ABC Innovative Projects

640 th Street E	Bridge	over Br	anch Raccoon	River							
Location	640 th Street over Branch Raccoon River southeast of the city of Storm Lake in Buena Vista County										
State	Iowa										
Owner	Buena	Vista Cou	nty								
Year ABC Built	2009										
State ID #	30365										
NBI#	84511										
Coordinates	Latitu	de: 42.590	006563		Longitude	• <i>:</i> -9	5.0583190	4			
Contact Person	Jon Ites Buena Vista County Engineer Storm Lake, Iowa Phone: 712-749-2704 Email: jites@co.buena-vista.ia.us										
Mobility Impact Time	ABC:		replace bridge (14 days for approach		Conventio	nal:	3-month closure				
Impact	7	Tier 1	Tier 2	T	ier 3		Tier 4	Tier 5			
Category					X						
Primary Driver(s) Description	This bridge replacement was part of an IBRC project. In addition, primary drivers were: • reduced onsite construction time • improved site constructability • minimized environmental impacts										
Description	 50.83-ft-long and 28-ft-wide single-span precast adjacent box beam bridge Rural location Average Daily Traffic count: 45 (2003) Traffic management alternative, if constructed conventionally: extended use of 1-mile detour Existing Bridge: The existing single-span steel I-beam bridge was 26.67-ft long and 18-ft wide with cast-in-place concrete substructure. Built in 1936, the bridge was posted and required replacement. Replacement Bridge: The 28-ft-wide replacement bridge is on an unpaved low-volume county road. The cross-section consists of seven 4-ft-wide 21-inch-deep adjacent pretensioned concrete box beams. The substructure consists of precast abutment caps founded on HP 10x57 steel piles, with separate precast backwall/wingwall units. This is the first bridge in lowa to include precast backwalls. Construction Methods: The precast caps, backwall/wingwall units, and box beams were fabricated in a precast plant and trucked to the site. The 3-ft-wide precast abutment caps are 28.17 ft long with five 1.75-ft-diameter pockets formed using full-depth galvanized corrugated metal pile sleeves. The caps vary from 3 ft deep on the sides to 3.24 ft in the middle to accommodate the roadway cross-slope. Each 1-ft-wide 1.79-ft-high precast backwall is composed of two 17.08-ft-long units, each with a 3.83-ft-deep beveled wingwall that 										

extends 3 ft past the abutment cap. Each box beam was fabricated with three polystyrene-foam voids to reduce self-weight (a 12-inch-diameter void on each side of a 10-inch-diameter void). Exterior box beams were fabricated with inserts in the exterior face for traffic rail attachment.

Prior to being shipped to the bridge site, the individual precast elements were inspected and partially assembled to ensure proper fit-up in the field. This also allowed the contractor, new to precast substructure construction, to become familiar with the elements and discuss the construction sequence.

The bridge was closed and traffic detoured. The existing bridge was removed in one day. Templates were used to accurately position the five piles at each abutment. The piles could not deviate by more than three inches in any direction to accommodate the precast abutment cap pockets. The ten piles were driven and cut to the required length in one day.

The abutment caps were erected over the piles with a mobile crane and supported on temporary blocking. The cap pockets were filled with concrete and allowed to cure over the weekend. The reinforcement bars for the backwall-to-cap connection were doweled into position.

The beams were erected onto neoprene pads, with the middle beam erected first and the exterior beams erected last to ensure proper tolerances. The total time from arrival of the first beam until the last beam was placed was four hours in spite of one truck getting lost between the precast plant and the bridge site. Actual placement time for all seven beams was approximately an hour. The 1-inch-diameter transverse tie rods were threaded through the beams at third points, and coupling nuts at the blockouts between beams were tightened. After the ungrouted transverse tie assembly was tightened, 1.5-inch-diameter holes were drilled one foot into the abutment caps, using the holes in the precast beam ends as guides. A 2.25-ft-long 1.5-inch-diameter dowel was placed in each hole, and the holes were epoxied.

The four backwall/wingwall units were transported to the site on two trucks. An epoxy layer was placed on the top surface of the cap, and the reinforcement bars extending from the cap were threaded into the backwall/wingwall unit as it was lowered onto the epoxy layer. The remaining three backwall/wingwall units were similarly installed. Polystyrene backer rods were placed in the shear keys between beams as required. The shear keys and blockouts between the beams and the voids between the backwall units and between the backwalls and box beams were filled with non-shrink grout. The bridge railing was then attached to the exterior box beams using anchor bolts that had been cast in the beams.

The road on which the bridge was located was closed September 10, 2009 and the new bridge was completed on September 14, 2009. County crews completed the dirt work and guardrails for the approaches to the new bridge in 14 days. If it had been necessary to open the road sooner, the work could have been bid out and completed more quickly to reduce the total number of days the bridge was closed.

The contract required the bridge be replaced within 10 working days, compared to a 90-day contract period for a typical conventional bridge replacement project. An incentive of \$5,000 per day was included for early completion, with a maximum of \$25,000. The contract also specified a disincentive of \$400 per day for late completion. The contractor replaced the bridge in five days and received the maximum incentive.

Stakeholder Feedback:

	Upon completion, the bridge was instrumented and load tested using two county trucks loaded with gravel. Approximately one year later, instrumentation was re-installed and the bridge was tested a second time to determine any changes in its performance. There was no change in performance. The county was pleased with the design and speed of the project.										
High Performance Materials	•										
Additional photos											
Project	Decision-Making	Tools	Site Procureme	ent	Pro	ject Delivery		Contracting			
Planning	•	,		Design-bid-build		Full lane closureIncentive / disincentive clauses					
Geotechnical Solutions	Fou	& Walls		Rapid Embankment							
	•										
Structural	Prefa	Prefabricated Bridge Elements & Systems Construction									
Solutions	Elements	Systems M		Misce	llaneous	•					
	Adjacent box be Precast abutme Precast backwa Precast wingwa	ent caps alls	•	 Grouted keys CIP pockets in precast substructure Epoxy joints							
Costs	The engineer's estimate for the project was \$200,000. The low bid was \$238,500 (7% higher than the engineer's estimate minus incentive). There were four bidders. The cost per square foot of bridge was \$170 compared to \$80 for conventional construction in this region in 2009.										
Funding	Federal only		State only		Federal and State			Other			
					X						
Incentive Program (\$)	Highways for LIF	Highways for LIFE IBRC \$200,000			SHRP2			Other			
Contract Plans	Complete Set:	Let Plan	Plans (link to pdf)			ABC *: Bric	lge F	Plans (link to pdf)			
Specifications	Complete Set: Proposal Addendum (link to pdf) ABC *:										
Bid Tabs	Bid Tabs (link to pdf)										
Schedule	Engineer's: No	ble.	Ac	Actual:							
Other Related Information	February 2012 ISU Evaluation Report (link to pdf)										
Photo Credits	Iowa Departmen	nt of Trai	nsportation; Bue	na Vis	sta Co	ounty; Iowa S	tate	University			

^{*} Specific to the ABC used in the project.