ABC Innovative Projects

Lagation	e over Monie	Creek							
Location	on MD Route 362 (Mount Vernon Road, a rural major collector) over Monie Creek in Somerset County near the town of Jason						Monie Creek in		
State	Maryland	Maryland							
Owner	State								
Year ABC Built	2009								
State ID #	19005								
NBI#	100000190005010								
Coordinates	Latitude: 38.22	540		Longitude:	-75	5.75220			
Contact Person	Jeffrey Robert, P.E. Senior Project Engineer, Office of Structures Maryland State Highway Administration Phone: 410-545-8327 Email: jrobert@sha.state.md.us								
Mobility Impact Time	ABC: 37-day clo	` ` ' II				nventional: 10 weeks for ≤55-ft spans			
Impact	Tier 1	Tier 2	7	ier 3	7	ïer 4	Tier 5		
Category						Х			
Benefits		npacts, reduced on al quality and produ							
	 55-ft-long and 45-ft-wide single-span prestressed concrete slab bridge Rural location Average Daily Traffic count: 1,700 (2005); 2,800 (projected 2030) Traffic management alternative, if constructed conventionally: extended use of 7-mile detour for low-weight vehicles (13.7 miles for trucks). Existing Bridge: The existing bridge was a 41-ft-long, 48-ft-wide single-span steel beam bridge with timber substructure. Built in 1934 and widened in 1963, the bridge had two 12-ft-wide traffic lanes, and two 12-ft-wide shoulders compared to two 8.5-ft-wide shoulders on the approaches. The substructure was structurally deficient and required replacement. Replacement Bridge: The replacement bridge has two 12-ft-wide traffic lanes and two 8.5-ft-wide shoulders. The cross-section consists of eleven 4-ft-wide 2.25-ft-deep prestressed concrete solid slab beams that are post-tensioned together transversely, with a 5-inch-thick reinforced cast-in-place modified latex concrete overlay and precast curbs with embedded rods to connect metal railing. The abutments consist of 4-ft-square precast abutment caps on 12.75-ft-diameter steel pipe piles, and precast wings. The MDSHA gave the contractor several options for constructing the abutments in pieces to allow weights of abutment sections to be no greater than slab beam weights. Construction Methods: The contractor chose to precast the abutment caps in half-sections with cast-in-place 								

The abutment caps were cast with partial-depth pockets for the piles to fit into, and circular pipes that extend from the top of the pocket to the top surface of the cap for completion of the closure joints. The caps were cast with threaded inserts in the top of the pockets to allow connection of development reinforcement in the field. The use of threaded inserts simplified fabrication and allowed the caps to be shipped upright instead of upside-down, eliminating the additional step and complexity of flipping the caps in the field.

The contractor drove the piles using single-lane weekend closures prior to closing the bridge. The piles were driven in a single row with a 6-inch tolerance to create a full-moment connection between the piles and abutment while allowing some movement. Steel caps were field-welded on top of the piles and covered with asphalt to allow traffic to be maintained.

The bridge was then closed, the superstructure demolished, and the piles excavated. The piles were cut to the correct elevation and filled with concrete to 20 ft below the bottom of footing elevation. A 19-ft-long reinforcement cage was lowered into each pile, and temporary leveling pads were welded to the piles to support the abutment caps. The caps were delivered to the site, and 4-ft-long development bars were threaded into the inserts in the tops of the cap pockets. The caps were then lowered over the piles. Mechanical coupler splices were installed to develop the top and bottom reinforcement that extended from the caps into the 2-ft-wide closure joints. The closure joints and cap pockets were then cast.

Because the bridge was directly against the stream and the ground elevations on site were close to sea level, driven sheet piling was specified in the contract for scour protection. The contractor installed the sheeting after excavation, before the piles were cut at their final elevation, to help dewater the area around the abutment. After the abutment cap was erected, the sheeting was connected to the cap with tie rods placed through preformed holes in the caps. The tie rods connected to a small waler in front of the sheeting. A top mat of reinforcement was placed over the top of the waler and sheeting, and the area between the existing and new abutment was filled with cast-in-place concrete as a cap for the sheeting. The scour protection and abutments were designed as a system, helping to minimize the section requirements of both the piles and the sheeting. After casting the cap, the existing timber abutment was cut off to the top of the cap and the formwork around the wing walls was removed.

The contractor then placed the slab beams, grouted the shear keys between beams, post-tensioned the beams together, and added reinforcement for the cast-in-place overlay. Immediately prior to placing the overlay, the contractor lifted the reinforcement mat with a crane and swept a layer of grout on the beams. This was required to ensure good contact between the beams and latex modified concrete overlay. The contractor then placed the overlay after applying a horizontal bond breaker between the abutment cap and overlay to create a semi-integral connection that allows for the typical ¼-inch movement. While the overlay cured for seven days, the contractor finished casting the wing walls, installed the bridge railing, and did other finish work. The bridge was then opened for traffic.

The contract required the bridge to be closed a maximum of 35 days (five weeks). It included an incentive of \$6,000 per day if the bridge was opened in 30 days or less, with a maximum of \$30,000. A disincentive of \$6,000 per day greater than 35 days was also included. The bridge was completed in 35 days and opened in 37 days, on August 14, the day before the first day of school.

	Stakeholder Fe	eedbac	:k:						
	The main construction steps that achieved the shorter timeline were driving the piles with weekend lane closures prior to closing the bridge, minimizing the dewatering requirements by using sheet piling, and using precast abutments and wing walls.								
High Performance Materials	•								
Additional photos									
Project	Decision-Making	Tools	Site Procureme	nt	Project I	Delivery		Contracting	
Planning	•		•	•				Full lane closure Incentive / disincentive clauses	
Geotechnical Solutions	Foundations & Walls				Rapid Embankment				
	•								
Structural Solutions	Prefabricated Bridge Elements & Systems Construction								
	Adjacent slab I Precast abutm Precast wingw Steel sheet pili	ent cap alls	Systems	reinforced concrete ure joints uted keys s in splice couplers uted PT ducts pockets in precast caps ex-modified overlay					
Costs	The engineer's estimate for the project was \$1.6 million. The low bid was \$1.7 million (\$100,000 = 6% higher than engineer's estimate). There were 5 bidders (4 bidders selected the precast element options, 1 selected conventional cast-in-place). The cost per square foot of bridge was \$308 compared to \$280 for conventional construction in this region in 2009.								
Funding	Federal only		State only	-	Federal a			Other	
	History 6 1155		1000		X			0.4	
Incentive Program (\$)	Highways for LI	FE	IBRD	SHRP2			Other		
Contract Plans	Complete Set:	Contra	act Plans (link to p	df)	ABC *:				
Contract i lans		CD D	roject Description ((link to doc)			s/Disincentives		
Specifications	Complete Set:	doc)	otice to Contractor	_(link		(IIIIK to do	C)		
	Complete Set:	doc) SP-No to doc	otice to Contractor	_ (link		(IIIIK to do	(c)		
Specifications	·	doc) SP-No to doc txt)	otice to Contractor	_ (link	Actual:	(IIIIK to do			

Information	
	"Accelerated Bridge Construction Project: the Replacement of MD 362 over Monie Creek," 2010 FHWA ABC Conference Proceedings (link to Conference CD)
Photo Credits	Maryland State Highway Administration

^{*} Specific to the ABC used in the project.