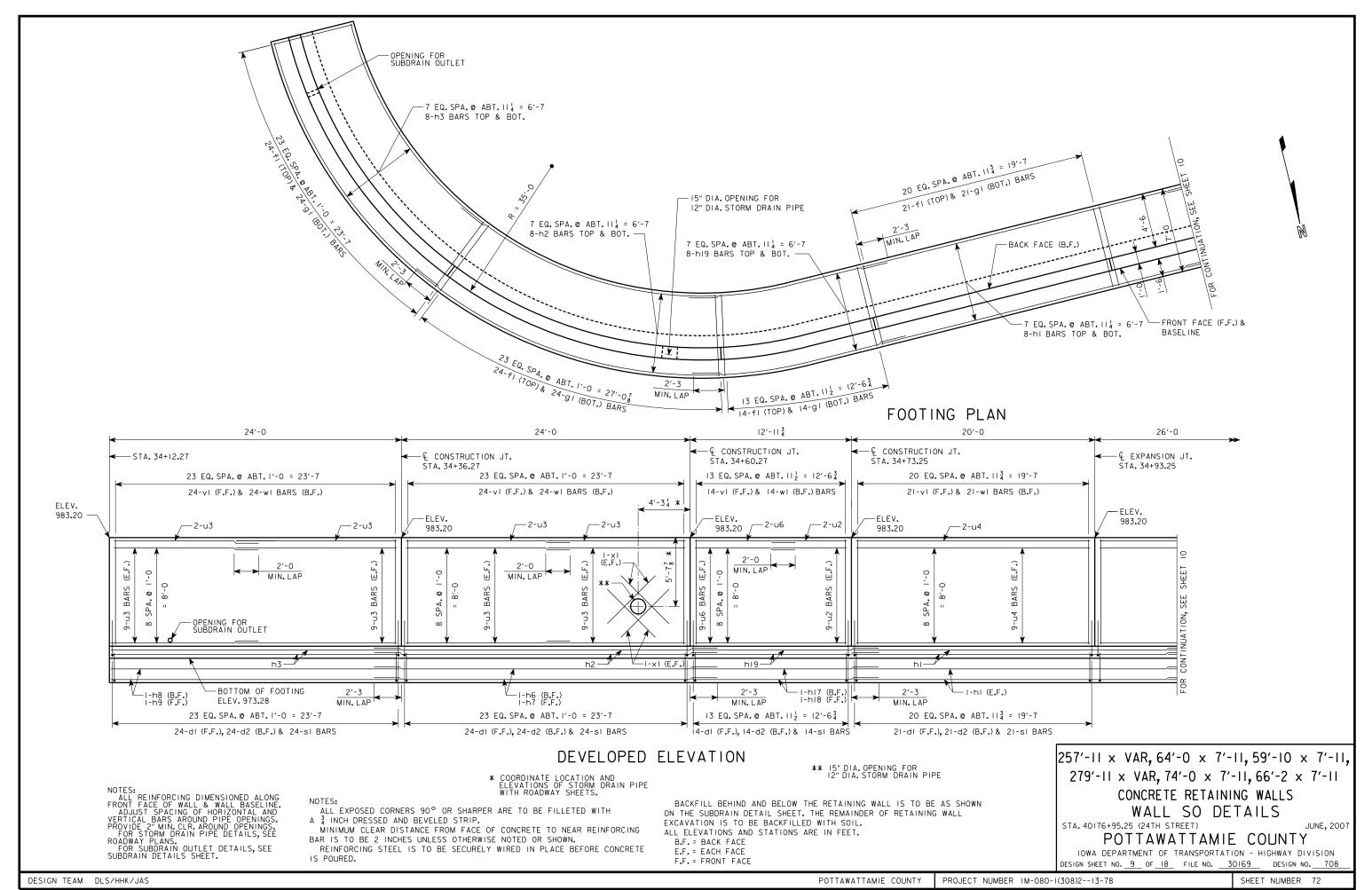


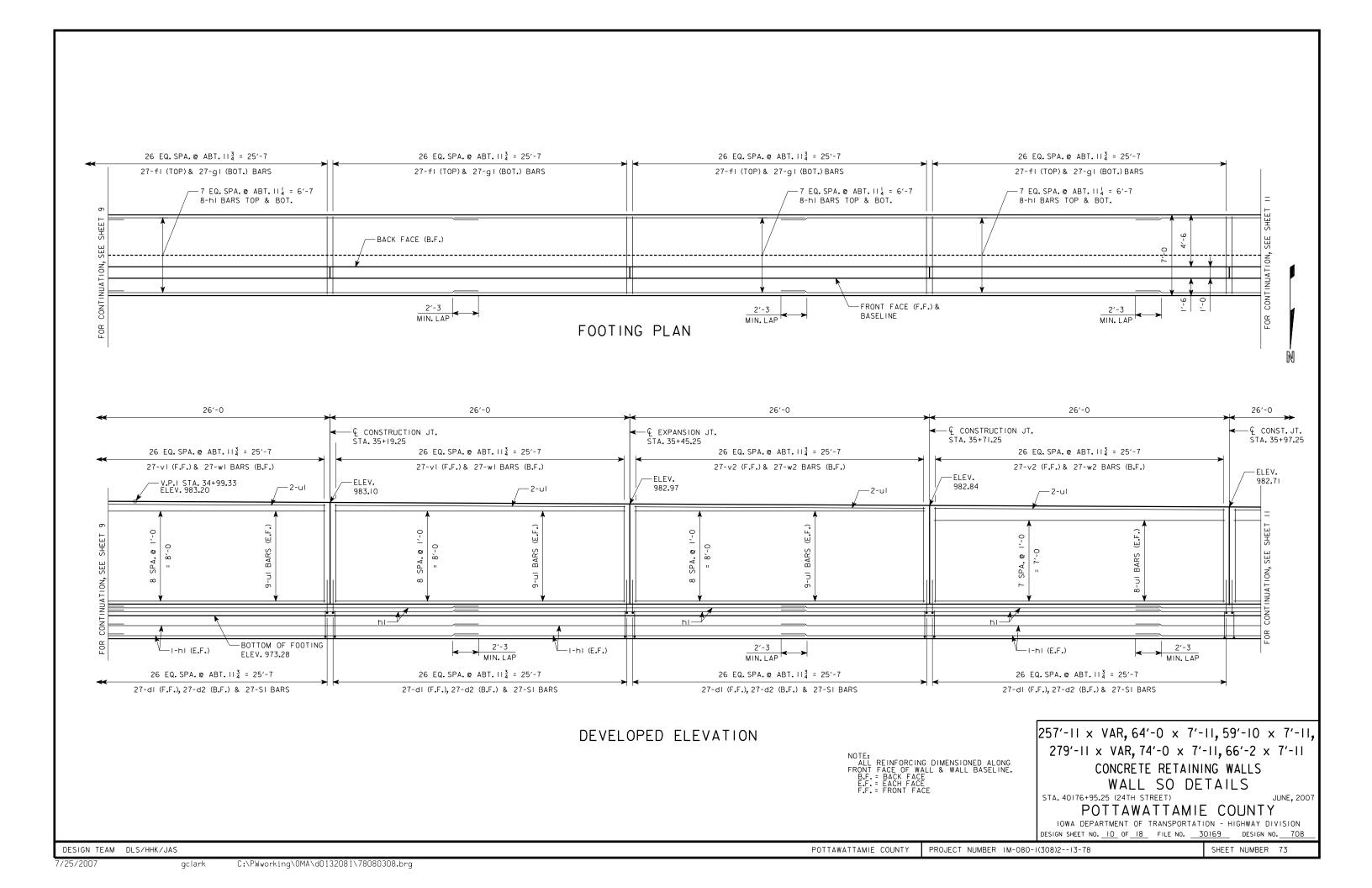


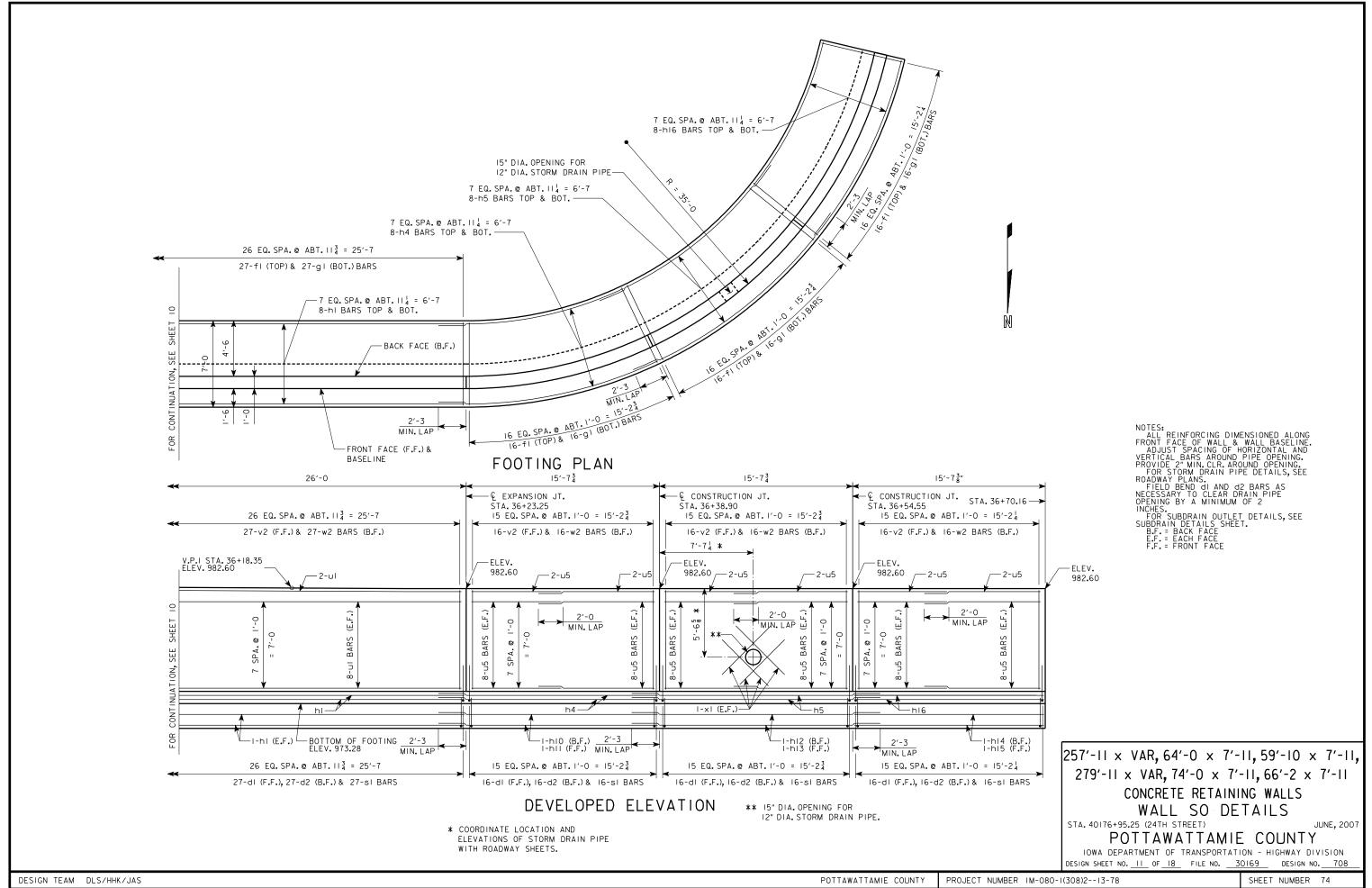


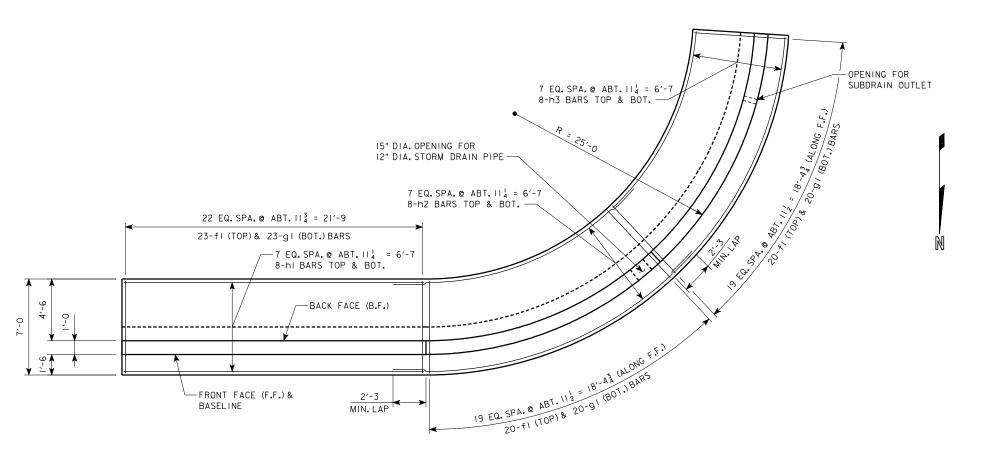




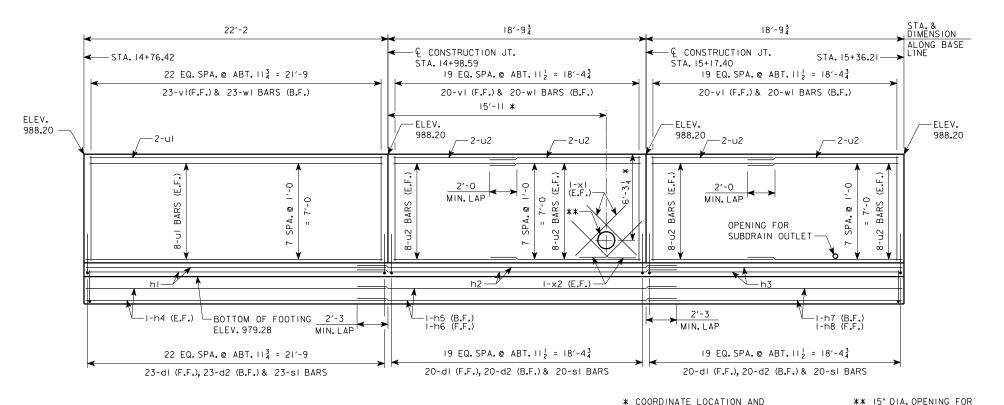








FOOTING PLAN



DEVELOPED ELEVATION

FLEVATION OF STORM DRAIN PIPE

WITH ROADWAY SHEETS.

** 15" DIA. OPENING FOR 12" DIA. STORM DRAIN PIPE.

NOTES:

ALL EXPOSED CORNERS 90° OR SHARPER ARE TO BE FILLETED WITH

A INCH DRESSED AND BEVELED STRIP.

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING

BAR IS TO BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.

REINFORCING STEEL IS TO BE SECURELY WIRED IN PLACE BEFORE CONCRETE

IS POURED.

BACKFILL BEHIND AND BELOW THE RETAINING WALL IS TO BE AS SHOWN

ON THE SUBDRAIN DETAIL SHEET. THE REMAINDER OF RETAINING WALL

EXCAVATION IS TO BE BACKFILLED WITH SOIL.

ALL ELEVATIONS AND STATIONS ARE IN FEET.

B.F. = BACK FACE

E.F. = EACH FACE

F.F. = FRONT FACE

NOTES:
ALL REINFORCING DIMENSIONED ALONG
FRONT FACE OF WALL & WALL BASELINE.
ADJUST SPACING OF HORIZONTAL AND
VERTICAL BARS AROUND PIPE OPENINGS.
PROVIDE 2" MIN. CLR. AROUND OPENINGS.
FOR STORM DRAIN PIPE DETAILS, SEE
ROADWAY PLANS.
FOR SUBDRAIN OUTLET DETAILS, SEE
SUBDRAIN DETAILS SHEET.
FIELD BEND dI AND d2 BARS AS
NECESSARY TO CLEAR STORM DRAIN
PIPE OPENING BY A MINIMUM OF 2
INCHES.

 $|257'-11 \times VAR, 64'-0 \times 7'-11, 59'-10 \times 7'-11,$ $279'-11 \times VAR, 74'-0 \times 7'-11, 66'-2 \times 7'-11$ CONCRETE RETAINING WALLS WALL SW DETAILS

STA. 40176+95.25 (24TH STREET)

JUNE, 2007 POTTAWATTAMIE COUNTY

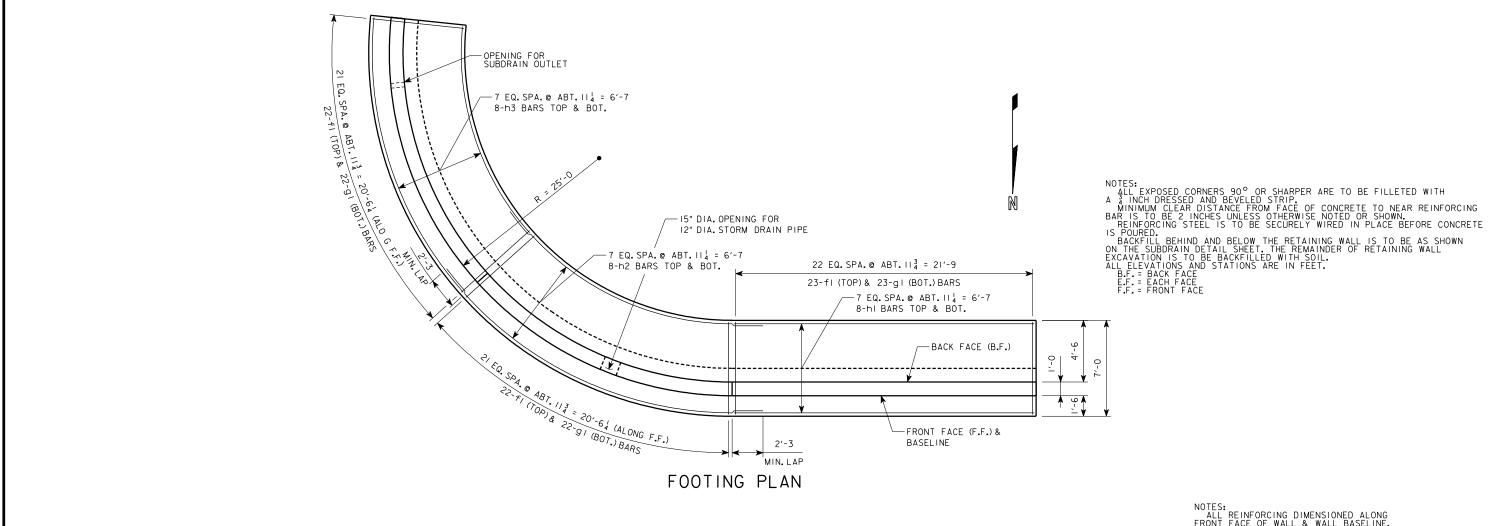
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 12 OF 18 FILE NO. 30169 DESIGN NO. 708

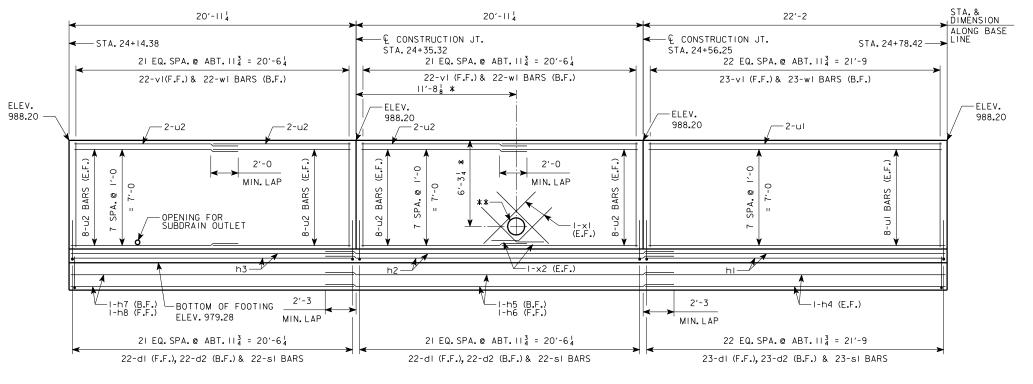
POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-I(308)2--13-78

SHEET NUMBER 75

DESIGN TEAM DLS/HHK/JAS





DEVELOPED ELEVATION

NOTES:
ALL REINFORCING DIMENSIONED ALONG
FRONT FACE OF WALL & WALL BASELINE.
ADJUST SPACING OF HORIZONTAL AND
VERTICAL BARS AROUND PIPE OPENINGS.
PROVIDE 2" MIN. CLR. AROUND OPENINGS.
FOR STORM DRAIN PIPE DETAILS, SEE
ROADWAY PLANS.
FOR SUBDRAIN OUTLET DETAILS, SEE
SUBDRAIN DETAILS SHEET.
FIELD BEND dI AND d2 BARS AS
NECESSARY TO CLEAR STORM DRAIN
PIPE OPENING BY A MINIMUM OF 2
INCHES.

257'-II × VAR, 64'-0 × 7'-II, 59'-IO × 7'-II, 279'-II × VAR, 74'-0 × 7'-II, 66'-2 × 7'-II CONCRETE RETAINING WALLS WALL SE DETAILS

STA. 40176+95.25 (24TH STREET)

TY 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. 13 OF 18 FILE NO. 30169 DESIGN NO. 708

POTTAWATTAMIE COUNTY

* COORDINATE LOCATION AN D ELEVATIONS OF STORM DRAIN PIPE WITH ROADWAY SHEETS.

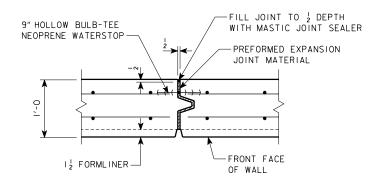
PROJECT NUMBER IM-080-I(308)2--13-78

SHEET NUMBER 76

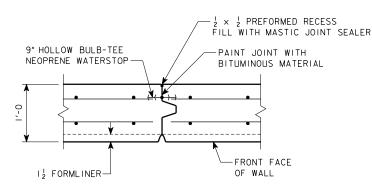
** 15" DIA. OPENING FOR 12" DIA. STORM DRAIN PIPE.

I MAXIMUM TEXTURE DEPTH "v" BARS-FORMLINER "u" BARS--WALL FRONT FACE

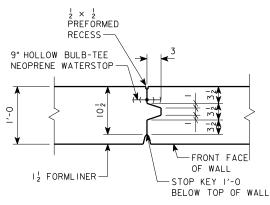
STONE FORMLINER DETAIL

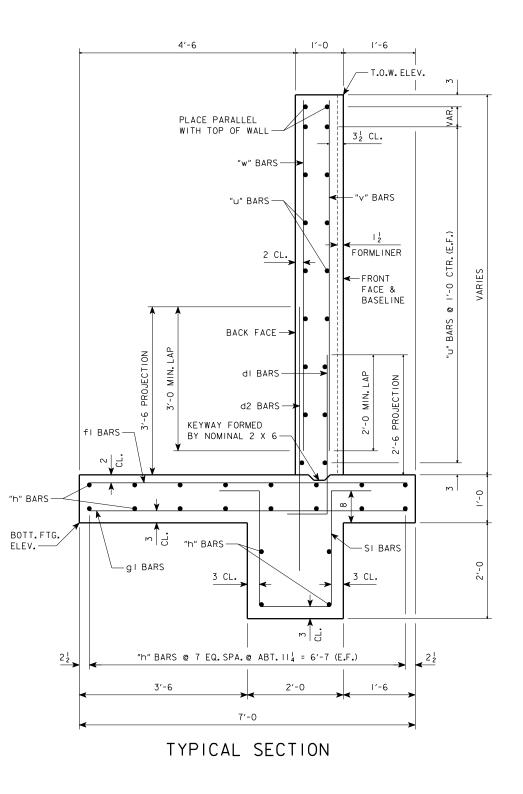


EXPANSION JOINT



CONSTRUCTION JOINT





COLORED SEALER COATINGS NOTES:

ALL EXPOSED VERTICAL CONCRETE SURFACES FROM TOP OF WALL TO A DISTANCE 6 INCHES BELOW THE FINISHED GRADE WILL RECEIVE APPLICATION OF COLORED SEALER COATING. CONCRETE COATING WORK SHALL BE IN ACCORDANCE WITH THE DEVELOPMENTAL SPECIFICATION, "COLORED SEALER COATING FOR STRUCTURAL CONCRETE".

FURTHERMORE, THE COLORED SEALER USED ON THE CONCRETE SURFACES OF DESIGN NO. 508, SHALL BE USED FOR THIS DESIGN. IT IS FURTHER ASSUMED THAT A CONCRÉTE SEALER AND PROCEDURE FOR APPLICATION HAS BEEN ACCEPTED FOR APPLICATION TO THE PIER SURFACES FOR DESIGN NO. 508 BASED ON THE REQUIREMENTS AS STATED IN THE PLANS FOR DESIGN NO.508 ASSOCIATED WITH THIS CONTRACT.

THE SIMULATED STONE TEXTURES ON THE WALLS WILL RECEIVE A THREE-COAT SYSTEM OF PENETRATING STAIN TO SIMULATE THE NATURAL TONES AND VARIATIONS OF THE STONE. APPEARANCE OF THE SIMULATED STONE TEXTURE IS INTENDED TO CLOSELY MATCH THE STONE VENEER USED ON THE ABUTMENT WINGS FOR DESIGN NO.508. COLOR SHALL BE A FULL RANGE OF NATURAL STONE COLORS WITH A FIRST COAT COLOR OF LIGHT OR MEDIUM BUFF AND INCLUDING SUBTLE COLOR VARIATIONS, MINERAL OXIDATION AND STAINING. THE FINAL COLORATION OF THE CONCRETE SURFACE SHALL ACCURATELY SIMULATE THE APPEARANCE OF REAL STONE INCLUDING THE MULTIPLE COLORS, SHADES, FLECKING, AND VEINING THAT ARE APPARENT IN REAL LIMESTONE. USE AT LEAST THREE COLOR SHADES TO SIMULATE THE APPEARANCE OF STONE. BEGIN WITH A BASE COLOR APPLICATION OF LIGHT BUFF. APPLY A SLIGHTLY LIGHTER OR DARKER BASE COLOR TO RANDOM STONES PRIOR TO ADDING THE COLOR VARIATIONS. WHEN ALL STONE COLORS HAVE BEEN APPLIED, APPLY A LIGHT GREY COLOR TO THE SIMULATED MORTARED JOINTS. THE GREY COLOR SHALL MATCH OR BE SLIGHTLY LIGHTER THAN THE PLAIN CONCRETE SURFACES OF THE STRUCTURE. APPLY JOINT COLOR NEATLY AND ONLY TO THE BOTTOM SURFACE OF SIMULATED JOINTS. SUBMIT PRODUCT SPECIFICATION SHEETS AND COATED CONCRETE SAMPLES AS DESCRIBED IN THE DEVELOPMENTAL SPECIFICATION.

MASK ADJACENT CONCRETE SURFACES THAT WILL NOT RECEIVE COATING. NO OVERSPRAY OR CONTAMINATION OF ADJACENT SURFACES IS ALLOWED.

COLORED SEALER COATING SURFACE AREA TABULATION:

WALL NO 159.0 SY WALL SO 145.6 SY

WALL NW 34.2 SY WALL SW 29.8 SY

WALL SE 38.3 SY 31.8 SY TOTAL (ALL WALLS) 438.7 SY ALL COSTS ASSOCIATED WITH CONCRETE SEALER COATING ARE TO BE INCLUDED IN THE BID ITEM "COLORED SEALER COATING FOR STRUCTURAL CONCRETE".

ANTI-GRAFFITI COATING NOTES:
AFTER COLORED SEALER COATINGS APPLICATION AT THE WALLS IS COMPLETE, APPLY ANTI-GRAFFITI COATING TO THE SURFACES RECEIVING COLOR SEALER. ANTI-GRAFFITI COATING SHALL BE PERMANENT AND INVISIBLE. COATING SHALL DRY TO A MATTE OR SATIN, NOT GLOSSY, FINISH. ANTI-GRAFFITI COATING PRODUCTS SHALL BE VERIFIED TO BE COMPATIBLE WITH THE STONE VENEER USED ON THE ABUTMENT WINGS. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR SURFACE PREPARATION, APPLICATION RATE AND METHODS. DO NOT DAMAGE THE STONE VENEER.

DO NOT ALLOW DRIPS OR OVERSPRAY TO CONTAMINATE UNTREATED SURFACES ADJACENT TO TREATED SURFACES.

ALL COSTS ASSOCIATED WITH THE ANTI-GRAFFITI COATING ARE TO BE INCLUDED IN THE BID ITEM "COLORED SEALER COATING FOR STRUCTURAL CONCRETE".

ANTI-GRAFFITI COATING SURFACE AREA TABULATION:

159.0 SY WALL SO 145.6 SY 34.2 SY WALL NW WALL SW 29.8 SY

TOTAL (ALL WALLS) 438.7 SY 38.3 SY WALL SE 31.8 SY

> $|257'-11 \times VAR, 64'-0 \times 7'-11, 59'-10 \times 7'-11,$ $279'-11 \times VAR, 74'-0 \times 7'-11, 66'-2 \times 7'-11$ CONCRETE RETAINING WALLS RETAINING WALL DETAILS STA. 40176+95.25 (24TH STREET) JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 14 OF 18 FILE NO. 30169 DESIGN NO. 708

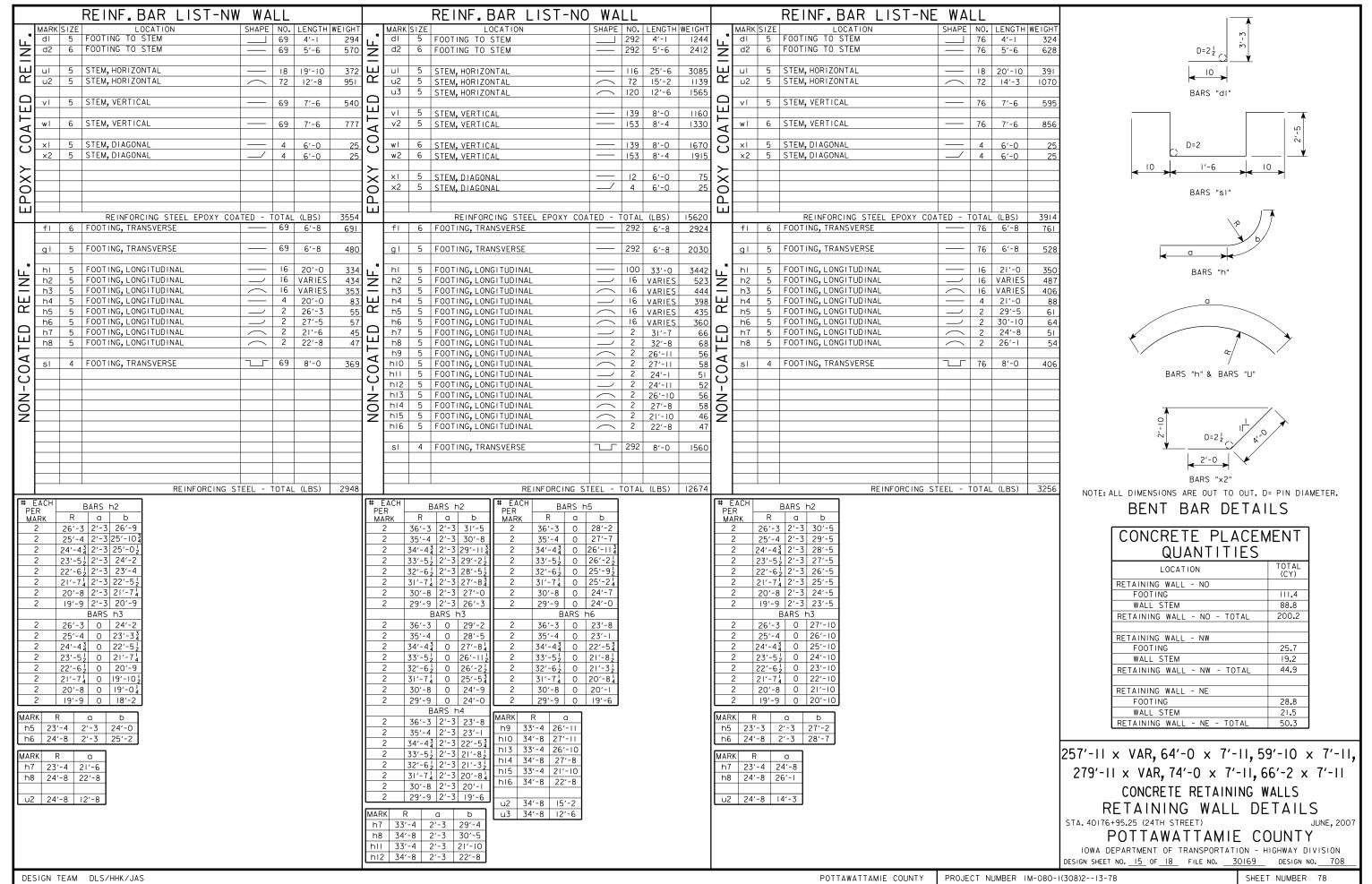
TYPICAL KEY DETAIL

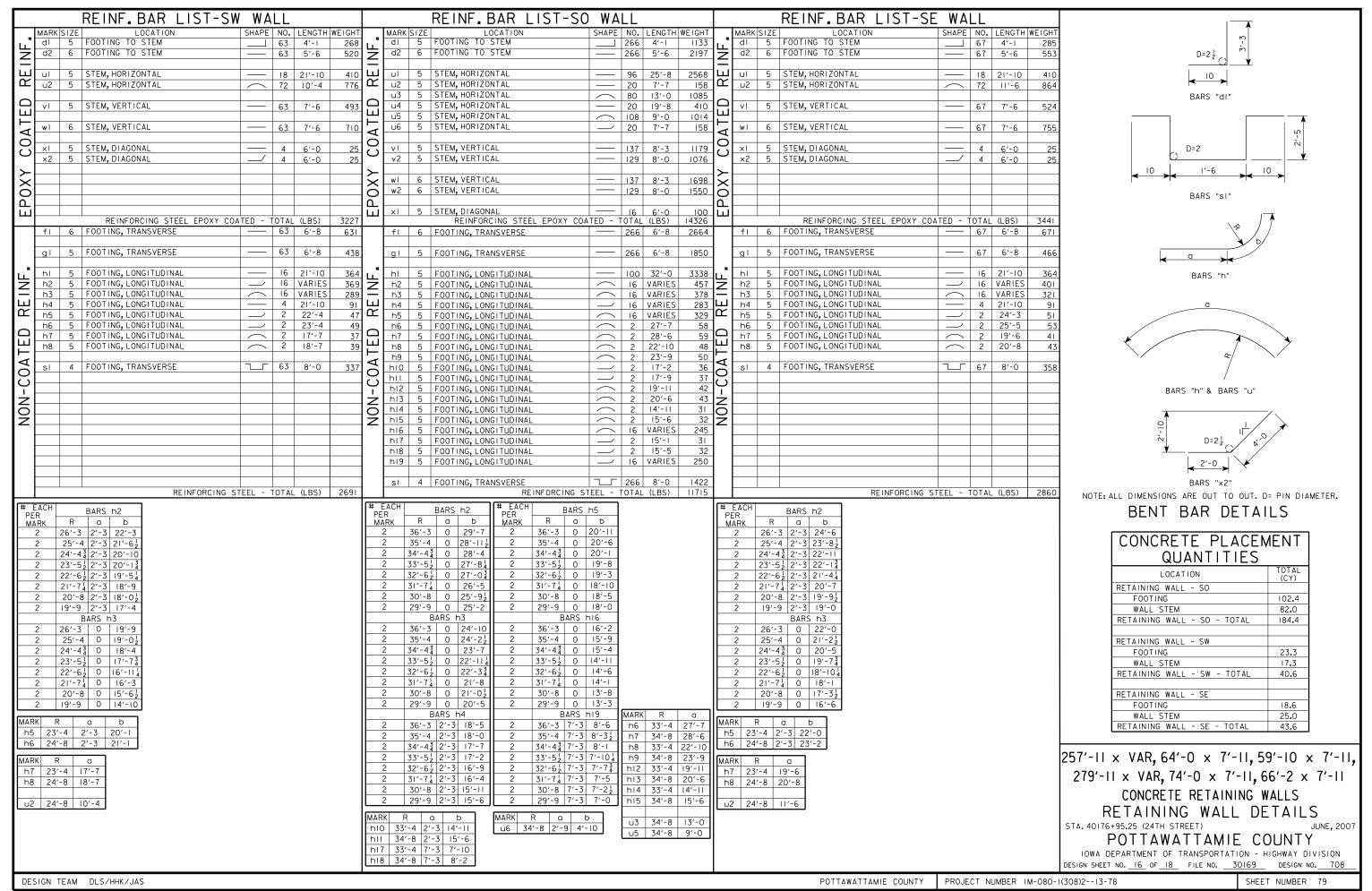
POTTAWATTAMIE COUNTY

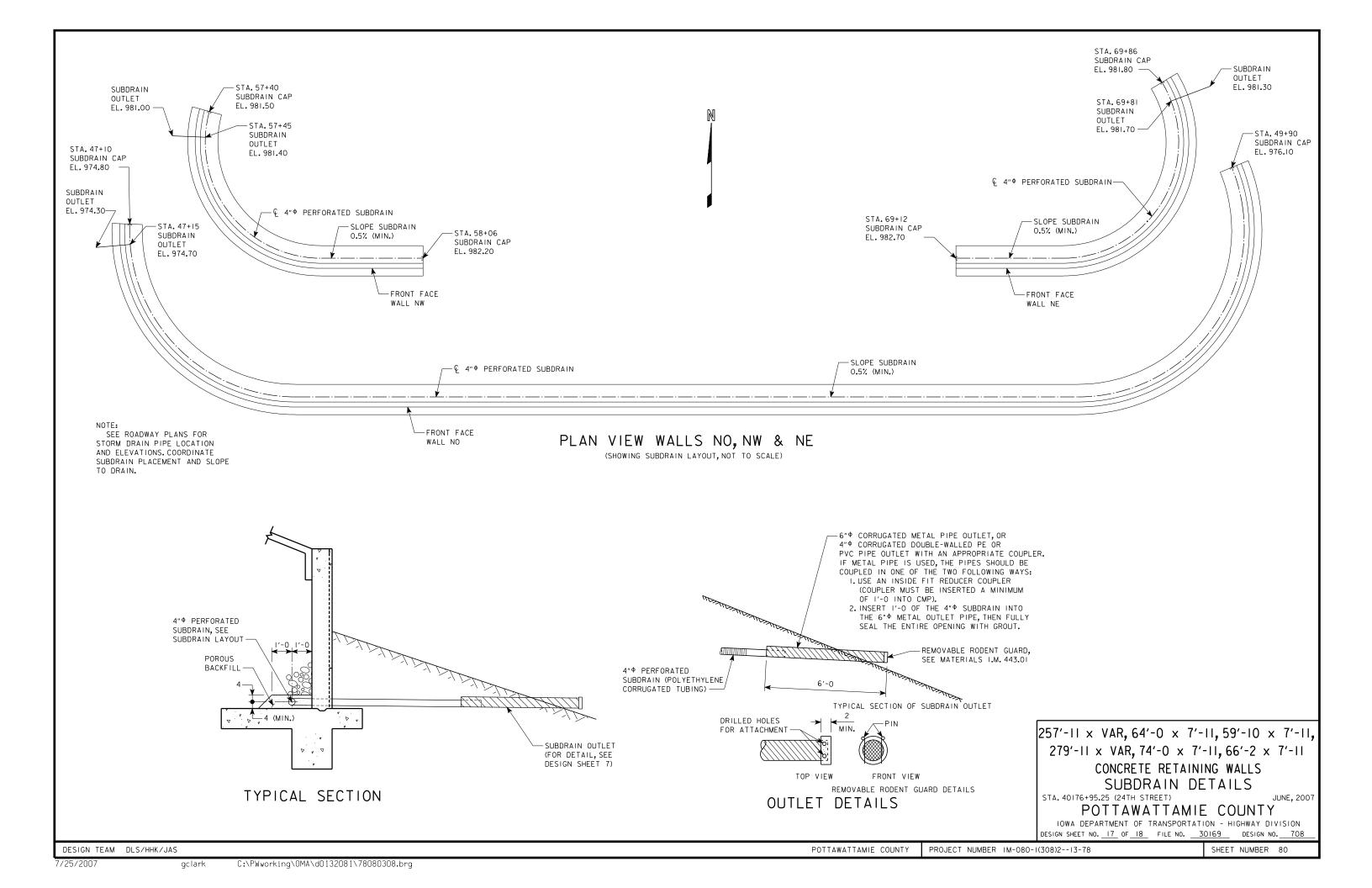
PROJECT NUMBER IM-080-1(308)2--13-78

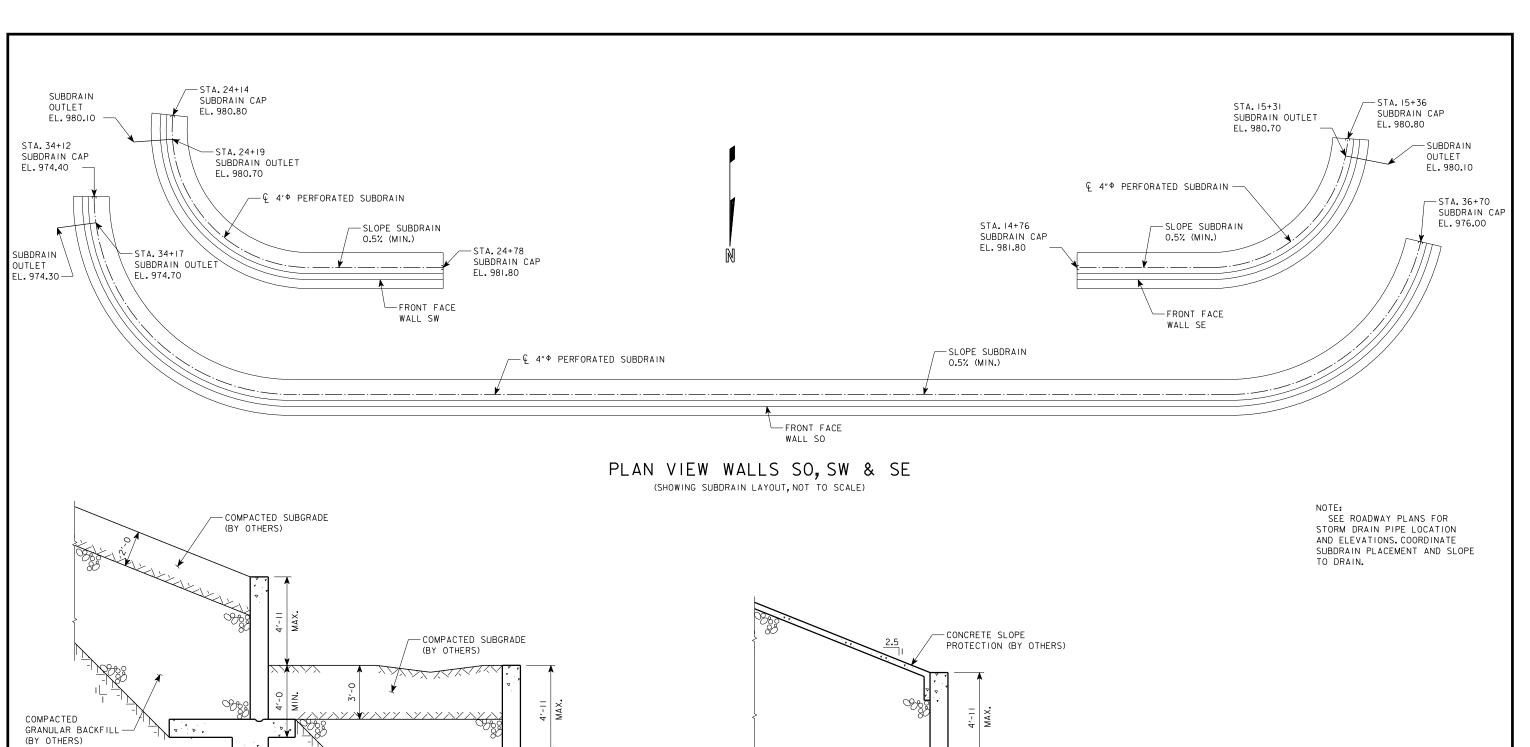
SHEET NUMBER 77

DESIGN TEAM DLS/HHK/JAS









257'-II × VAR, 64'-0 × 7'-II, 59'-IO × 7'-II, 279'-II × VAR, 74'-0 × 7'-II, 66'-2 × 7'-II CONCRETE RETAINING WALLS SUBDRAIN DETAILS

STA. 40176+95.25 (24TH STREET)

JUNE, 2007

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

DESIGN SHEET NO. 18 OF 18 FILE NO. 30169 DESIGN NO. 708

TYPICAL SECTION
(OUTSIDE BRIDGE LIMITS)

B 6

COMPACTED GRANULAR BACKFILL (BY OTHERS)

TYPICAL SECTION
(UNDER BRIDGE)

DESIGN TEAM DLS/HHK/JAS

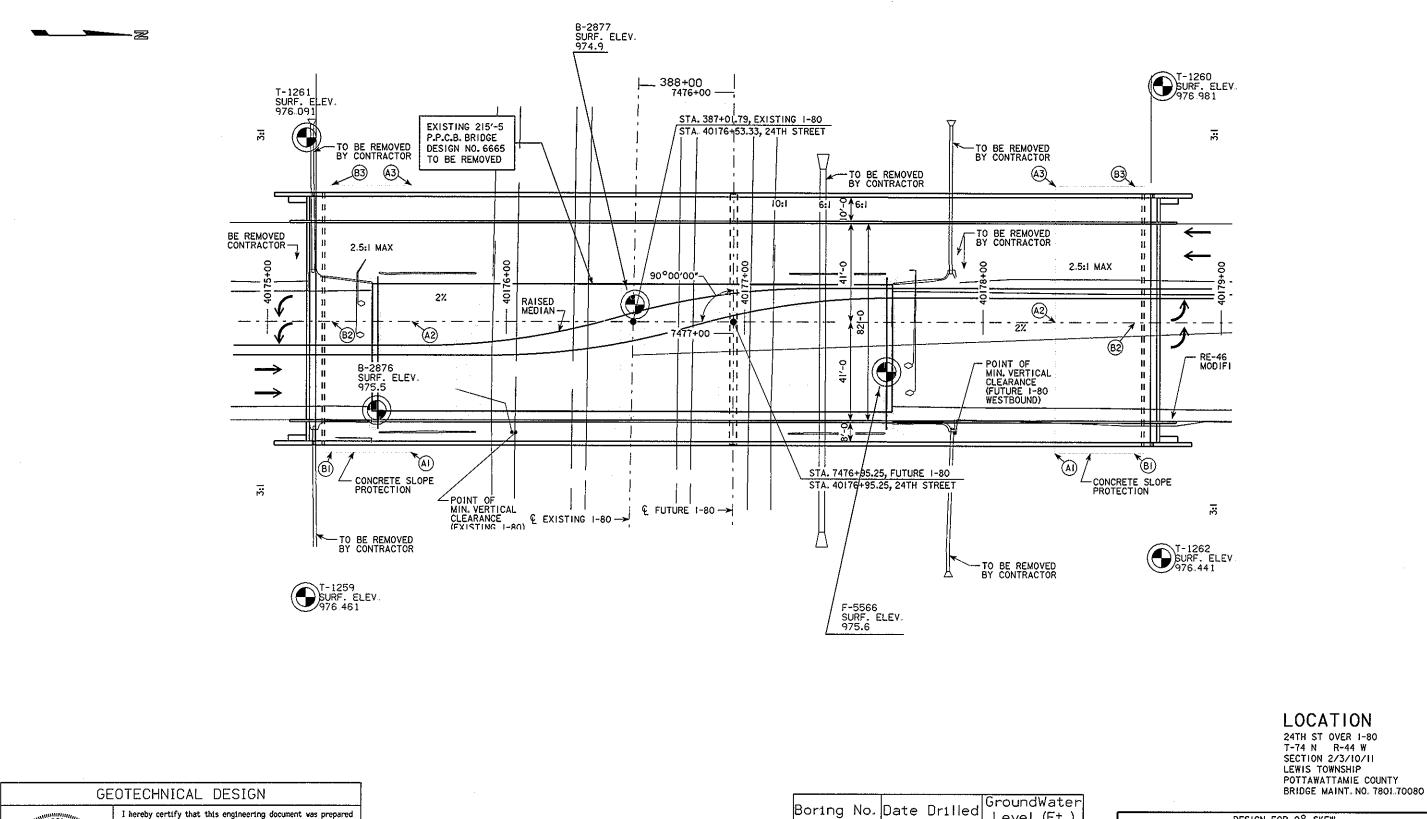
POTTAWATTAMIE COUNTY

PROJECT NUMBER IM-080-1(308)2--13-78

SHEET NUMBER 81

COMPACTED GRANULAR BACKFILL—

(BY OTHERS)



PROFESSION4 Robert L. Stanley 08468 AWO!

by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iova.

Robert L.

My license renewal date is December 31, 20 08

Pages or sheets covered by this seal: _SPS.01, SPS.02, & SPS.03

Boring No.	Date Drilled	Level (Ft.)
T-1259	01/04/06	14.8 PLUGGED
T-1260	01/04/06	17.2 PLUGGED
T-1261	01/05/06	16.4 PLUGGED
T-1262	01/10/06	16.4 PLUGGED
B-2876	04/1967	NA
B-2877	04/1967	NA
F-5566	04/1967	NA

SECTION 2/3/10/11 LEWIS TOWNSHIP POTTAWATTAMIE COUNTY

353'-6X82' CONT. WELDED GIRDER BRIDGE

W/8' SIDEWALK & 10' SHARED USE PATH

24TH STREET OVER 1-80
178'-6 & 175'-0 SPANS
SITUATION PLAN
SEPTEMBER, 2006

STA. 40176+93..50

POTTAWATTAMIE COUNTY

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 1 OF 2 FILE NO. 30169 DESIGN NO. 508

DESIGN TEAM STANLEY/MEGIVERN/KRETLOW

POTTAWATTAMIE

IOWA DOT * OFFICE OF DESIGN

COUNTY

PROJECT NUMBER BRFIM-080-1(308)2--05-78

SHEET NUMBER SPS.01

