

Otego

699.040001

MOBILIZATION

FILE NAME = 11,2005/2005,166,115 RT 23
DATE/TIME = 10/23/2008 8:01:20 AM
USER = ssingh

DESIGN SUPERVISOR JRM

JOE

ESTIMATE OF QUANTITIES ITEM NO. DESCRIPTION UNIT ESTIMATED FINAL DISMANTLING AND REMOVING EXISTING SUPERSTRUCTURES 202.12000 REMOVING EXISTING SUPERSTRUCTURES NEC. REMOVAL OF SUBSTRUCTURES СМ 202.19 390 REMOVING OLD BITUMINOUS CONCRETE OVERLAY SQM 202.20 280 UNCLASSIFIED EXCAVATION AND DISPOSAL СМ 203.02 146 203.03 EMBANKMENT IN PLACE СМ 470 203.21 SELECT STRUCTURE FIL СМ 150 206.01 STRUCTURE EXCAVATION СМ 1000 206.02 207.20 TRENCH AND CULVERT EXCAVATION СМ 115 SQM GEOTEXTILE BEDDING 790 PREFABRICATED COMPOSITE INTEGRAL ABUTMENT DRAIN SQM 207.2 136 85 TURBIDITY CURTAIN 24209.1501 SUBBASE COURSE OPTIONAL TYPE СМ 304.15 551.11 CAST-IN-PLACE CONCRETE PILES М 971 551.13 555.09 FURNISHING EQUIPMENT FOR DRIVING PILES NEC. CONCRETE FOR STRUCTURES, CLASS HP 556.0201 UNCOATED BAR REINFORCEMENT FOR CONCRETE STRUCTURES 2396 EPOXY-COATED BAR REINFORCEMENT FOR CONCRETE STRUCTURES Kg 3444 STUD SHEAR CONNECTORS FOR BRIDGES EΑ 1278 16557,0501 PRECAST CONCRETE DECK SYSTEM SQM 505 FIELD CAST JOINTS BETWEEN PRECAST CONCRETE UNITS 169 16557,2101 STRUCTURAL APPROACH SLAB WITH INTEGRAL WEARING SURFACE - TYPE 2 FRICTION SQM SQM 170 LONGITUDINAL SAWCUT GROOVING OF STRUCTURAL SLAB SURFACE 657 PROTECTIVE SEALING OF STRUCTURAL CONCRETE ON NEW BRIDGE DECKS AND BRIDGE DECK 695 SQM 18559.1896 564.0501 NEC. STRUCTURAL STEEL (TYPE 1) 565.30 RUBBER IMPREGNATED WOVEN COTTON-POLYESTER FABRIC 10 568.51 STEEL BRIDGE RAILING (FOUR-RAIL) 84 568,70 TRANSITION BRIDGE RAILING 570.090001 ENVIRONMENTAL GROUND PROTECTION 570.100001 ENVIRONMENTAL WATERWAY PROTECTION NEC. 571.010001 СМ TREATMENT AND DISPOSAL OF PAINT REMOVAL WASTE 0.1 СМ 620.05 STONE FILLING (HEAVY) 660 FΑ FIELD LABORATORY CONCRETE CYLINDER CURING BOX ΕA 637.03 697.03 FIELD CHANGE PAYMENT DC 698.04 ASPHALT PRICE ADJUSTMENT 698.05 DC FUEL PRICE ADJUSTMENT STEEL/IRON PRICE ADJUSTMENT

BAR SIZES - I	NCH POUND AND	SOFT METRIC.
BAR DESIGNATION NO.	NOMINAL DIAMETER, IN. (mm)	NOMINAL AREA, IN.º (mm²)
3(10)	0.375(9.5)	0.11(71)
4(13)	0.500(12.7)	0.20(129)
5(16)	0.625(15.9)	0.31(199)
6(19)	0.750(19.1)	0.44(284)
7(22)	0.875(22.2)	0.60(387)
8(25)	1.000(25.4)	0.79(510)
9(29)	1.128(28.7)	1.00(645)
10(32)	1.270(32.3)	1.27(819)
11(36)	1.410(35.8)	1.56(1006)
14(43)	1.693(43.0)	2.25(1452)
18(57)	2.257(57.3)	4.00(2581)

• SOFT METRIC BAR DESIGNATION NUMBERS, NOMINAL DIAMETERS AND AREAS ARE THE VALUES ENCLOSED WITHIN THE BRACKETS. BAR DESIGNATION NUMBERS APPROXIMATE THE NUMBER OF MILLIMETERS OF THE NOMINAL DIAMETER OF THE BAR.

	ALTERED BY: ON:
TE OF NEW LOAD AND AND AND AND AND AND AND AND AND A	

## HIGH VOLTAGE ELECTRICAL LINES

LS

NEC

HIGH VOLTAGE ELECTRICAL LINES ARE IN PROXIMITY TO THIS BRIDGE. REFER TO SUBSECTION 107-05 OF THE STANDARD SPECIFICATIONS FOR CONTRACTOR SAFETY REQUIREMENTS.

# GENERAL NOTES

DESIGN SPECIFICATIONS: NYSDOT LRFD BRIDGE DESIGN SPECIFICATIONS WITH ALL PROVISIONS IN EFFECT AS OF OCTOBER 2008 FOR DESIGN PURPOSES, COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS:

SUBSTRUCTURES, f'c = 21 MPA
PRECAST DECK PANELS, f'c = 35 MPA
UHPC GROUT, f'c = 120 MPA

LIVE LOAD: AASHTO HL-93 AND NYSDOT DESIGN PERMIT VEHICLE.

CONSTRUCTION AND MATERIAL SPECIFICATIONS: STANDARD SPECIFICATIONS, CONSTRUCTION AND MATERIALS, NEW YORK STATE DEPARTMENT OF TRANSPORTATION, OFFICE OF ENGINEERING, DATED MAY 4, 2006 WITH CURRENT ADDITIONS AND MODIFICATIONS.

ALL SHOP DRAWINGS SUBMITTED FOR THIS PROJECT SHALL BE IN SI UNITS. ERECTION DRAWINGS ARE TO BE PREPARED IN DUAL UNITS.

THE COST OF WATER USED FOR COMPACTION OF SELECT FILL ITEMS SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 203.21 - SELECT STRUCTURE FILL.

THE COST OF ALL JOINT MATERIAL SHALL BE INCLUDED IN THE UNIT PRICES BID FOR THE VARIOUS ITEMS OF THE CONTRACT, UNLESS OTHERWISE SPECIFIED ON THE PLANS.

THE LOAD RATINGS ARE IN ACCORDANCE WITH THE AASHTO "MANUAL FOR CONDITION EVALUATION OF BRIDGES - 1994" AND THE MANUAL FOR CONDITION EVALUATION AND LOAD AND RESISTANCE FACTOR RATING (LRFR) OF HIGHWAY BRIDGES - OCTOBER 2003 WITH ALL INTERIM PROVISIONS IN EFFECT.

DIMENSIONS FOR THICKNESSES OF STEEL ROLLED ANGLE SHAPES AND STRUCTURAL TUBING ARE SHOWN ACCORDING TO THE AISC MANUAL "METRIC PROPERTIES OF STRUCTURE SHAPES WITH DIMENSIONS ACCORDING TO ASTM A6M."

THIS BRIDGE SHALL BE MAINTAINED IN ACCORDANCE WITH THE GUIDELINES CONTAINED IN THE CURRENT EDITION OF AASHTO MAINTENANCE MANUAL: THE MAINTENANCE AND MANAGEMENT OF ROADWAYS AND BRIDGES.

## SUPERSTRUCTURE NOTES

ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A709M GRADE

CLEANING CONTROLLED OXIDIZING STRUCTURAL STEEL ASTM A709M GRADE 345W.

A. IN THE FABRICATION SHOP:
GIRDERS SHALL BE BLAST CLEANED IN ACCORDANCE WITH SSPC-SP6
(COMMERCIAL BLAST CLEANING). HEAVY COATINGS OF OIL OR
GREASE SHALL BE REMOVED BEFORE BLASTING IN ACCORDANCE
WITH SSPC-SP1 (SOLVENT CLEANING).

B. IN THE FIELD:
THE OUTSIDE SURFACE OF THE FASCIA STRINGERS SHALL BE
CLEANED SO THAT ALL DIRT, GREASE, PAINT OR OTHER FOREIGN
MATERIAL IS REMOVED AT THE COMPLETION OF THE BRIDGE
CONSTRUCTION. THE PURPOSE OF THE CLEANING IS TO RETURN
THE FASCIA SURFACES TO THE CONDITION IN WHICH THEY LEFT
THE FABRICATION SHOP.

THE COST OF CLEANING THIS STEEL IN THE FABRICATION SHOP AND THE FIELD SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE VARIOUS ITEMS IN THE CONTRACT.

FOR THE VARIOUS LUMP SUM STRUCTURAL STEEL MEMBERS IN THE CONTRACT, THE "TOTAL MASS FOR PROGRESS PAYMENT" IS AS FOLLOWS:

ITEM TOTAL MASS FOR PROGRESS PAYMENT 564.0501 82,000 KILOGRAMS

THIS MASS SHALL BE USED IN DETERMINING PARTIAL PAYMENTS AND PROGRESS. UNDER NO CIRCUMSTANCES SHALL THE "TOTAL MASS FOR PROGRESS PAYMENT" BE USED FOR FINAL PAYMENT PURPOSES. THE CONTRACTOR IS ADVISED NOT TO USE THE "TOTAL MASS FOR PROGRESS PAYMENTS" AS A BIDDING TOOL. DISCREPANCIES WHICH MAY OCCUR BETWEEN THE TOTAL MASS SHIPPED AND "TOTAL MASS FOR PROGRESS PAYMENT" SHALL NOT BE A BASIS FOR ADDITIONAL COMPENSATION.

TOP SURFACES OF THE NEW BRIDGE DECK OVERLAY AND APPROACH SLABS SHALL BE SEALED ACCORDING TO ITEM 18559.1896 PROTECTIVE SEALING OF STRUCTURAL CONCRETE ON NEW BRIDGE DECKS AND BRIDGE DECK OVERLAYS.

## STEEL ERECTION NOTES

THE CONTRACTOR SHALL PROVIDE FOR THE STABILITY OF STRUCTURAL STEEL DURING ALL PHASES OF ERECTION AND CONSTRUCTION, AS PROVIDED IN PARAGRAPH 204.2 OF THE NEW YORK STATE STEEL CONSTRUCTION MANUAL (SCM).

#### SUBSTRUCTURE NOTES

ALL PLACEMENTS OF SELECT STRUCTURE FILL, ITEM 203.21, SHALL BE COMPACTED TO 95 PERCENT OF STANDARD PROCTOR MAXIMUM DENSITY.

BRIDGES

9120.32

OCT. 23, 2008

PS&E DATE

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**CUL VERTS** 

HIGHWAY EMBANKMENT MATERIAL (HIGHWAY ESTIMATE) AND SELECT STRUCTURE FILL, ITEM 203.21, SHALL BE PLACED SIMULTANEOUSLY, IN CONTACT, ON BOTH SIDES OF THE PAYMENT LINE.

### REMOVAL NOTES

EXISTING SUBSTRUCTURE SHALL BE REMOVED WITHIN THE LIMITS SHOWN ON THE PLANS UNDER ITEM 202.19 IN THE BRIDGE ESTIMATE.

EXISTING SUPERSTRUCTURE SHALL BE REMOVED UNDER ITEM 202.120001 IN THE BRIDGE ESTIMATE.

HYDRO-DEMOLITION METHODS WILL NOT BE ALLOWED.

THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE REQUIREMENTS OF SUBSECTION 202-3.01 GENERAL AND SAFETY REQUIREMENTS. A REMOVAL PLAN SHALL BE SUBMITTED TO THE ENGINEER FIFTEEN (15) DAYS PRIOR TO BEGINNING THE DEMOLITION. THE REQUIREMENT THAT IT BE SIGNED BY A REGISTERED PROFESSIONAL ENGINEER IS WAIVED.

#### SUPERSTRUCTURE (OR SUBSTRUCTURE) REMOVAL NOTES

LIMITS AND METHODS FOR REMOVAL OF PAINT AT LOCATIONS OF FASTENER REMOVAL OR FLAME CUTTING SHALL BE AS DESCRIBED IN SUBSECTIONS 202-3.05 AND 574 OF THE STANDARD SPECIFICATIONS. THE COST OF PAINT REMOVAL SHALL BE INCLUDED IN THE LUMP SUM PRICE(S) BID FOR THE SUPERSTRUCTURE REMOVAL ITEM(S) (OR THE UNIT PRICE BID FOR THE SUBSTRUCTURE REMOVAL ITEM), PAINT WASTE NOT COLLECTED BY VACUUM METHODS SHALL BE COLLECTED USING THE ENVIRONMENTAL GROUND AND/OR WATERWAY PROTECTION ITEM(S). WASTE SHALL BE DISPOSED OF USING THE TREATMENT AND DISPOSAL OF PAINT REMOVAL WASTE ITEM.

LOOSE AND/OR PEELING PAINT ON STEEL SURFACES MAY BECOME DISLODGED DURING REMOVAL OPERATIONS OR DURING TRANSPORTATION FROM THE SITE UNLESS APPROPRIATE MEASURES ARE TAKEN. THE CONTRACTOR SHALL FORMULATE AND SUBMIT A METHOD OF REMEDIATING THE CONDITION FOR APPROVAL BY THE ENGINEER. WORKER LEAD PROTECTION IN ACCORDANCE WITH OSHA 1926.62 MUST BE SATISFIED. ALTERNATIVES COULD INCLUDE TRANSPORTING AFFECTED MEMBERS IN CLOSED TRUCKS, WRAPPING AFFECTED MEMBERS PRIOR TO REMOVAL, EXCAPSULATING THE LOOSE PAINT OR REMOVAL OF LOOSE PAINT PRIOR TO DISMANTLING OPERATIONS. THE COST OF REMEDIATING THIS CONDITION SHALL BE INCLUDED IN THE LUMP SUM PRICE(S) BID FOR THE SUPERSTRUCTURE REMOVAL ITEM(S) (OR THE UNIT PRICE BID FOR THE SUBSTRUCUTRE REMOVAL ITEM.) THE USE OF ENVIRONMENTAL GROUND AND/OR WATERWAY PROTECTION ITEMS WILL BE REQUIRED. DEPENDING ON THE ALTERNATIVE CHOSEN, THE TREATMENT AND DISPOSAL OF PAINT REMOVAL WASTE ITEM MAY BE REQUIRED. BECAUSE OF THE ABOVE-MENTIONED CONDITION, THE CONTRACTOR SHOULD EXAMINE THE CONDITION OF THE STRUCTURE'S PAINT PRIOR TO SUBMITTING A BID.

REFER TO SUBSECTION 107-05 OF THE STANDARD SPECIFICATIONS FOR SAFETY AND HEALTH REQUIREMENTS.

#### STRUCTURAL SLAB OVERLAY NOTES

THE NOMINAL THICKNESS OF THE CONCRETE OVERLAY SHALL BE 50 (40 MINIMUM, 75 MAXIMUM).

SHOULD THE INSTALLATION OF THE PRECAST DECK PANELS, TOGETHER WITH APPLICATION OF THE SPECIALIZED CONCRETE OVERLAY, RESULT IN A REVISED FINISHED PROFILE HIGHER THAN THAT SHOWN ON THE PLANS, THE CONTRACTOR SHALL SUBMIT THE REVISED PROFILE TO THE REGIONAL DIRECTOR FOR APPROVAL AT LEAST TWO WEEKS PRIOR TO PLACEMENT OF THE CONCRETE OVERLAY.

NO OVERLAY MATERIAL SHALL BE PLACED UNTIL THE REGIONAL DIRECTOR HAS APPROVED THE CONTRACTOR'S PROPOSED REVISIONS.

ALL ROADWAY SURFACES RECEIVING A SPECIALIZED CONCRETE OVERLAY SHALL BE GROOVED UNDER THE SAWCUT GROOVING OF THE STRUCTURAL SLAB SURFACE ITEM AND BE SEALED UNDER THE PROTECTIVE SEALING OF STRUCTURAL CONCRETE ON NEW BRIODGE DECKS AND BRIDGE DECK OVERLAYS ITEM.

AND NOTES

ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED CONTR.

ESTIMATE OF QUANTITIES D:

CONTRACT NUMBER
D260931

DRAWING NO. ST-4 SHEET NO. 56

NEW YORK STATE DEPARTMENT OF TRANSPORTATION REGION 9

AS BUILT REVISIONS
DESCRIPTION OF WORK:

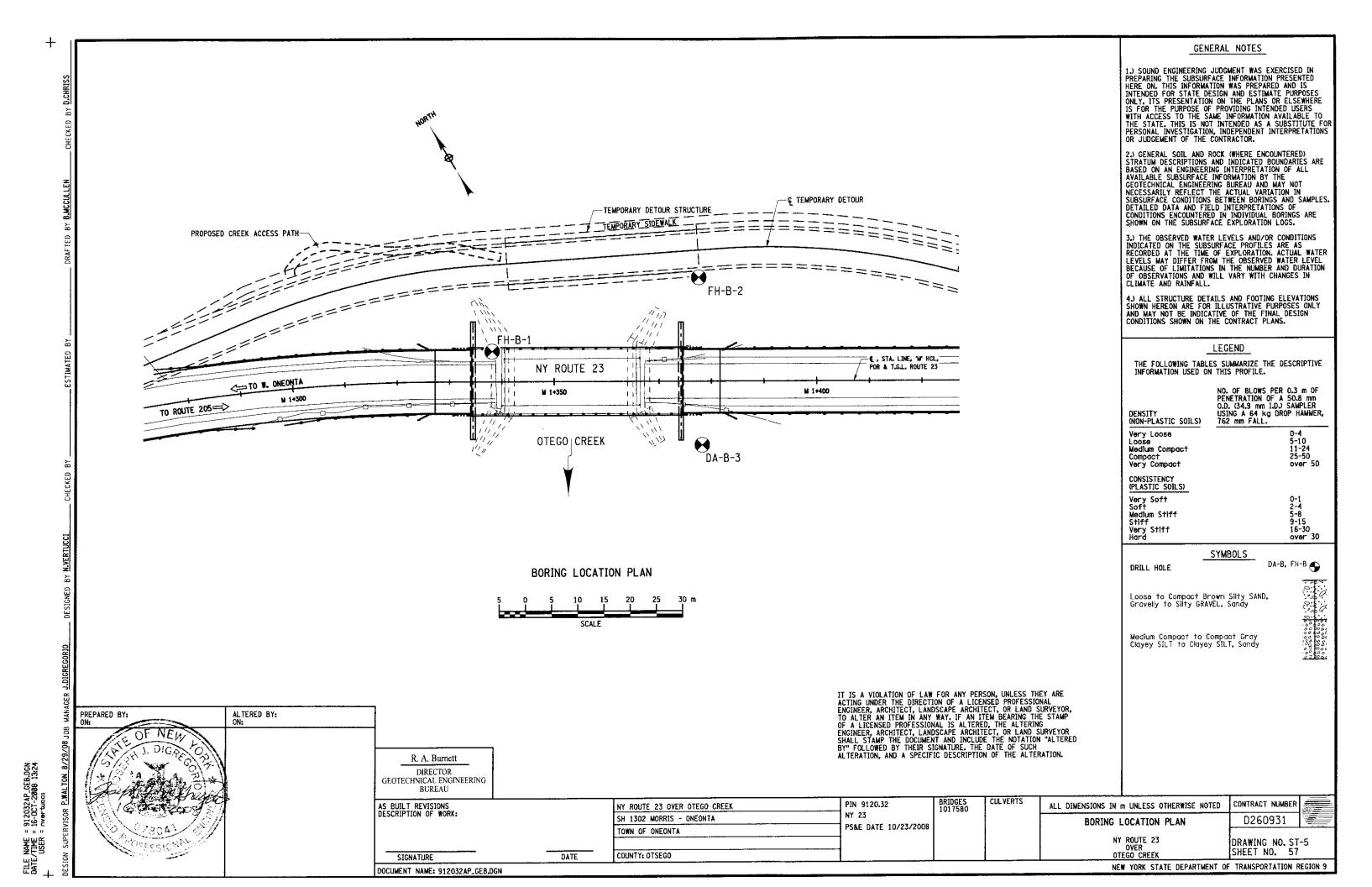
NY ROUTE 23 OVER OTEGO CREEK
SH 1302 MORRIS - ONEONTA
TOWN OF ONEONTA

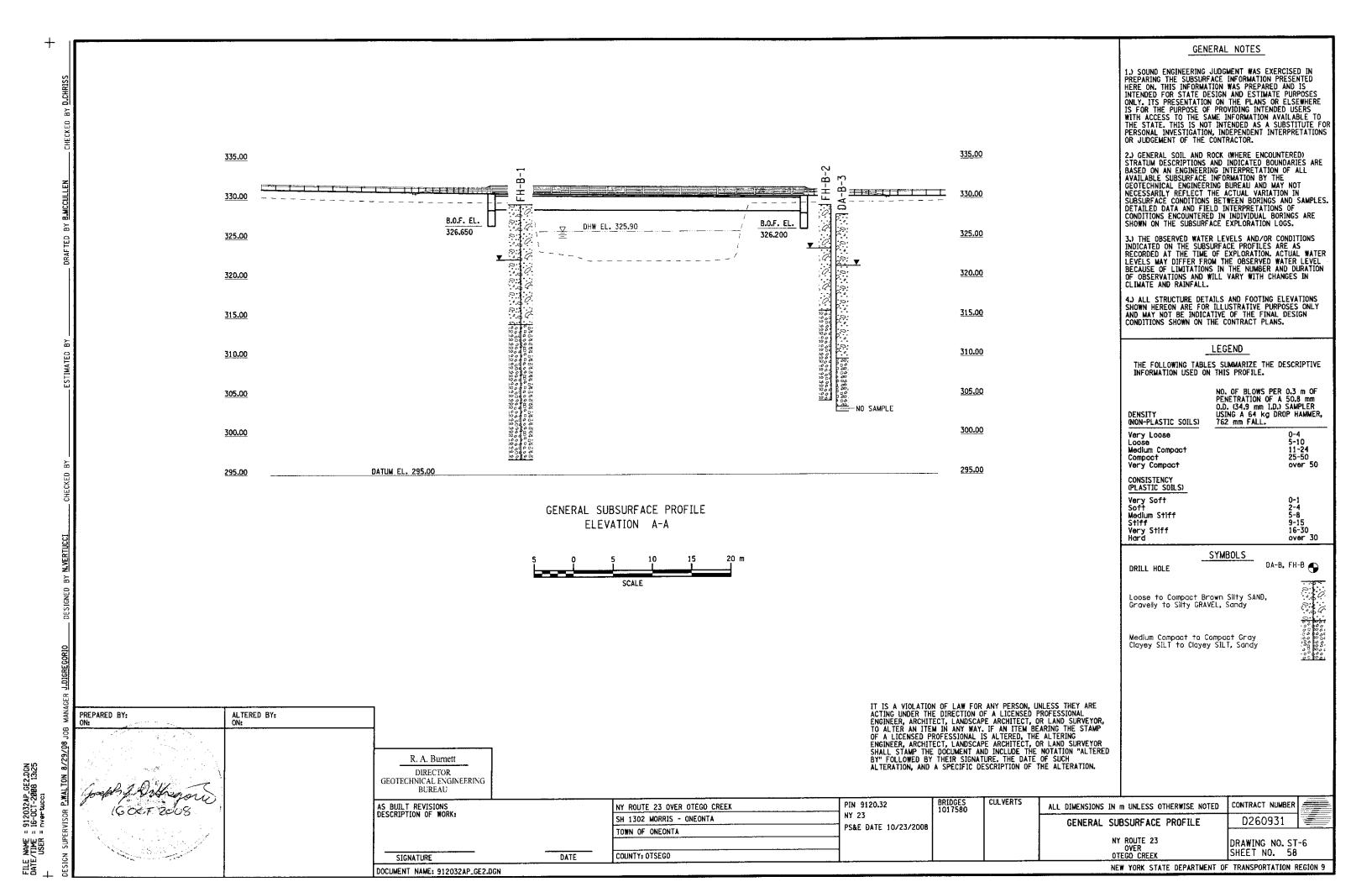
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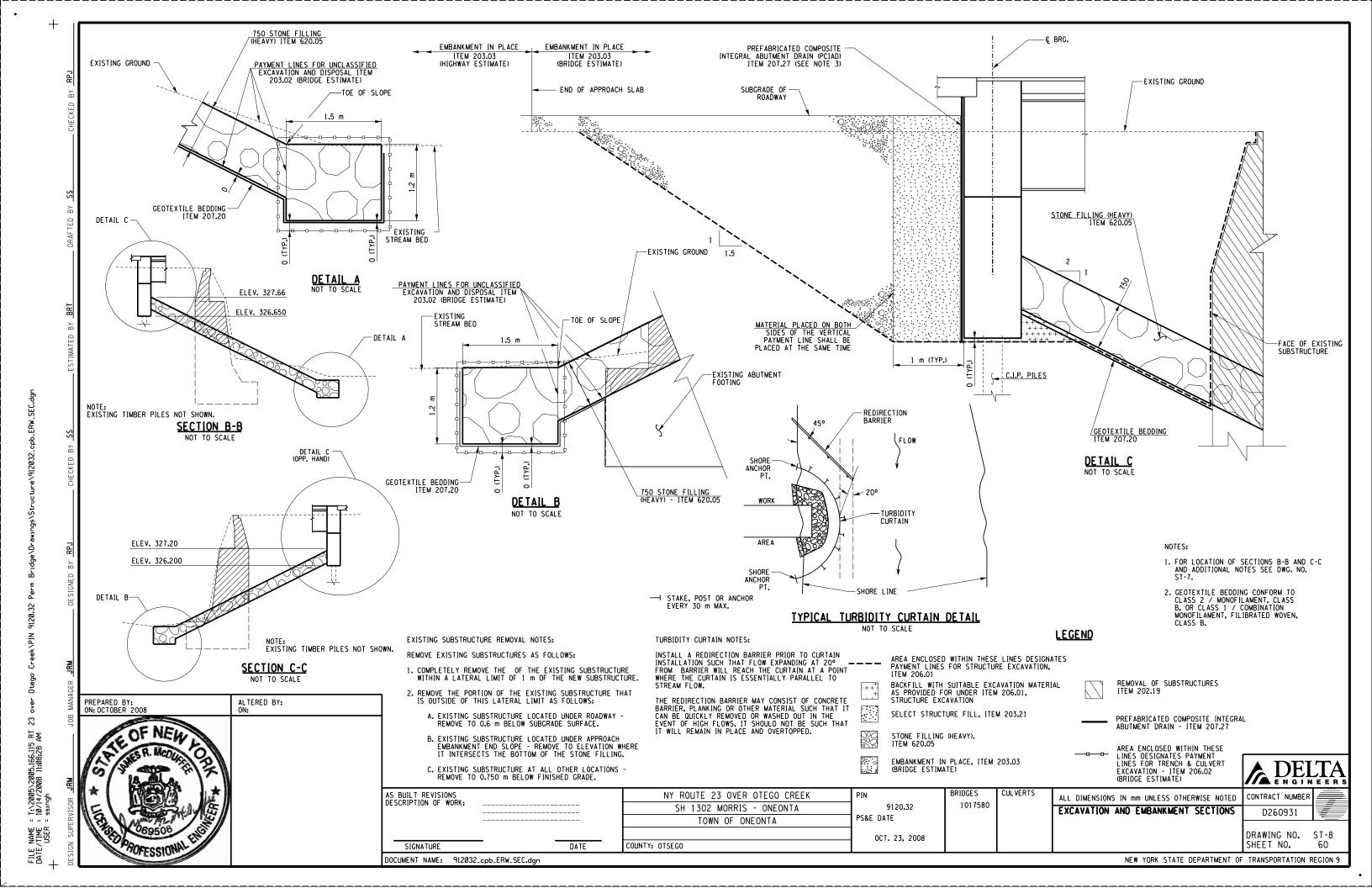
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COUNTY: OTSEGO

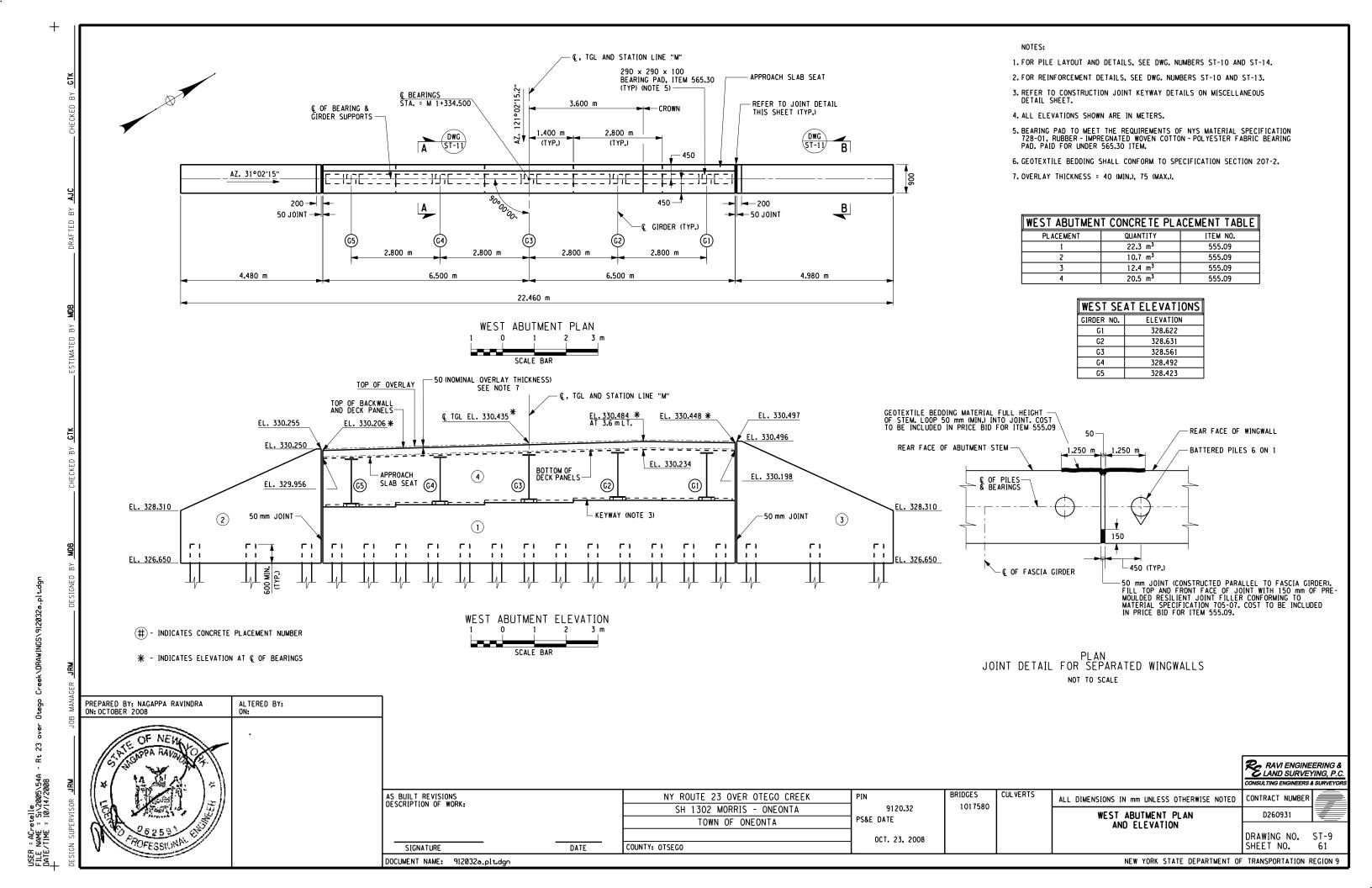
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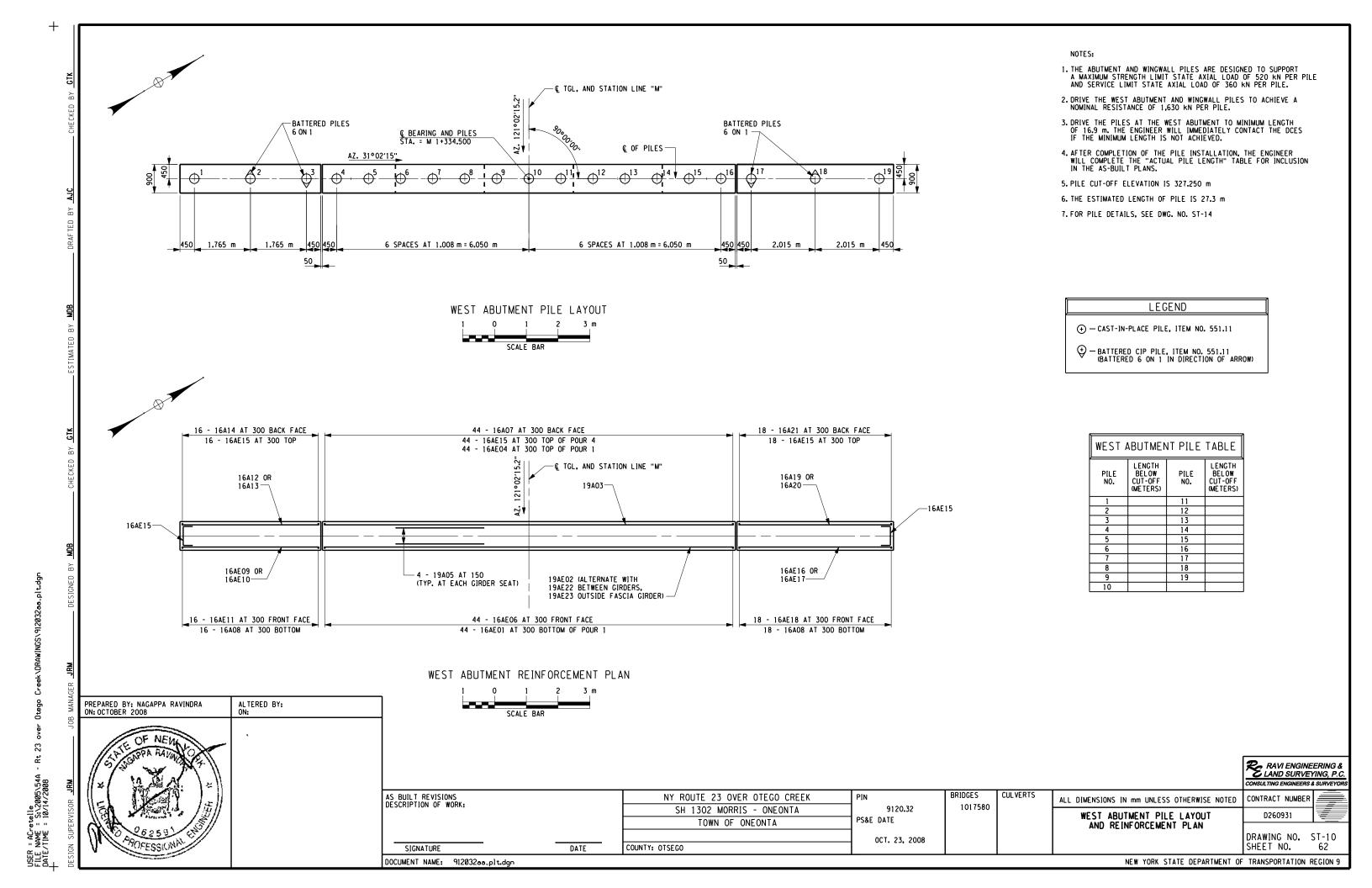
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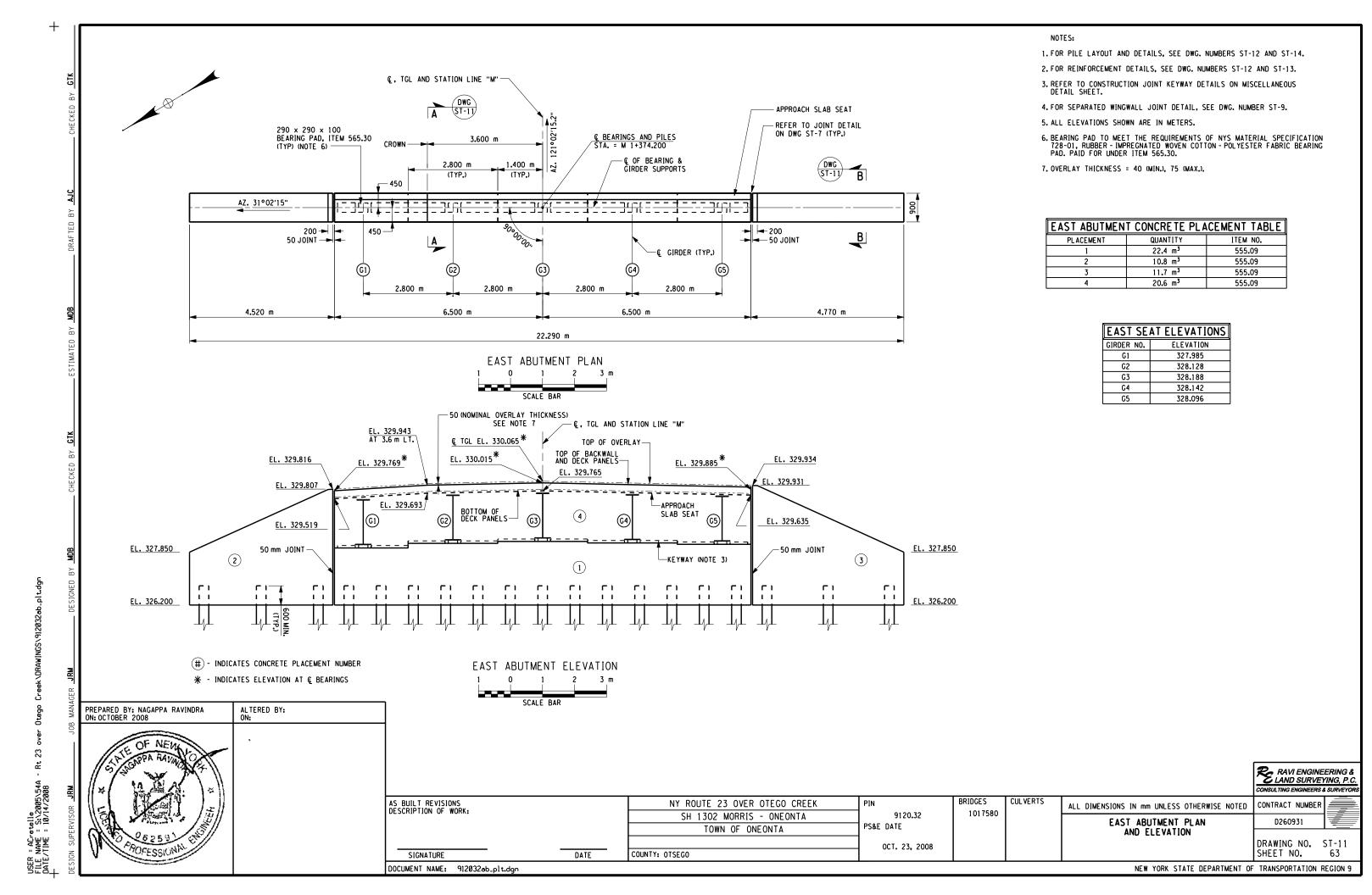


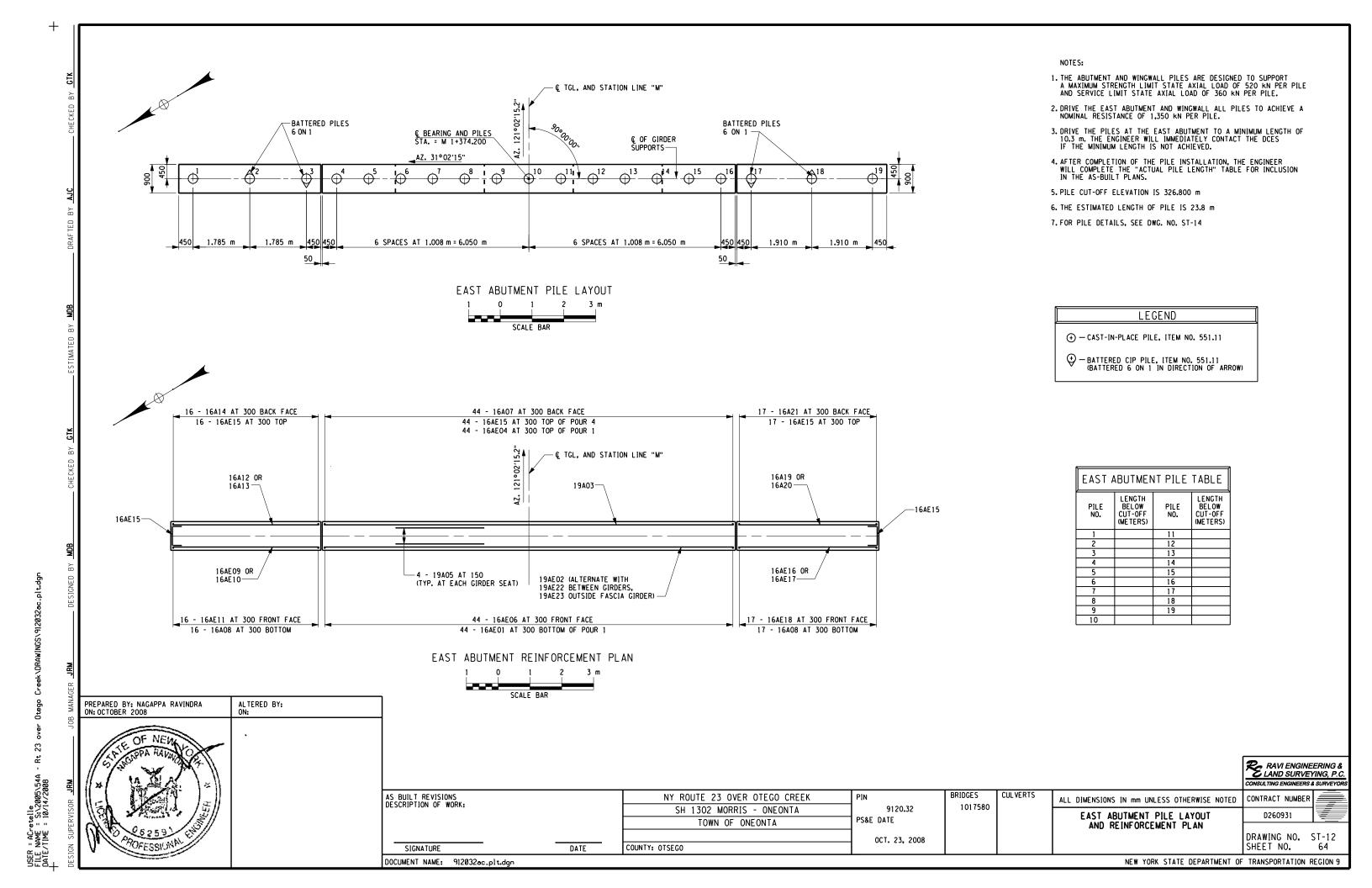


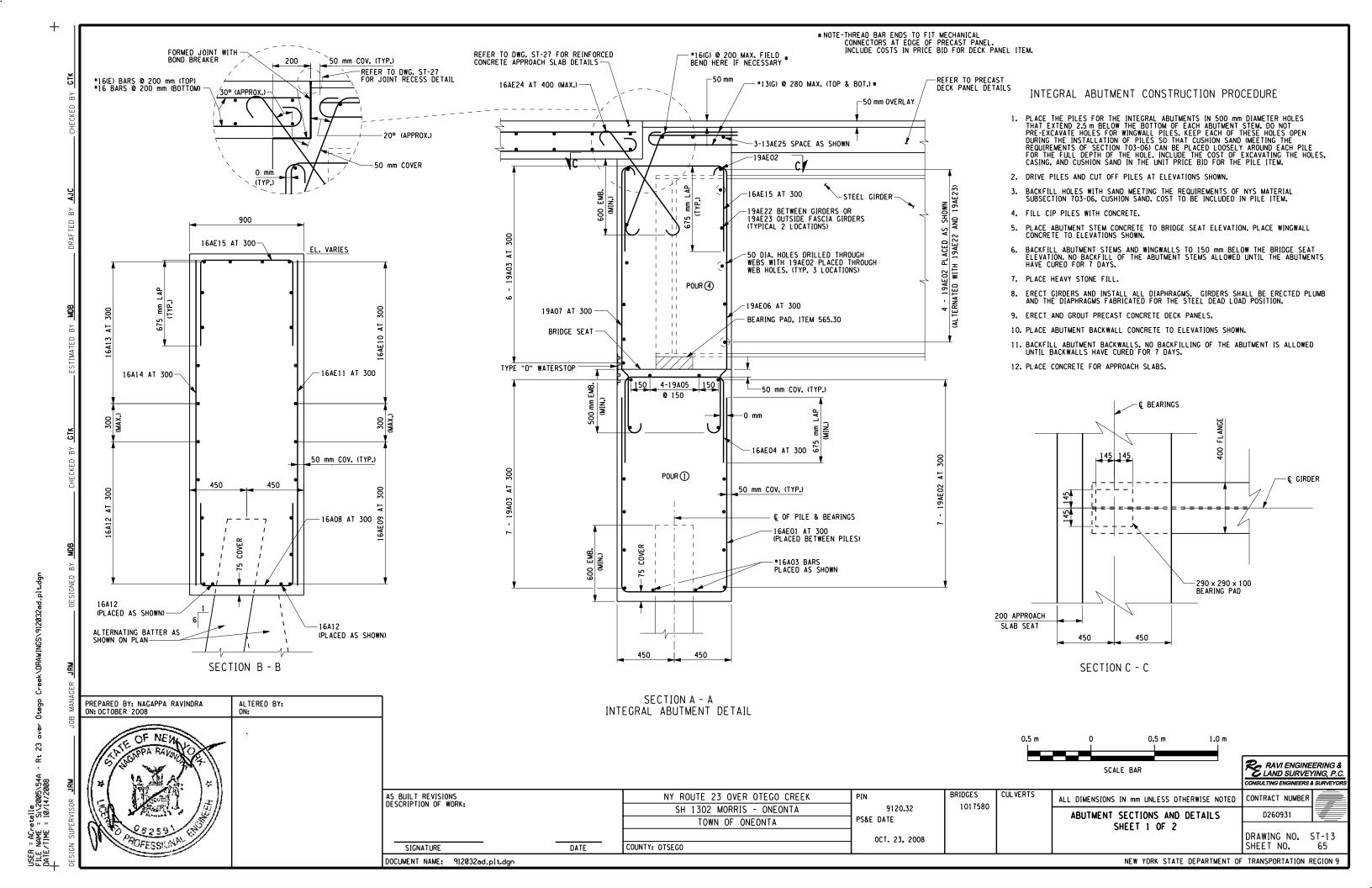


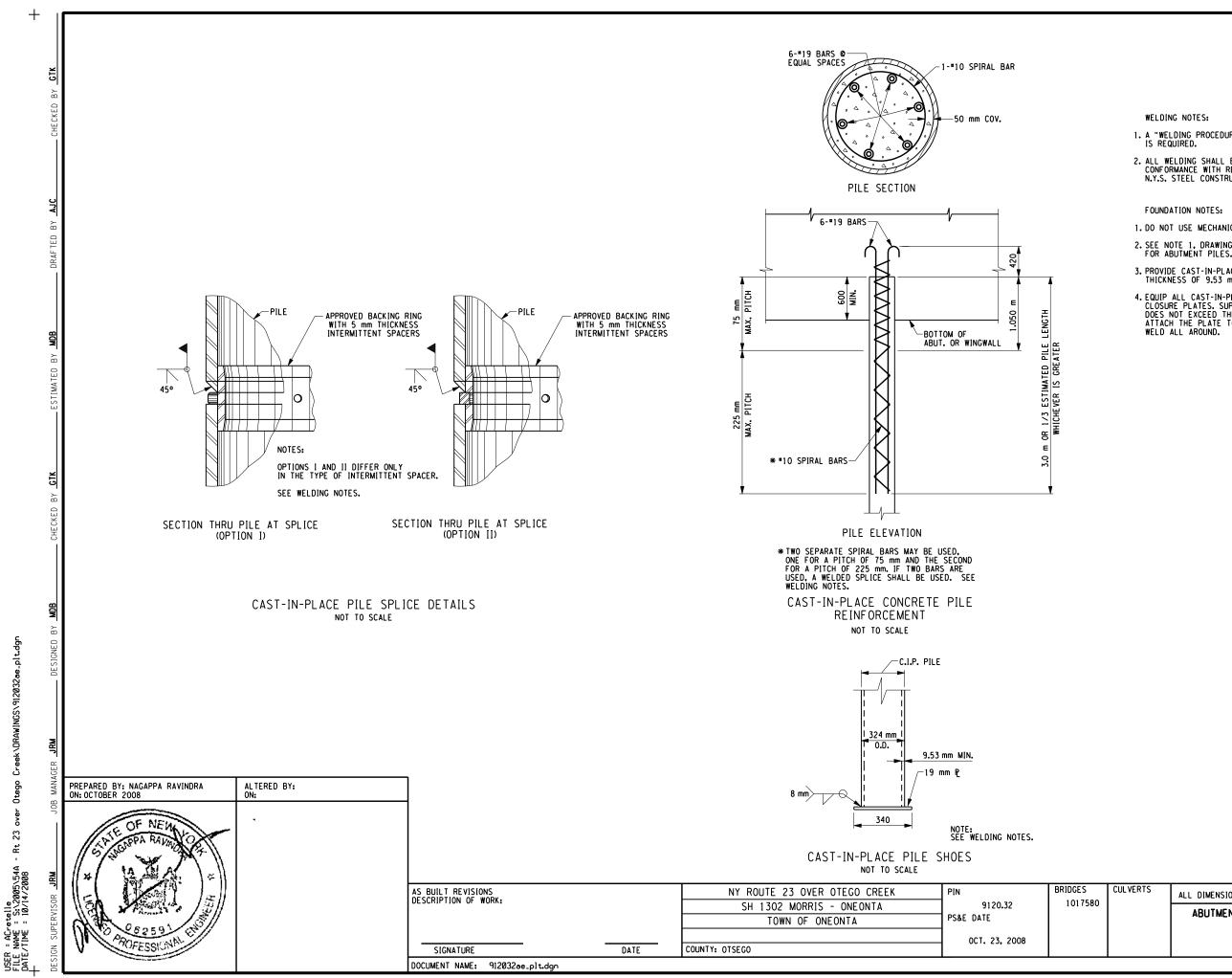












- 1. A "WELDING PROCEDURE SPECIFICATION" (WPS) APPROVED BY THE D.C.E.S. IS REQUIRED.
- 2. ALL WELDING SHALL BE PERFORMED BY A CERTIFIED WELDER IN CONFORMANCE WITH REQUIREMENTS FOR WELDING SPECIFIED IN THE N.Y.S. STEEL CONSTRUCTION MANUAL.
- 1. DO NOT USE MECHANICAL PILE SPLICES ON THIS STRUCTURE.
- 2. SEE NOTE 1. DRAWING ST-12 FOR PRE-EXCAVATION REQUIREMENTS FOR ABUTMENT PILES.
- 3. PROVIDE CAST-IN-PLACE CONCRETE PILES WITH A MINIMUM WALL THICKNESS OF 9.53 mm.
- 4. EQUIP ALL CAST-IN-PLACE CONCRETE PILES WITH 19 mm THICK FLAT CLOSURE PLATES. SUPPLY A FLAT PLATE WITH A DIAMETER THAT DOES NOT EXCEED THE PILE DIAMETER BY MORE THAT 16 mm. ATTACH THE PLATE TO THE PILE WITH AN 8 mm FILLET WELD. WELD ALL AROUND.

RAVI ENGINEERING & LAND SURVEYING, P.C. CONSULTING ENGINEERS & SURVEYO

ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED 1017580 ABUTMENT SECTIONS AND DETAILS SHEET 2 OF 2

PS&E DATE

OCT. 23, 2008

TOWN OF ONEONTA

COUNTY: OTSEGO

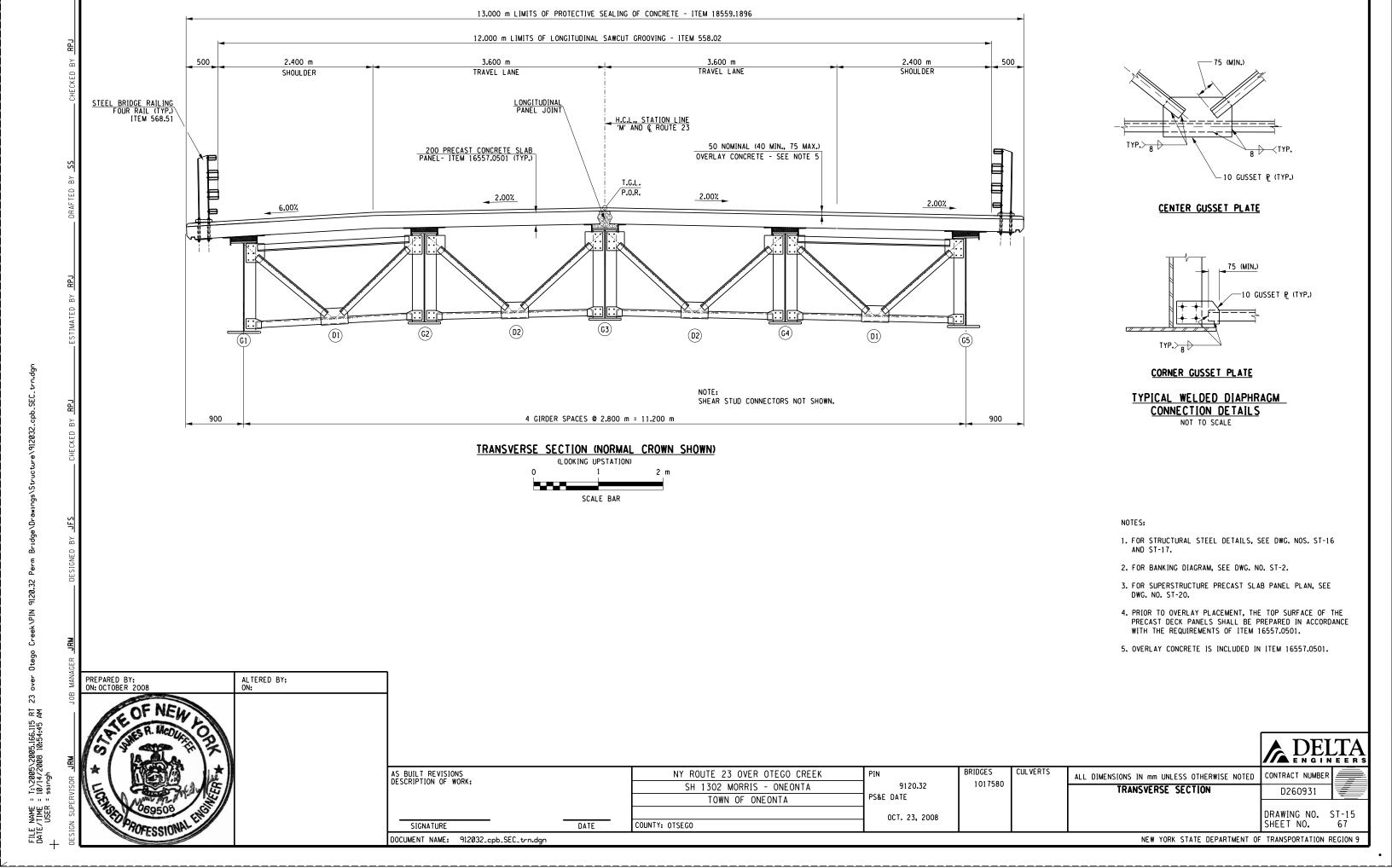
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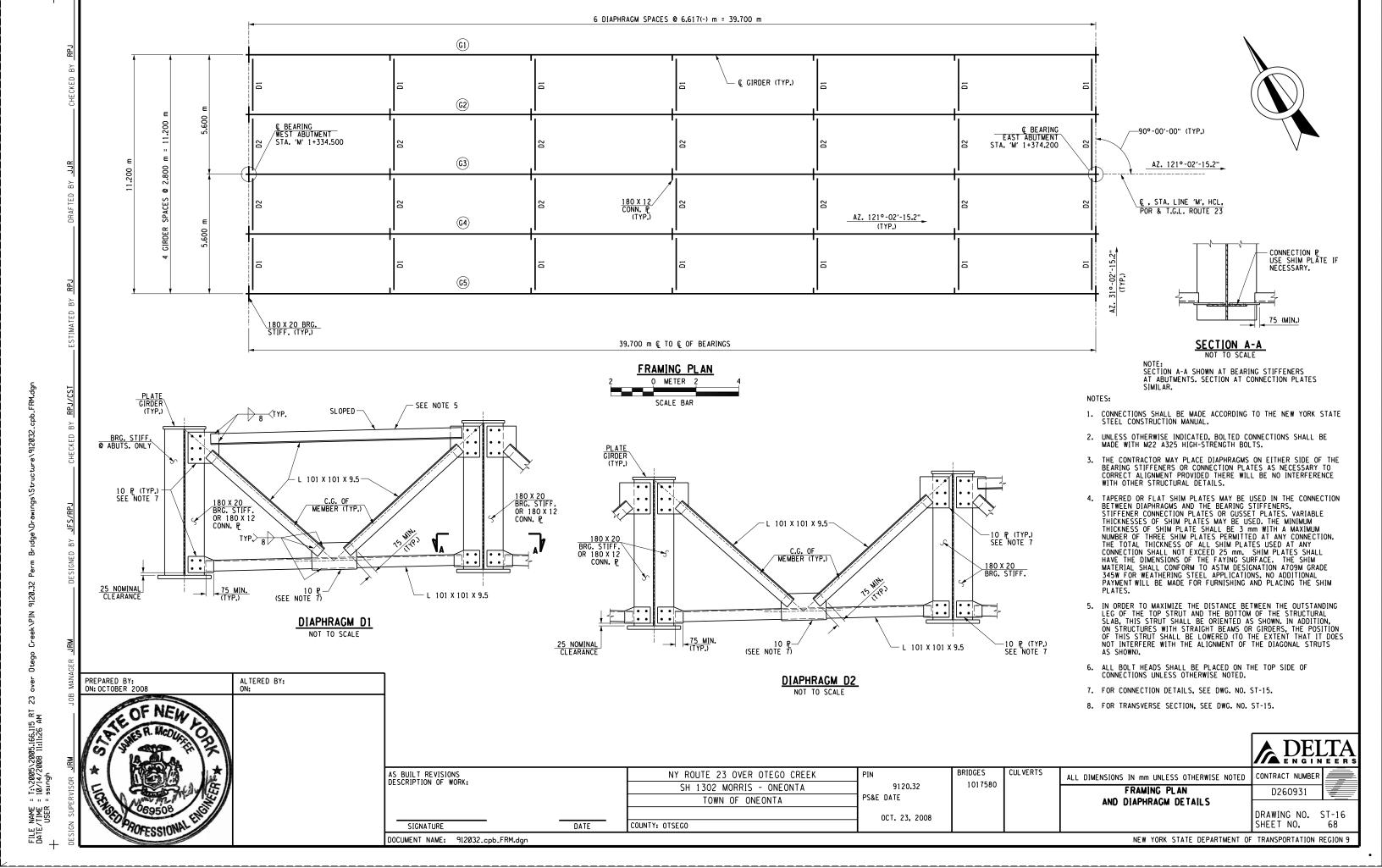
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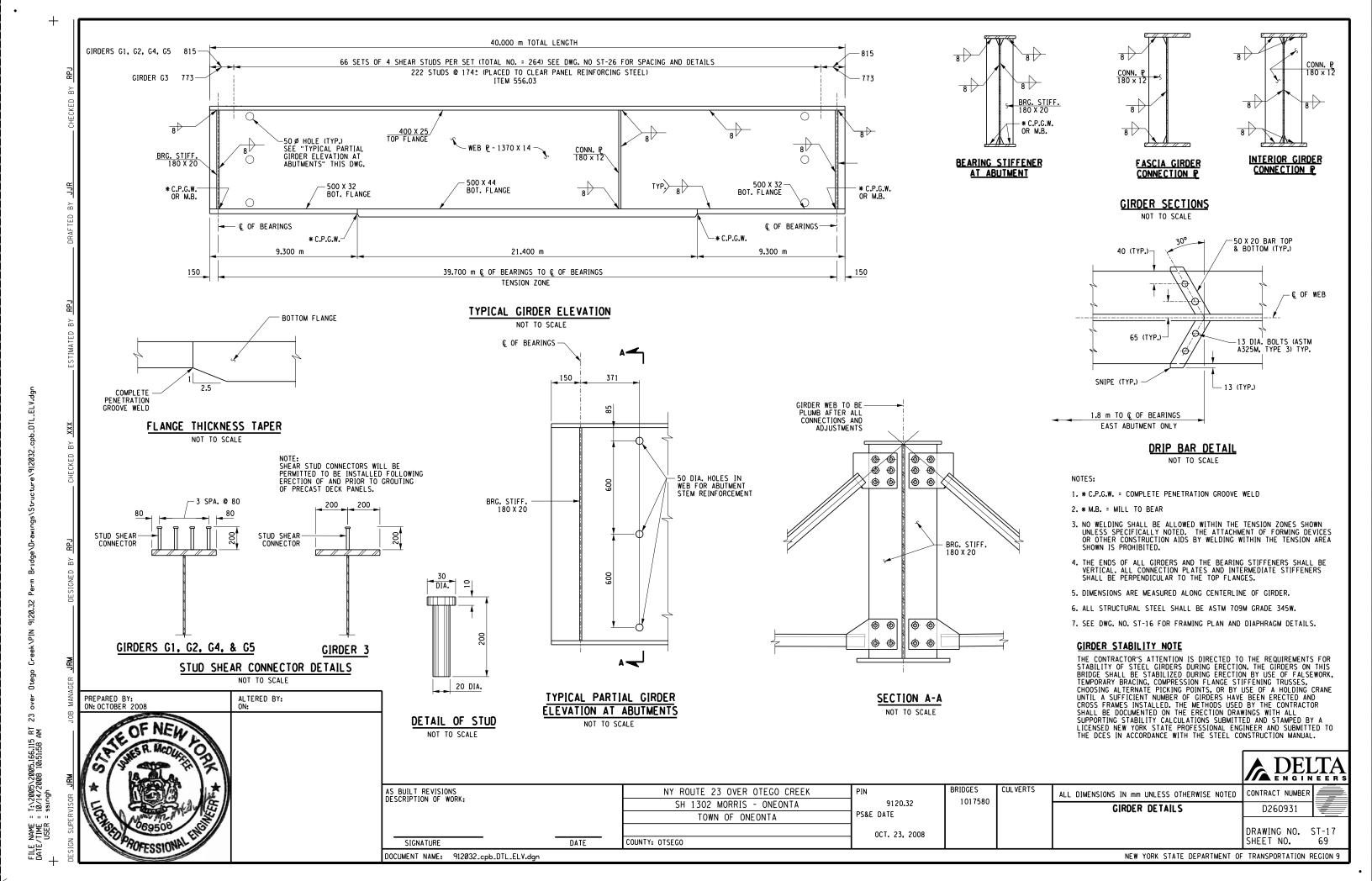
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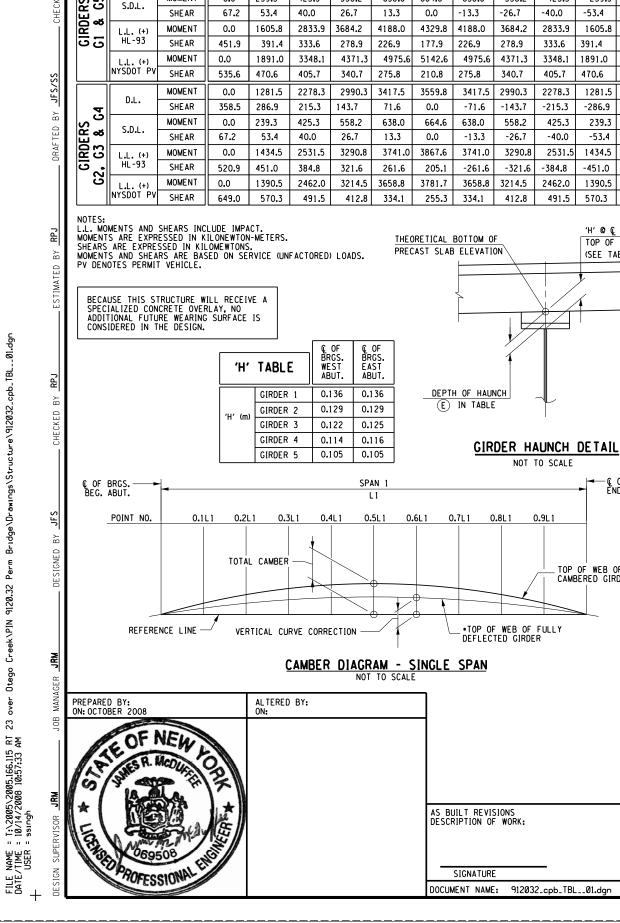
CONTRACT NUMBER D260931

DRAWING NO. ST-14 SHEET NO.









0.2 L

2188.7

425.3

206.8

0.3 L<sub>1</sub>

2872.7

137.9

558.2

0.4 L<sub>1</sub>

3283.1

68.9

638.0

MOMENT & SHEAR

TABLE

MOMENT

SHEAR

MOMENT

0.0

0.0

344.7

1231.2

275.8

239.3

€ OF BRGS.

ABUT.

0.0

-344.7

0.0

-67.2

0.0

451.9

0.0

535.6

0.0

-358.5

0.0

-67.2

0.0

-520.9

0.0

649.0

'H' @ @ BRG. FROM

TOP OF WEB

(SEE TABLE)

TOP OF WEB OF FULLY CAMBERED GIRDER

DATE

COUNTY: OTSEGO

0.6 L

-68.9

638.0

0.7 L<sub>1</sub>

558.2

3283.1 2872.7

0.8 L<sub>1</sub>

2188.7

-206.8

425.3

-40.0

2833.9

333.6

3348.1

405.7

-215.3

425.3

-40.0

2531.5

-384.8

2462.0

491.5

0.9 L

1231.2

-275.8

-53.4

391.4

1891.0

470.6

1281.5

-286.9

239.3

-53.4

1434.5

-451.0

1390.5

570.3

239.3

1605.8

0.5 L<sub>1</sub>

3419.9

0.0

664.6

	HAUNCH TABLE	1)*	② <sup>*</sup>	<b>3</b> *	4*	⑤ <b>*</b>	<b>6</b> *	7*	8*	<b>9</b> *	10*	11)*	12*
5	A REQ'D BOTTOM OF PRECAST SLAB ELEVATION	330.250	330.177	330.106	330.038	329.973	329.911	329.852	329.796	329.743	329.694	329.648	329.625
ıı	B TOP OF STEEL EL. (FIELD MEASURE)												
IRDER	C = A - B												
<u>~</u>	D CONCRETE + S.D.L. DEFLECTION	0.005	0.045	0.080	0.108	0.127	0.138	0.138	0.127	0.108	0.080	0.045	0.005
ပ	E DEPTH OF HAUNCH REQ'D = C + D (m)												
<u>ي</u>	A REQ'D BOTTOM OF PRECAST SLAB ELEVATION	330.255	330.194	330.136	330.081	330.028	329.979	329.933	329.890	329.851	329.814	329.781	329.761
- 1	B TOP OF STEEL EL. (FIELD MEASURE)												
KUEK	C = A - B												
	D CONCRETE + S.D.L. DEFLECTION	0.005	0.046	0.083	0.112	0.133	0.143	0.143	0.133	0.112	0.085	0.046	0.005
<u>د</u>	(E) DEPTH OF HAUNCH REQ'D = (C)+(D)(m)												
3	A REQ'D BOTTOM OF PRECAST SLAB ELEVATION	330.179	330.131	330.086	330.043	330.004	329.968	329.935	329.905	329.878	329.855	329.834	329.817
<u>۔</u>	B TOP OF STEEL EL. (FIELD MEASURE)												
الد	C = A - B												
3	D CONCRETE + S.D.L. DEFLECTION	0.005	0.046	0.083	0.112	0.133	0.143	0.143	0.133	0.112	0.085	0.046	0.005
ဗျ	(E) DEPTH OF HAUNCH REQ'D = (C)+(D)(m)												
ठ	(A) REQ'D BOTTOM OF PRECAST SLAB ELEVATION	330.103	330.068	330.030	329.987	329.948	329.912	329.879	329.849	329.822	329.799	329.778	329.761
- 1	B TOP OF STEEL EL. (FIELD MEASURE)												
품	© = A - B												
운	D CONCRETE + S.D.L. DEFLECTION	0.005	0.046	0.083	0.112	0.133	0.143	0.143	0.133	0.112	0.085	0.046	0.005
ဗျ	(E) DEPTH OF HAUNCH REQ'D = (C)+(D)(m)												
ည	(A) REQ'D BOTTOM OF PRECAST SLAB ELEVATION	330.027	330.005	329.974	329.931	329.892	329.856	329.823	329.793	329.766	329.743	329.722	329.705
- 1	B TOP OF STEEL EL. (FIELD MEASURE)												
EE EE	© = A - B												
S.	(D) CONCRETE + S.D.L. DEFLECTION	0.005	0.045	0.080	0.108	0.127	0.138	0.138	0.127	0.108	0.080	0.045	0.005
ပ	E DEPTH OF HAUNCH REQ'D = C + D (m)						אי טבאיס	TEC EDOE	OF END	DANEL OD	€ JOINT	DETWEEN	INTERMED

	CAMBER TABLE	BRGS. WEST ABUT.	0.1 L <sub>1</sub>	0.2 L <sub>1</sub>	0.3 L <sub>1</sub>	0.4 L <sub>1</sub>	0.5 L <sub>1</sub>	0.6 L <sub>1</sub>	0.7 L <sub>1</sub>	0.8 L <sub>1</sub>	0.9 L <sub>1</sub>	BRGS. EAST ABUT.
	I STEEL D.L. (m)	0.0	0.0123	0.0232	0.0315	0.0368	0.0386	0.0368	0.0315	0.0232	0.0123	0.0
స్ట్	II CONCRETE D.L. (m)	0.0	0.0392	0.0737	0.1002	0.1169	0.1227	0.1169	0.1002	0.0737	0.0392	0.0
	III SUPERIMPOSED D.L. (m)	0.0	0.0057	0.0108	0.0146	0.0170	0.0179	0.0170	0.0146	0.0108	0.0057	0.0
GIRDEI G1 &	IV VERTICAL CURVE (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	TOTAL = [+[[+[[]+[V (m)	0.0	0.057	0.108	0.146	0.171	0.179	0.171	0.146	0.108	0.057	0.0
	I STEEL D.L. (m)	0.0	0.0123	0.0232	0.0315	0.0368	0.0386	0.0368	0.0315	0.0232	0.0123	0.0
13.25 13.25	II CONCRETE D.L. (m)	0.0	0.0413	0.0777	0.1056	0.1232	0.1293	0.1232	0.1056	0.0777	0.0413	0.0
8.5	III SUPERIMPOSED D.L. (m)	0.0	0.0054	0.0102	0.0138	0.0162	0.0169	0.0162	0.0138	0.0102	0.0054	0.0
GIRDERS G2, G3 & G4	IV VERTICAL CURVE (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	TOTAL = I+II+III+IV (m)	0.0	0.059	0.111	0.151	0.176	0.185	0.176	0.151	0.111	0.059	0.0

## CAMBER NOTES:

NY ROUTE 23 OVER OTEGO CREEK

SH 1302 MORRIS - ONEONTA

TOWN OF ONEONTA

- 1. THE CAMBER LABELED "VERTICAL CURVE" IN THE TABLE IN THE CAMBER REQUIRED TO FOLLOW THE VERTICAL CURVES.
- 2. THE CAMBER LABELED "STEEL D.L." IN THE TABLE IS THE CAMBER REQUIRED TO OFFSET THE DEFLECTION DUE TO THE DEAD LOAD WEIGHT OF THE GIRDERS AS FABRICATED.
- 3. THE CAMBER LABELED "CONCRETE D.L." IN THE TABLE IS THE CAMBER REQUIRED TO OFFSET THE DEFLECTION DUE TO THE DEAD LOAD WEIGHT OF THE PRECAST CONCRETE SLAB UNITS, GROUTED HAUNCHES, AND JOINTS.
- 4. THE CAMBER LABELED "SUPERIMPOSED D.L." IN THE TABLE IS THE CAMBER REQUIRED TO OFFSET THE DEFLECTION DUE TO THE WEIGHT OF THE BARRIER AND SPECIALIZED CONCRETE
- 5. THE TOTAL CAMBER IS THE SUM OF VERTICAL CURVE, STEEL DEAD LOAD, CONCRETE DEAD LOAD AND SUPERIMPOSED DEAD LOAD. ALL CAMBER OFFSETS ARE MEASURED VERTICALLY TO THE TOP OF WEB FROM A STRAIGHT REFERENCE LINE DRAWN FROM THE INTERSECTION OF TOP OF WEB AND CENTERLINE OF BEARINGS AT ONE END OF THE GIRDER TO THE CORRESPONDING POINT AT THE OTHER END OF THE GIRDER.
- 6. POSITIVE NUMBERS IN THE TABLE ARE ABOVE THE STRAIGHT REFERENCE LINE.
- 7. NEGATIVE NUMBERS IN THE TABLE ARE BELOW THE STRAIGHT REFERENCE LINE.

1017580

8. THE CAMBER OFFSETS ARE TABULATED IN DECIMALS OF A METER.

9120.32

OCT. 23, 2008

PS&E DATE



€ OF

ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED SUPERSTRUCTURE TABLES (1 OF 2)

D260931 DRAWING NO. ST-18

SHEET NO.

FILE NAME : DATE/TIME : USER :

2\*\* (3)\*\* 4)\*\* (5)\*\* 6)\*\* 8)\*\* (10)\*\* (12)\*\* (11)\*\* (7)\*\* 9)\*\* OVERLAY TABLE A REQUIRED TOP OF ROADWAY ELEVATION 330.500 330.046 329.944 | 329.898 | 329.875 330.427 330.356 | 330.288 | 330.223 | 330.161 330.102 329.993 0.001 0.006 0.010 0.014 0.016 0.018 0.016 0.014 0.010 0.006 0.001 B) S.D.L. DEFLECTION 0.018  $\widehat{C}$  =  $\widehat{A}$  +  $\widehat{B}$  ADJUSTED TOP OF ROADWAY ELEVATION 330.501 330.433 330.366 330.302 | 330.239 330.179 330.120 330.062 330.007 329.954 329.904 329.876 (D) TOP OF PANEL ELEVATION (REQUIRED) 330.383 | 330.316 | 330.252 | 330.189 | 330.129 | 330.070 | 330.012 | 329.957 | 329.904 | 329.854 | 329.826 330.451 (E) TOP OF PANEL ELEVATION (FIELD MEASURED) F ADJUSTMENT TO TOP OF PANEL = D - E (m) G DEPTH OF OVERLAY = C - D (m) • (A) REQUIRED TOP OF ROADWAY ELEVATION 330.505 330.444 | 330.386 | 330.331 | 330.278 | 330.229 | 330.183 | 330.140 | 330.101 | 330.064 | 330.031 | 330.011 B S.D.L. DEFLECTION 0.001 0.005 0.010 0.013 0.015 0.017 0.017 0.015 0.013 0.010 0.005 0.001 (C) = (A) + (B) ADJUSTED TOP OF ROADWAY ELEVATION 330.506 330.449 330.396 330.344 330.293 330.246 330.200 330.155 330.114 330.074 330.036 330.012 (D) TOP OF PANEL ELEVATION (REQUIRED) 330.346 | 330.294 | 330.243 | 330.196 330.456 330.399 330.150 330.105 330.064 330.024 329.986 329.962 (E) TOP OF PANEL ELEVATION (FIELD MEASURED)  $\widehat{F}$  ADJUSTMENT TO TOP OF PANEL =  $\widehat{D}$  -  $\widehat{E}$  (m) G DEPTH OF OVERLAY = (C)-(D)(m) • (A) REQUIRED TOP OF ROADWAY ELEVATION 330.429 330.381 330.336 330.293 330.254 330.218 330.185 330.155 330.128 330.105 330.084 330.067 B S.D.L. DEFLECTION 0.001 0.005 0.010 0.013 0.015 0.017 0.017 0.015 0.013 0.010 0.005 0.001 (C) = (A) + (B) ADJUSTED TOP OF ROADWAY ELEVATION 330.430 330.386 330.346 | 330.306 | 330.269 | 330.235 330.202 330.170 | 330.141 | 330.115 | 330.089 | 330.068 D) TOP OF PANEL ELEVATION (REQUIRED) 330.380 330.336 | 330.296 | 330.256 | 330.219 | 330.185 | 330.152 | 330.120 | 330.091 | 330.065 | 330.039 | 330.018 (E) TOP OF PANEL ELEVATION (FIELD MEASURED) F ADJUSTMENT TO TOP OF PANEL =(D)-(E)(m) G DEPTH OF OVERLAY = C - D (m) • A REQUIRED TOP OF ROADWAY ELEVATION 330,353 330.318 | 330.280 | 330.237 | 330.198 | 330.162 | 330.129 | 330.099 | 330.072 | 330.049 | 330.028 | 330.011 B S.D.L. DEFLECTION 0.001 0.005 0.010 0.013 0.015 0.017 0.017 0.015 0.013 0.010 0.005 0.001 (C) = (A) + (B) ADJUSTED TOP OF ROADWAY ELEVATION 330.354 330.323 | 330.290 | 330.250 | 330.213 | 330.179 | 330.146 330.114 | 330.085 330.059 330.033 330.012 (D) TOP OF PANEL ELEVATION (REQUIRED) 330.304 330.273 | 330.240 | 330.200 | 330.163 | 330.129 | 330.096 330.064 330.035 330.009 329.983 E TOP OF PANEL ELEVATION (FIELD MEASURED) F ADJUSTMENT TO TOP OF PANEL = (D) - (E) (m) G DEPTH OF OVERLAY = C - D (m) • (A) REQUIRED TOP OF ROADWAY ELEVATION 330.255 330.224 330.181 330.142 330.106 330.073 330.043 | 330.016 | 329.993 | 329.972 | 329.955 0.014 0.010 (B) S.D.L. DEFLECTION 0.001 0.006 0.010 0.014 0.016 0.018 0.018 0.016 0.006 0.001 C = A + B ADJUSTED TOP OF ROADWAY ELEVATION 330.234 | 330.195 | 330.158 | 330.124 330.059 | 330.030 | 330.003 | 329.978 | 330,278 330.261 330.091 329.956 D) TOP OF PANEL ELEVATION (REQUIRED) 330.228 330.211 330.184 | 330.145 | 330.108 | 330.074 330.041 330.009 | 329.980 329.953 | 329.928 329.906 (E) TOP OF PANEL ELEVATION (FIELD MEASURED) (F) ADJUSTMENT TO TOP OF PANEL =(D)-(E) (m)

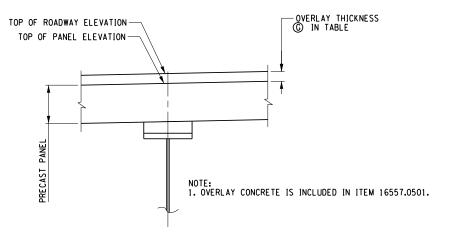
IN SOME CASES, THE ADJUSTMENT TO THE TOP OF PANEL THAT IS REQUIRED IN ROW (F) CANNOT BE FULLY ACHIEVED IN THE FIELD. IF THIS IS THE CASE, THE VALUE COMPUTED FOR THE DEPTH OF OVERLAY IN ROW (G) SHOULD USE THE REQUIRED TOP OF ROADWAY ELEVATION VALUE FROM ROW (A), AND THE ACTUAL TOP OF PANEL ELEVATION IN THE FIELD AFTER ADJUSTMENTS TO COME UP WITH THE FINAL OVERLAY DEPTH IN ROW (C)

G DEPTH OF OVERLAY = (C) - (D)(m) •

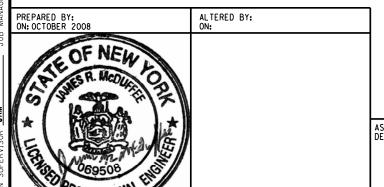
•• DENOTES EDGE OF END PANEL OR Q JOINT BETWEEN INTERMEDIATE PANELS. SEE DWG. NO. ST-20 FOR PANEL JOINT DESIGNATIONS.

		DESIGN LOAD	TABLE
		UNIT	LOAD KN/m
		PRECAST SLAB	12.489
		HAUNCH	0.707
65	D.L.	GIRDER	3.785
ంర	0	DIAPHRAGMS	0.107
5			
		TOTAL	17.088
GIRDERS			
ğ	ا نــ	RAILING	0.250
5	S.D.L.	CONCRETE OVERLAY	3.370
$\sqsubseteq$		TOTAL	3.620
4		PRECAST SLAB	13.196
5		HAUNCH	0.707
త	D.L.	GIRDER	3.785
63	0	DIAPHRAGMS	0.212
<b>C2</b> .			
ပ်		TOTAL	17.900
RS.		RAILING	0.250
S	S.D.L	CONCRETE OVERLAY	3.370
GIRDERS	장		
Ĺ		TOTAL	3 <b>.</b> 620

SERVICE (UNFACTORED) LOADS SHOWN. ASSUMED LIVE LOAD = HL-93 AND NYSDOT PERMIT VEHICLE (MS-23 FOR LFD).



DECK OVERLAY DETAIL NOT TO SCALE



AS BUILT REVISIONS DESCRIPTION OF WORK: NY ROUTE 23 OVER OTEGO CREEK SH 1302 MORRIS - ONEONTA TOWN OF ONEONTA COUNTY: OTSEGO SIGNATURE DATE

BRIDGES **CUL VERTS** 1017580

ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED SUPERSTRUCTURE TABLES (2 OF 2)

CONTRACT NUMBER D260931

DRAWING NO. ST-19 SHEET NO.

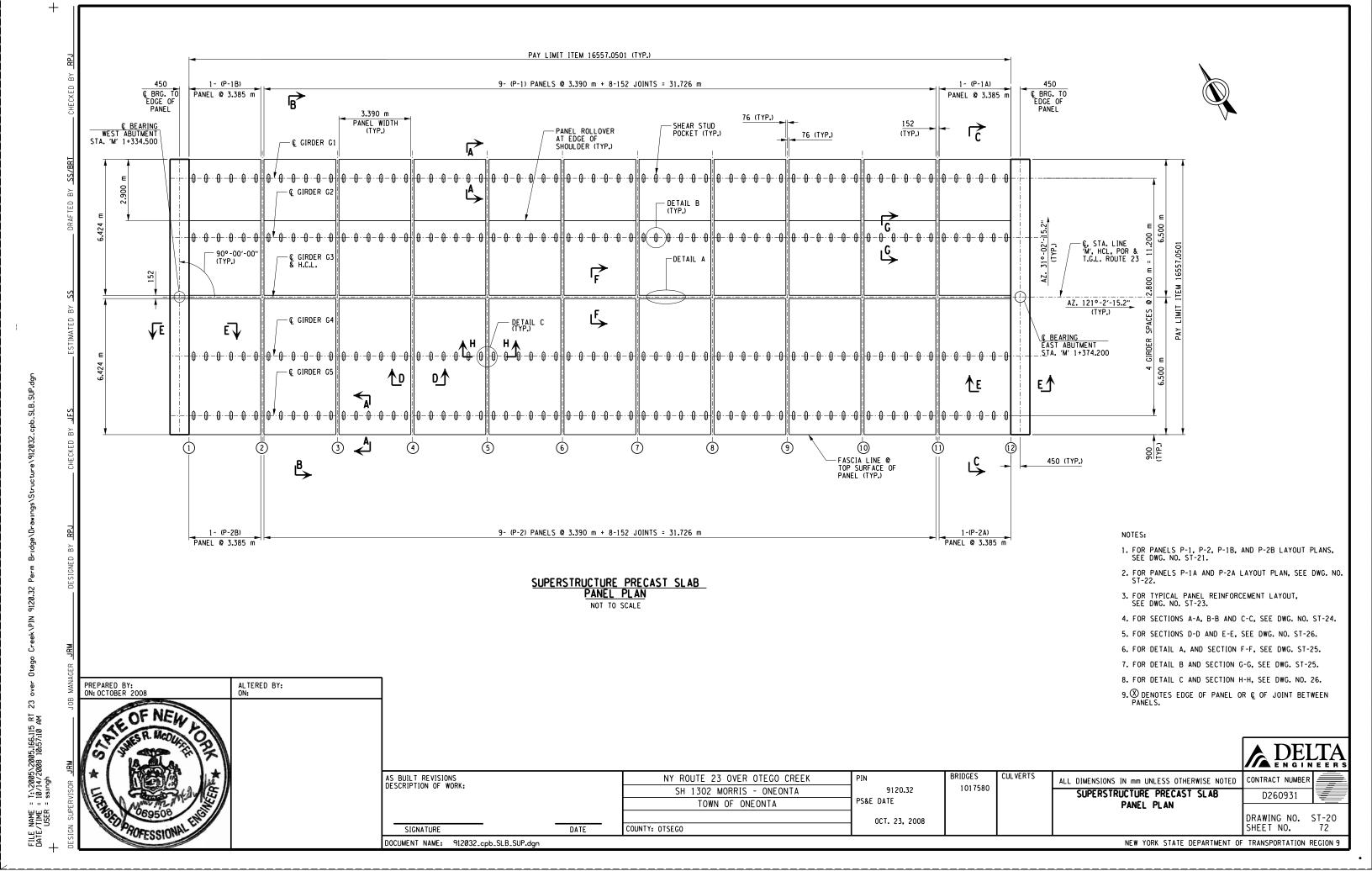
NEW YORK STATE DEPARTMENT OF TRANSPORTATION REGION 9

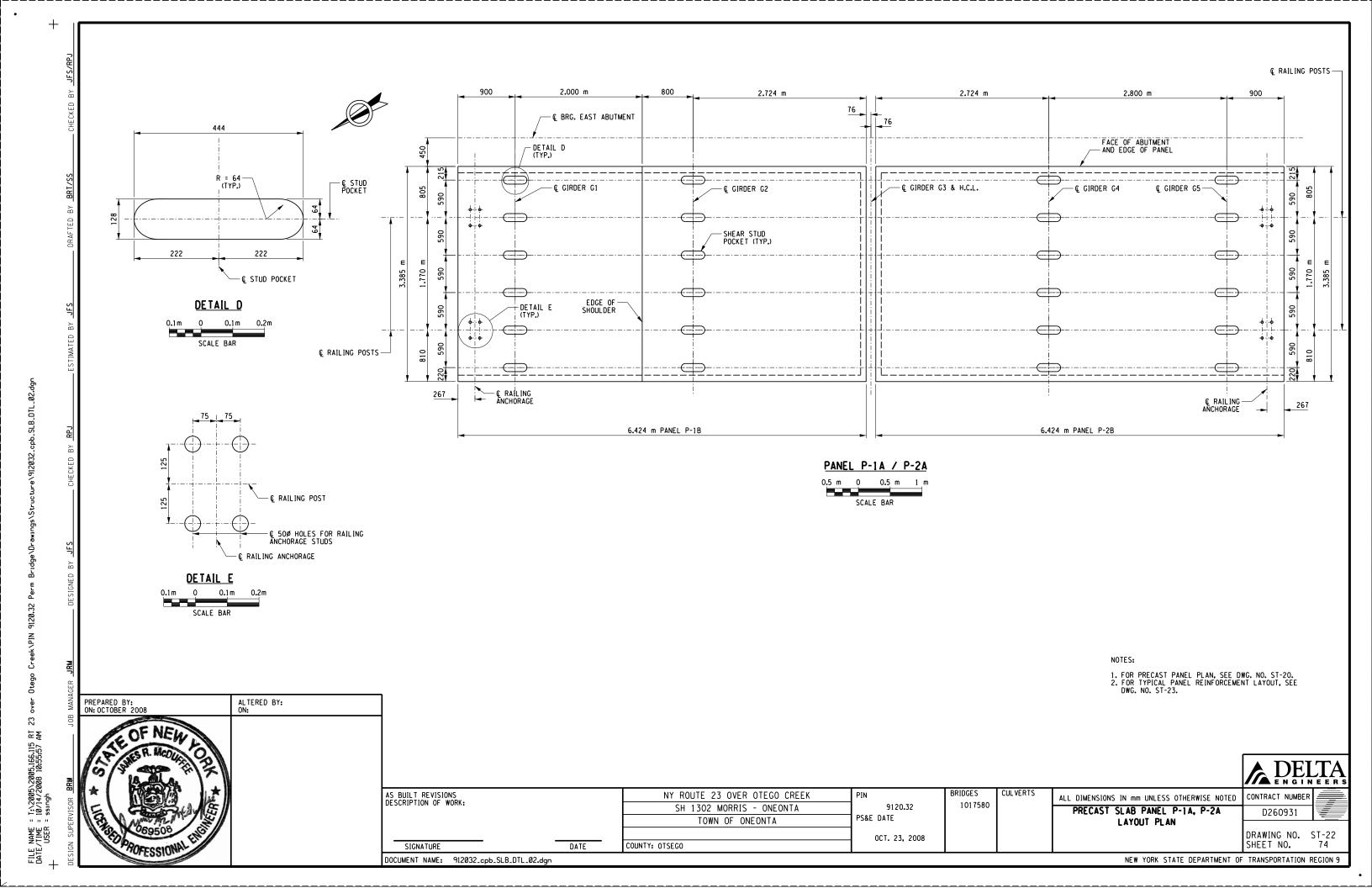
DOCUMENT NAME: 912032\_cpb\_TBL\_\_02.dqn

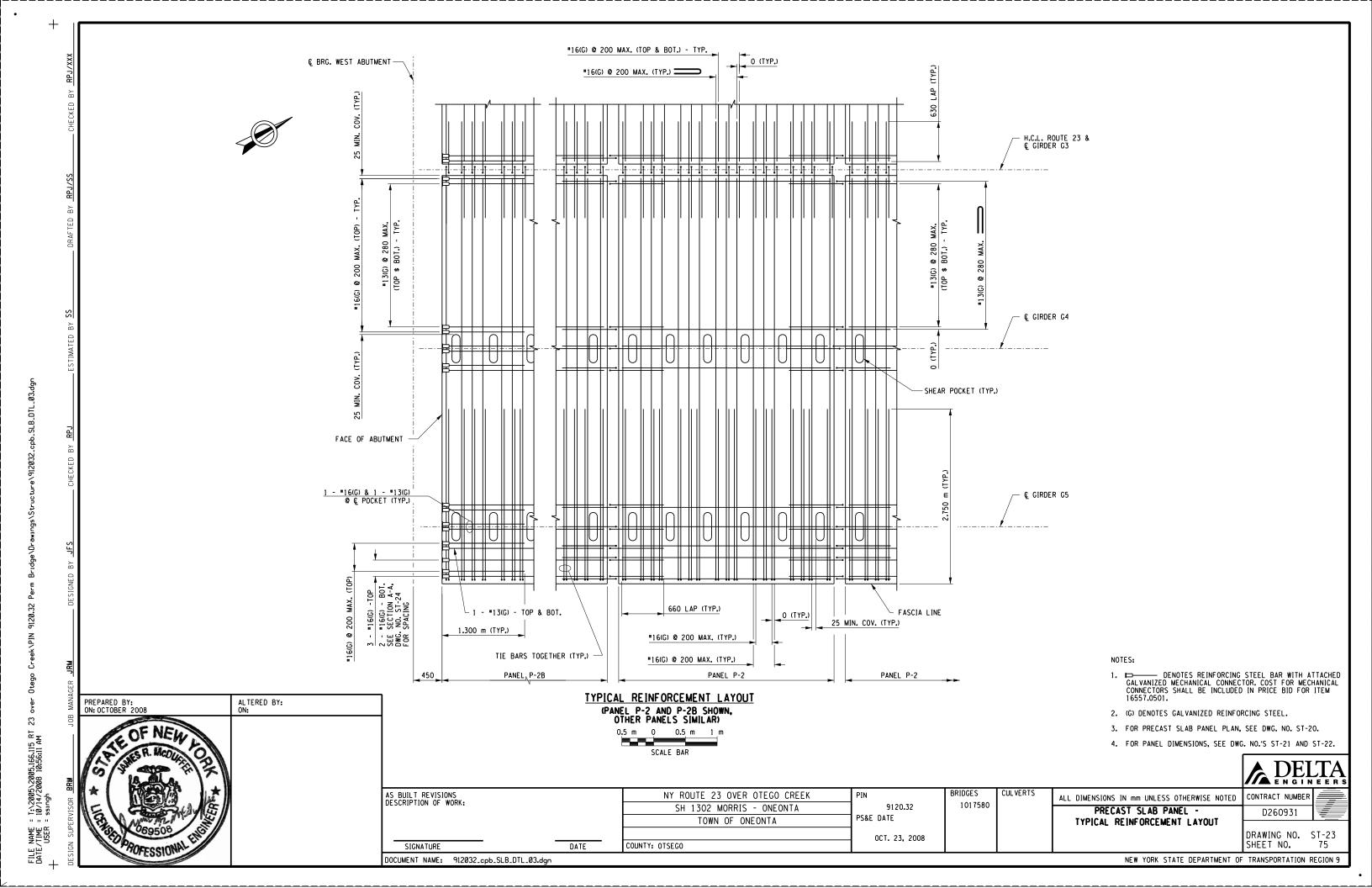
PS&E DATE

9120.32

OCT. 23, 2008

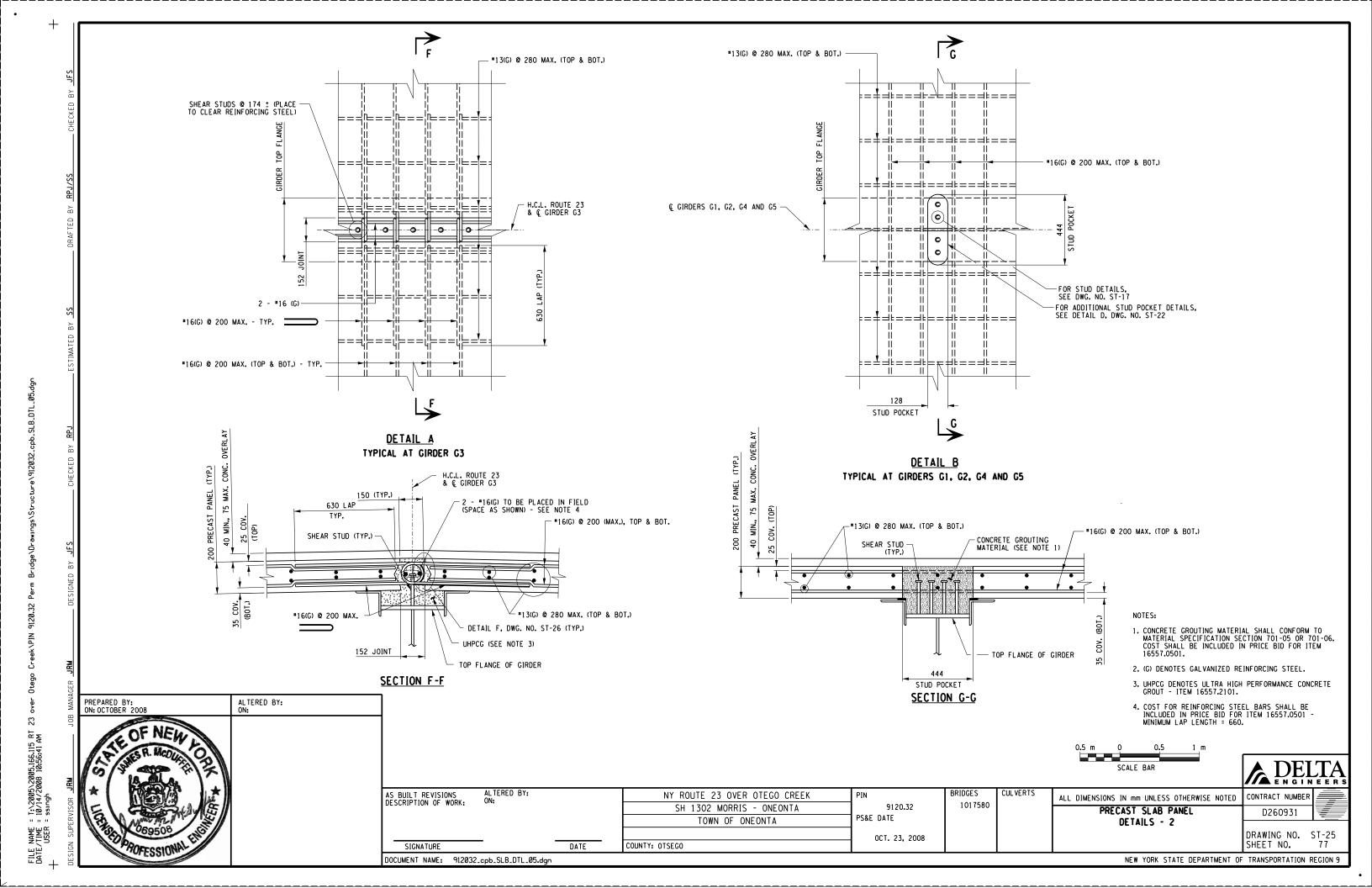




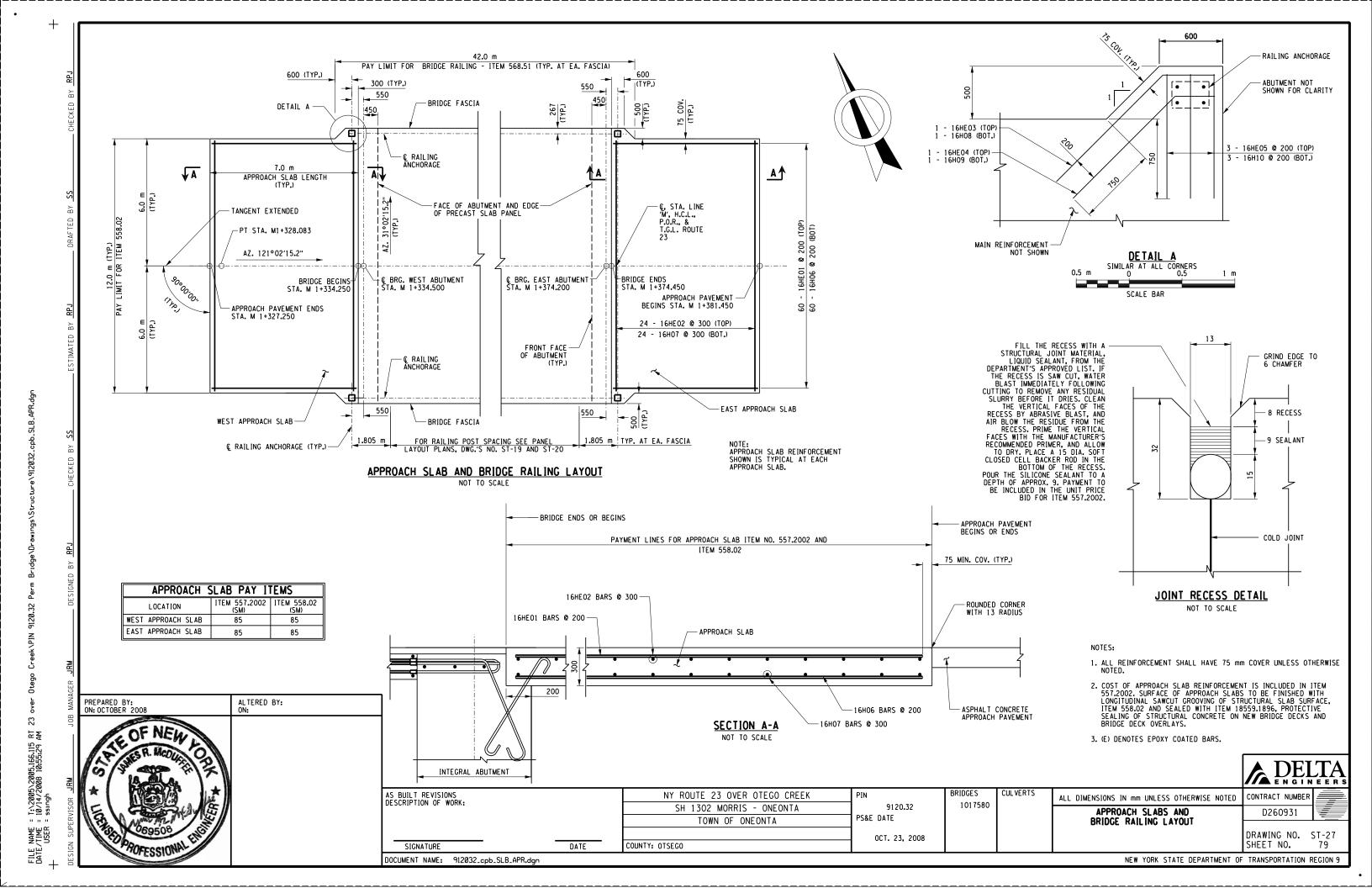


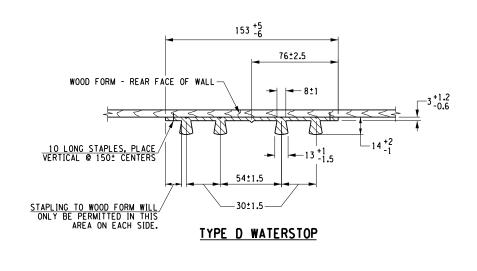
0tego

23



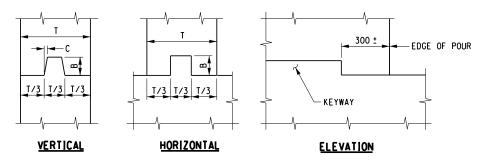
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## WATERSTOP NOTES:

- HOLES MUST NOT BE MADE IN WATERSTOP FOR ANY PURPOSE EXCEPT AS REQUIRED FOR STAPLING TO FORMS.
- 2. TYPE 'D' WATERSTOP SHALL BE LIGHT GRAY IN COLOR.
- 3. PVC USED IN WATERSTOPS SHALL CONFORM TO THE REQUIREMENTS OF SECTION 705-11 OF THE SPECIFICATIONS.
- THE COST OF FURNISHING AND PLACING WATERSTOP SHALL BE INCLUDED IN THE PRICE BID FOR THE CONCRETE ITEMS.
- 5. FIELD SPLICES SHALL BE AVOIDED IF POSSIBLE, HOWEVER, HEAT WELDED BUTT SPLICES WILL BE PERMITTED ON LONG STRAIGHT RUNS (GENERALLY IN EXCESS OF 15 METERS) AT POINTS APPROVED BY THE ENGINEER.
- 6. WATERSTOP SHALL BE SHIPPED IN STRAIGHT SECTIONS HAVING A MINIMUM LENGTH OF 3 METERS UNLESS SHORTER LENGTHS ARE REQUIRED.



#### NOTES:

- ALL KEYS SHOULD BEGIN AND END 300± FROM THE EDGE OF POUR AS INDICATED.
- 2. WATERSTOP NOT SHOWN,

# KEYWAY DETAILS

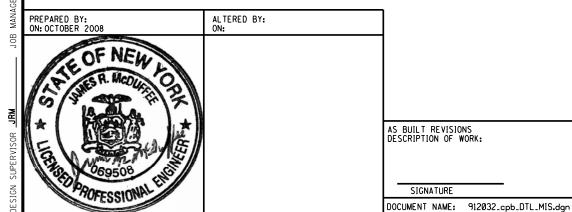
9120.32

OCT. 23, 2008

PS&E DATE

CONSTRUCTION & CONTRACTION JOINTS				
С	В	T/3		
5	40	0 TO 150		
10	90	150-250		
20	140	OVER 250		

EXPANSION JOINTS					
С	В	T/3			
10	90	0 TO 250			
20	140	OVER 250			



AS BUILT REVISIONS
DESCRIPTION OF WORK:

NY ROUTE 23 OVER OTEGO CREEK

SH 1302 MORRIS - ONEONTA

TOWN OF ONEONTA

SIGNATURE

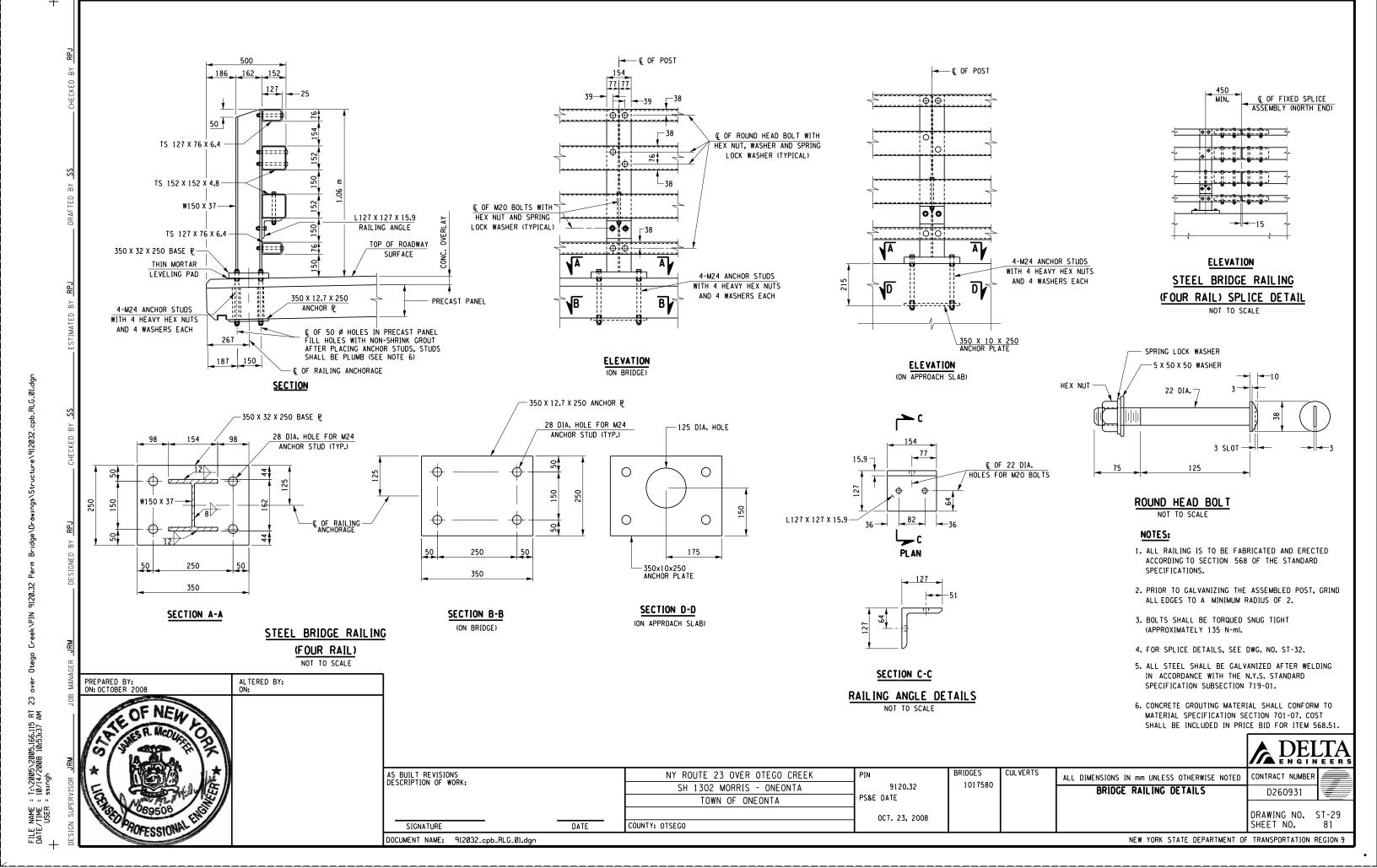
DATE

COUNTY: OTSEGO

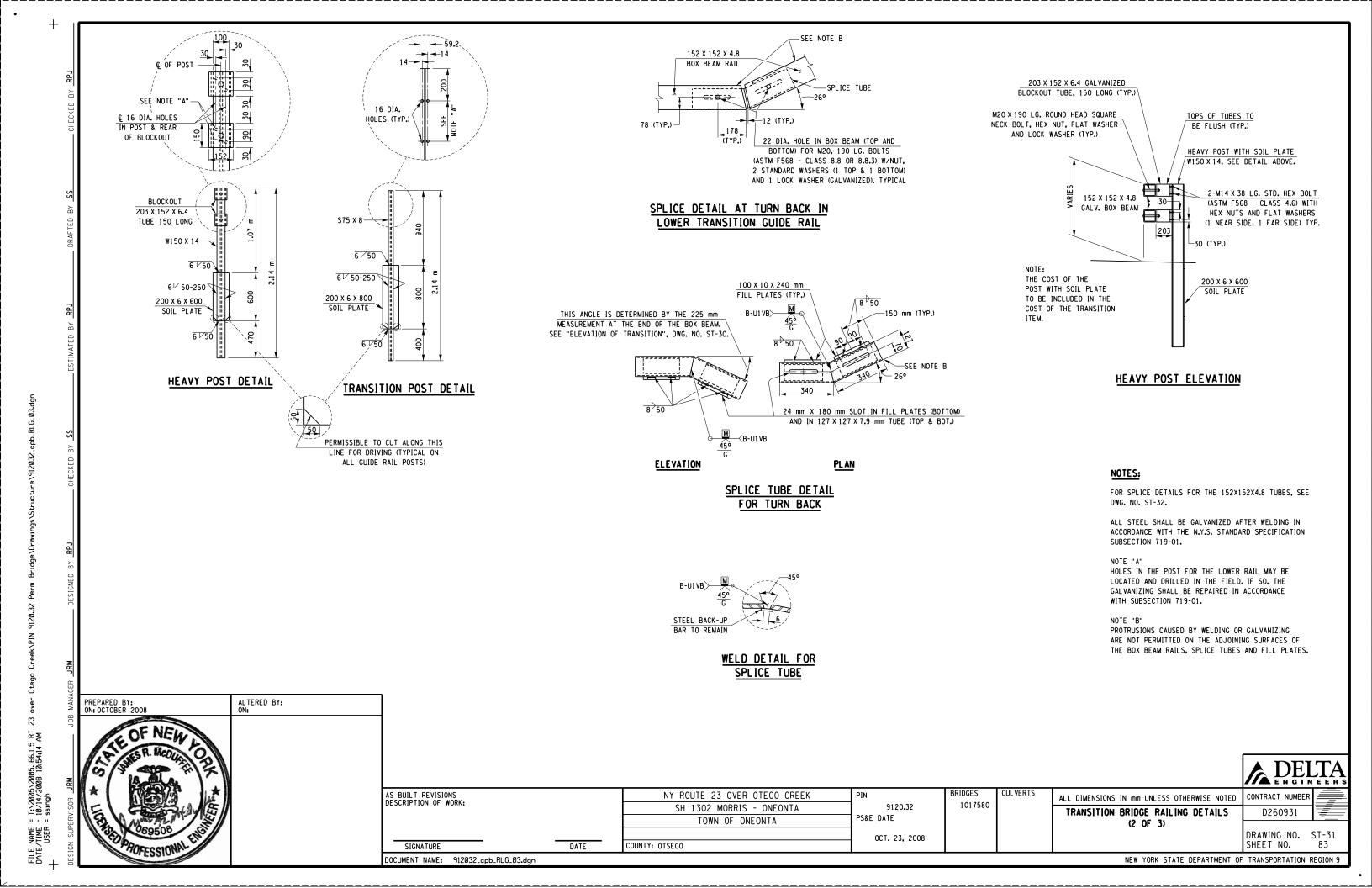
BRIDGES
1017580 CULVERTS
ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED
MISCELLANEOUS DETAILS

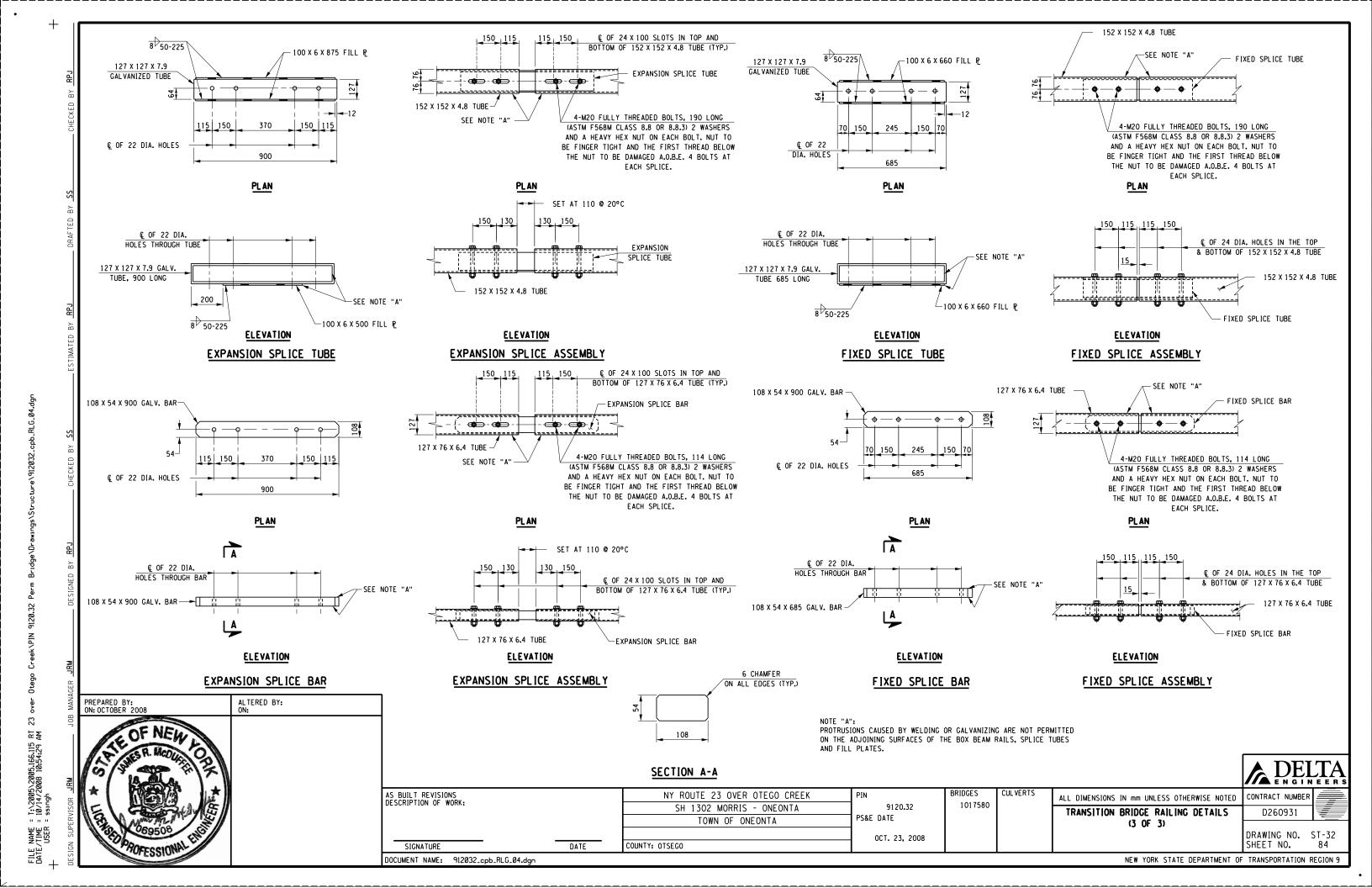
D260931

DRAWING NO. ST-28 SHEET NO. 80



Otego





Otego Creek\PIN 9120.32 Perm

USER = ACretelle FILE NAME = 5:\2005\54A - Rt 23 over DATE/TIME = 10/14/2008

PREPARED BY: NAGAPPA RAVINDRA ON: OCTOBER 2008 ALTERED BY: ON:

LENGTH

12.900

12.900

2.856

2.700

2.476

2.476

EAST ABUTMENT

16AE01

16AE04

19A05

19AE06

19A07

POUR 1 - ABUTMENT LOWER STEM

44

20

44

44

SUBTOTAL EPOXY COATED BARS =

SUBTOTAL PLAIN BARS =

44 3.792

TYPE

N19

N1

N1

N19

N1

WEIGHT

259

202

259 195

121

243

243

0.200 2.076

0.200 2.076

1.496 0.800 1.496

1.028 0.800 1.028

KG

624 KG

899

0.200

0.200

0.155

0.155

AS BUILT F DESCRIPTIO

DOCUMENT NAME: 912032af\_plt.dgn

BUILT REVISIONS		NY ROUTE 23 OVER OTEGO CREEK
CRIPTION OF WORK:		SH 1302 MORRIS - ONEONTA
		TOWN OF ONEONTA
SIGNATURE	DATE	COUNTY: OTSEGO

WEST ABUT. TOTAL PLAIN BARS =

WEST ABUT. TOTAL EPOXY COATED BARS =

12.900 12.900

2.700

BRIDGES	CULVERTS	Δ
1017580		

1206 KG

1730 KG

9120.32

OCT. 23, 2008

PS&E DATE

ALL DIMENSIONS IN mm UNLESS OTHERWISE NOTED | CONTRACT NUMBER BAR LIST (2 OF 2)

RAVI ENGINEERING & LAND SURVEYING, P.C.
CONSULTING ENGINEERS & SURVEYORS

D260931	1
DRAWING NO. SHEET NO.	ST-34 86

	AS BUILT REVISIONS DESCRIPTION OF WORK:
PESSIONAL PROPERTY.	CICMATURE

																		ii.
POUR 2 - NO	ORTH WING	VALL																
16A08	16	2.150	N19	53		0.675	0.800	0.675										
16AE09	5	4.370	N1	34													4.370	
16AE10	7	2.560	N1	28					NOTE: O	VARIES I	ROM 0.75	0 m TO 4	.370 m				2.560	
16AE11	16	2.504	N1	62					NOTE: O VARIES FROM 1.525 m TO 3.482 m								2.504	
16A12	7	4.370	N1	47													4.370	
16A13	7	2.560	N1	28					NOTE: O VARIES FROM 0.750 m TO 4.370 m								2.560	
16A14	16	2.504	N1	62					NOTE: O	VARIES I	ROM 1.52	25 m TO 3	.482 m				2.504	
16AE15	21	2.150	N19	70		0.675	0.800	0.675										
SUBTOTAL	DI AIN BADS	<u> </u>				191	KG						-					
SUBTOTAL PLAIN BARS = SUBTOTAL EPOXY COATED BARS =						194	KG											
POUR 3 - SC	OUTH WINGV	VALL																
16A08	17	2.150	N19	57		0.675	0.800	0.675										
16AE16	5	4.620	N1	36													4.620	
16AE17	7	2.685	N1	29					NOTE: O	VARIES I	ROM 0.75	0 m TO 4	.620 m				2.685	
16AE18	17	2.504	N1	66					NOTE: O VARIES FROM 1.525 m TO 3.482 m								2.504	
16A19	7	4.620	N1	50													4.620	
16A20	7	2.685	N1	29					NOTE: O VARIES FROM 0.750 m TO 4.620 m								2.685	
16A21	17	2.504	N1	66					NOTE: O	VARIES I	ROM 1.52	25 m TO 3	.482 m				2.504	
16AE15	22	2.150	N19	73		0.675	0.800	0.675										
SUBTOTAL	PLAIN BARS	 				202	KG											
SUBTOTAL FPOXY COATED BARS =					204	KG												
	<u> </u>	1																
POUR 4 - UP	PER STEM																	
19AE02	4	12.900	N1	115													12.900	
19AE22	8	2.700	N1	48													2.700	
19AE23	4	0.800	N1	7													0.800	
19A03	6	12.900	N1	173	1												12.900	
16AE15	44	2.150	N19	147	1	0.675	0.800	0.675										
16AE24	34	1.150	1	61	0.175	0.800					0.175			0.130				
13AE25	3	12.900	N1	38													12.900	
OUDTOTAL	DI AINI DADO					470	1/0											
SUBTOTAL PLAIN BARS = SUBTOTAL EPOXY COATED BARS =						173	KG											
SUBIOIAL	EPOXY COA	I ED BARS =				417	KG											
EAST ABUTMENT TOTAL PLAIN BARS =						1190	KG											
	EAST_ABUTMENT_TOTAL_EPOXY_COATED BARS =						KG						+	-		_	 1	

				1											1/		1		
MARK	NO.	LENGTH	TYPE	WEIGHT	Α	В	С	D	E	F	G	H H1	H2	J	K K1	K2	L	0	R
WEST ABUT	TMENT																		
DOUD 4 AF	NITMENT I O	WED OTEM											<u> </u>						
	BUTMENT LO		1140	057		4 404	0.000	4 404					<u> </u>						
16AE01	44	3.768	N19	257		1.484	0.800	1.484					<u> </u>					40.000	
19AE02	7	12.900	N1	201					-	-			<u> </u>					12.900	
19A03	9	12.900	N1	259		4 022	0.000	4.022	-	-			<u> </u>					12.900	
16AE04	44	2.866	N19	196		1.033	0.800	1.033	-	-			<u> </u>					0.700	
19A05	20	2.700	N1	121	0.000	0.070					0.000			0.455				2.700	
19AE06	44	2.473	1	243	0.200	2.073					0.200			0.155					
19A07	44	2.473	1	243	0.200	2.073					0.200		<u> </u>	0.155					
SURTOTAL	PLAIN BARS	!=			-	623	KG	-	1	-	-		1			-			
	EPOXY COA					897	KG						1						
SUBTUTAL	I	I ED BARS -			-	091	NG	-	1	-	-		1			-			
POUR 2 - SC	I DUTH WINGW	<u>Ι</u> /ΔΙΙ											1						
16A08	16	2.150	N19	53		0.675	0.800	0.675					<del>                                     </del>						
16AE09	5	4.330	N1	34		0.073	0.000	0.073					1					4.330	
16AE10	7	2.540	N1	28					NOTE: O	VARIES	FROM 0.7	50 m T O 4	330 m					2.540	
16AE11	16	2.505	N1	62					NOTE: O VARIES FROM 0.750 m TO 4.330 m NOTE: O VARIES FROM 1.535 m TO 3.475 m									2.505	
16A12	7	4.330	N1	47					14012.0	I	1.0.	J	T					4.330	
16A13	7	2.540	N1	28					NOTE: O	VARIES	FROM 0.7	50 m T O 4	330 m					2.540	
16A14	16	2.505	N1	62						VARIES								2.505	
16AE15	21	2.150	N19	70		0.675	0.800	0.675	NOTE. O	VAIGLO	I KOW 1.3	J 111 1 0 3	I					2.303	
IUALIS	- 21	2.100	1413	70		0.073	0.000	0.073					1						
SUBTOTAL	L PLAIN BARS	<u>                                     </u>				190	KG						<u> </u>						
	SUBTOTAL FLAM BARS =					193	KG						-						
	1	1											1						
POUR 3 - NO	DRTH WINGW	/ALL											1						
16A08	18	2.150	N19	60		0.675	0.800	0.675					1						
16AE16	5	4.830	N1	37		0.0.0	0.000	0.0.0					1					4.830	
16AE17	8	2.790	N1	35					NOTE: O	VARIES	FROM 0.75	50 m TO 4	.830 m					2.790	
16AE18	18	2.628	N1	73											2.628				
16A19	7	4.830	N1	52					NOTE: O VARIES FROM 1.535 m TO 3.721 m									4.830	
16A20	8	2.790	N1	35					NOTE: O	VARIES	FROM 0.75	50 m TO 4	.830 m					2.790	
16A21	18	2.628	N1	73						VARIES								2.628	
16AE15	23	2.150	N19	77		0.675	0.800	0.675					1						
													<u> </u>						
SUBTOTAL	PLAIN BARS	} =				221	KG												
SUBTOTAL	EPOXY COA	TED BARS =				222	KG												
													1						
POUR 4 - UF	PER STEM												1						
19AE02	4	12.900	N1	115														12.900	
19AE22	8	2.700	N1	48														2.700	
19AE23	4	0.800	N1	7									1					0.800	
19A03	6	12.900	N1	173							1		l				l	12.900	
16AE15	44	2.150	N19	147		0.675	0.800	0.675					1						
16AE24	34	1.150	1	61	0.175	0.800					0.175		<b>†</b>	0.130					
13AE25	3	12.900	N1	38							1		l				l	12.900	
								l					<b>†</b>				l		
SUBTOTAL	PLAIN BARS	; =				173	KG				1		1						
		TED BARS =				417	KG	1		1		1	1				i		
					<del>                                     </del>			<del>                                     </del>	<del>                                     </del>	1	<del>                                     </del>	1	<b>†</b>	1		<del>                                     </del>			